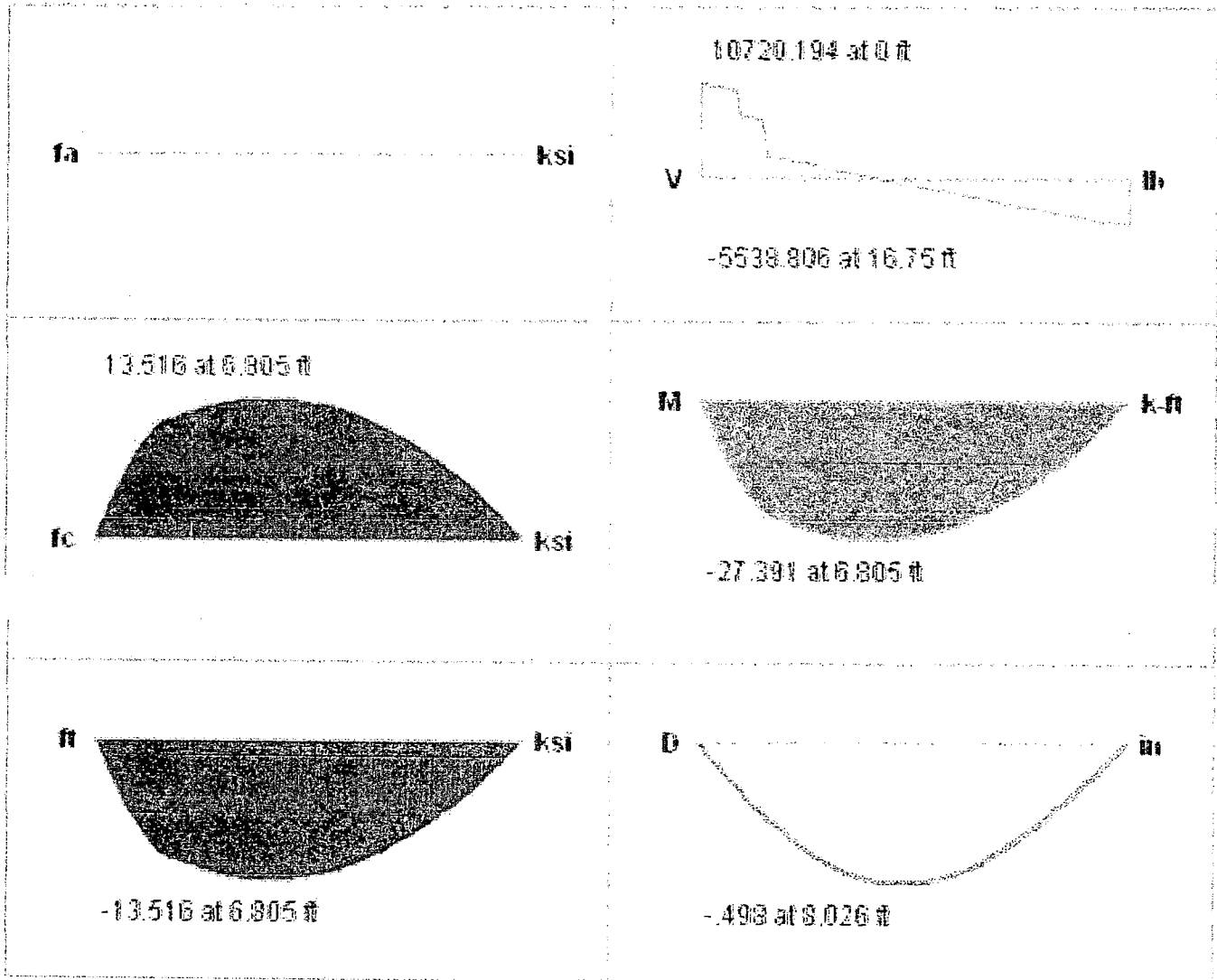


Beam: **WT-3**
 Shape: **W8X28**
 Material: **A36 Gr. 36**
 Length: **16.75 ft**
 I Joint: **N5**
 J Joint: **N6**

LC 1:1
 Code Check: **0.609 (bending)**
 Report Based On **97** Sections



AISC 13th ASD Code Check

Max Bending Check	0.609	Max Shear Check	0.324
Location	6.805 ft	Location	0 ft
Equation	H1-1b	Max Def Ratio	L/404
Bending Flange	Compact	Compression Flange	Non-Slender
Bending Web	Compact	Compression Web	Non-Slender

Fy	36 ksi	Out Plane	In Plane
Pn/om	79225.57 lb	Lb	16.75 ft
Pn/om	177844.311 lb	KL/r	123.935
Mn/om	44.966 k-ft	Sway	No
Vn/om	33.078 lb	L Comp Flange	16.75 ft
Cb	1.117		

SHIPPING TICKET

CUSTOMER KINGS WAY HOMES INC.	QUOTE # B1000359	ORDER # J1000359
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ROOF TRUSSES	LOADING INFORMATION	<table border="1" style="font-size: small;"> <tr> <td>TCLL-TCDL-BCLL-BCDL</td> <td>STRESS INCR</td> </tr> <tr> <td>30.0,7.0,0.0,10.0</td> <td>1.15</td> </tr> </table>	TCLL-TCDL-BCLL-BCDL	STRESS INCR	30.0,7.0,0.0,10.0	1.15	ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)
TCLL-TCDL-BCLL-BCDL	STRESS INCR						
30.0,7.0,0.0,10.0	1.15						

PROFILE	QTY	PITCH		TYPE ID	BASE SPAN	O/A SPAN	LUMBER		OVERHANG		CANTILEVER		HEELS		CHECKED BY	QTY SHIPPED
		PLY	TOP				BOT	TOP	BOT	LEFT	RIGHT	LEFT	RIGHT	LEFT		
	2		7.00	0.00	Jack-Open J15	01-11-04	01-11-04	2 X 4	2 X 4	00-11-04				00-04-15	00-10-11	
	1		12.00	0.00	Common Truss T01E	23-06-00	23-06-00	2 X 4	2 X 4	00-11-04	00-11-04			01-04-00	01-04-00	
	2		12.00	0.00	Common Truss T02	23-06-00	23-06-00	2 X 4	2 X 4	00-11-04	00-11-04			01-04-00	01-04-00	
	11		12.00	12.00	Special Truss T03	23-00-00	23-00-00	2 X 4	2 X 4	00-11-04				01-03-00	01-04-00	
	1		12.00	0.00	Special Truss T04	21-05-04	21-05-04	2 X 4	2 X 4	00-11-04				01-10-12	01-04-00	
	1		12.00	0.00	Common Truss T05E	11-08-00	11-08-00	2 X 4	2 X 4	00-11-04	00-11-04			01-04-00	01-04-00	
	1 2 Ply		12.00	0.00	Common Truss T06G	11-08-00	11-08-00	2 X 4	2 X 6					01-04-00	01-04-00	
	1		7.00	0.00	Hip Truss T07G	14-07-00	14-07-00	2 X 4	2 X 6	00-11-04	00-11-04			00-10-11	00-10-11	
	2		7.00	0.00	Common Truss T08	14-07-00	14-07-00	2 X 4	2 X 4	00-11-04				00-10-11	00-10-11	
	1 2 Ply		7.00	0.00	Hip Truss T09G	42-00-00	42-00-00	2 X 4	2 X 6	00-11-04	00-11-04			00-10-11	00-10-11	
	1 2 Ply		7.00	0.00	Hip Truss T09GA	42-00-00	42-00-00	2 X 4	2 X 6	00-11-04	00-11-04			00-10-11	00-10-11	
	2		7.00	0.00	Hip Truss T10	42-00-00	42-00-00	2 X 4	2 X 4	00-11-04	00-11-04			00-10-11	00-10-11	
	2		7.00	0.00	Hip Truss T11	42-00-00	42-00-00	2 X 4	2 X 4	00-11-04	00-11-04			00-10-11	00-10-11	
	1		7.00	0.00	Hip Truss T12	42-00-00	42-00-00	2 X 4	2 X 4	00-11-04	00-11-04			00-10-11	00-10-11	
	1		7.00	0.00	Hip Truss T12A	41-08-14	41-08-14	2 X 4	2 X 4	00-11-04				01-00-08	00-10-11	
	1		7.00	0.00	Common Truss T13	42-00-00	42-00-00	2 X 4	2 X 4	00-11-04	00-11-04			00-10-11	00-10-11	
	6		7.00	0.00	Common Truss T13A	41-08-14	41-08-14	2 X 4	2 X 4	00-11-04				01-00-08	00-10-11	
	1		7.00	0.00	Hip Truss T14G	18-00-00	18-00-00	2 X 4	2 X 6	00-11-04	00-11-04			00-10-11	00-10-11	
	4		7.00	0.00	Common Truss T15	18-00-00	18-00-00	2 X 4	2 X 4	00-11-04	00-11-04			00-10-11	00-10-11	
	4		7.00	0.00	Common Truss T16	18-00-00	18-00-00	2 X 4	2 X 4	00-11-04				00-10-11	00-10-11	
	3		7.00	0.00	Common Truss T16A	17-10-04	17-10-04	2 X 4	2 X 4	00-11-04				00-11-11	00-10-11	
	2		4.00	0.00	Monopitch T17E	08-00-00	08-00-00	2 X 4	2 X 4	00-11-04				03-03-08	00-07-08	
	13		4.00	0.00	Monopitch T18	08-00-00	08-00-00	2 X 4	2 X 4	00-11-04				03-03-08	00-07-08	

ITEMS

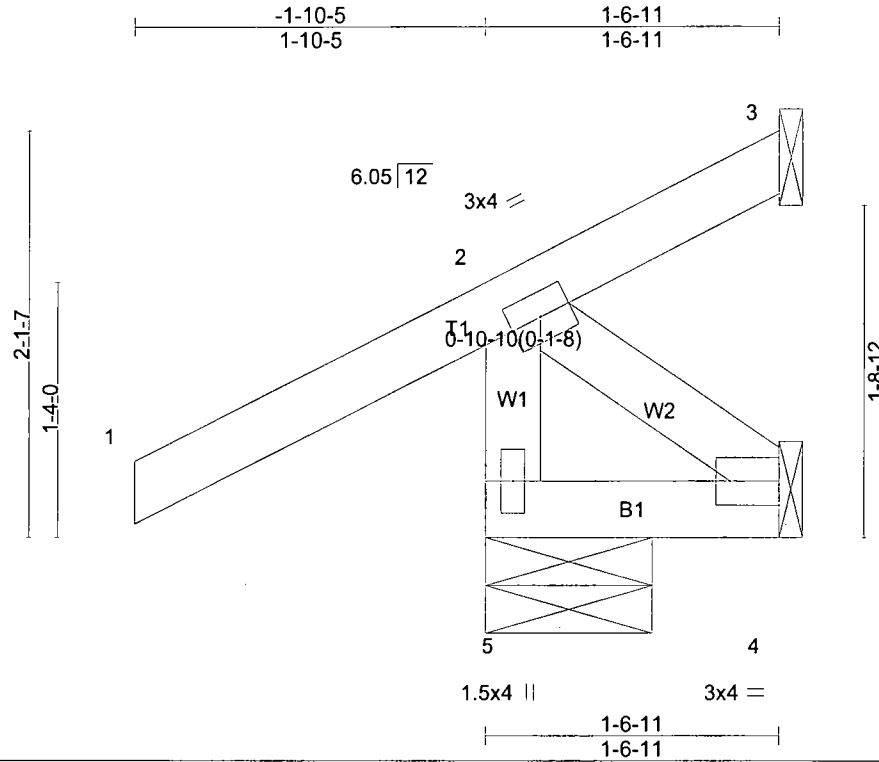
QTY	ITEM TYPE	SIZE	LENGTH FT-IN-16	NOTES	CHECKED BY	QTY SHIPPED
1	EWP	1 3/4x9 1/2 V-LAM SP 2.0 3100	10-00-00			
130	Hardware	H2.5T				
6	Hardware	HUS26				
16	Hardware	LUS24				
6	Hardware	LUS26				
11	Hardware	MUS26				

	Rec'd By:	
	Rec'd Time:	
	Rec'd Date:	
	Del By:	
	Del Date:	

Job J1000359	Truss CJ01	Truss Type Jack-Open Truss	Qty 2	Ply 1	KINGS WAY - PIERRI RES. #5302
					Job Reference (optional)

Richco Structures, Sheboygan Falls, WI, Heather Kraning

7.220 s Dec 29 2009 MiTek Industries, Inc. Wed Jan 27 08:38:59 2010 Page 1



Scale = 1:12.3

Plate Offsets (X,Y): [4:Edge,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 30.0	Plates Increase 1.15	TC 0.49	Vert(LL) -0.00	5	>999	240	MT20	197/144
TCDL 7.0	Lumber Increase 1.15	BC 0.02	Vert(TL) -0.00	5	>999	180		
BCLL 0.0	Rep Stress Incr YES	WB 0.07	Horz(TL) -0.00	3	n/a	n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002	(Matrix)						Weight: 9 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SPF No.1 or SPF No.2	TOP CHORD Structural wood sheathing directly applied or 1-6-11 oc purlins, except end verticals.
BOT CHORD 2 X 4 SPF No.1 or SPF No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SPF No.1 or SPF No.2 *Except* W2: 2 X 4 SPF Stud	

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

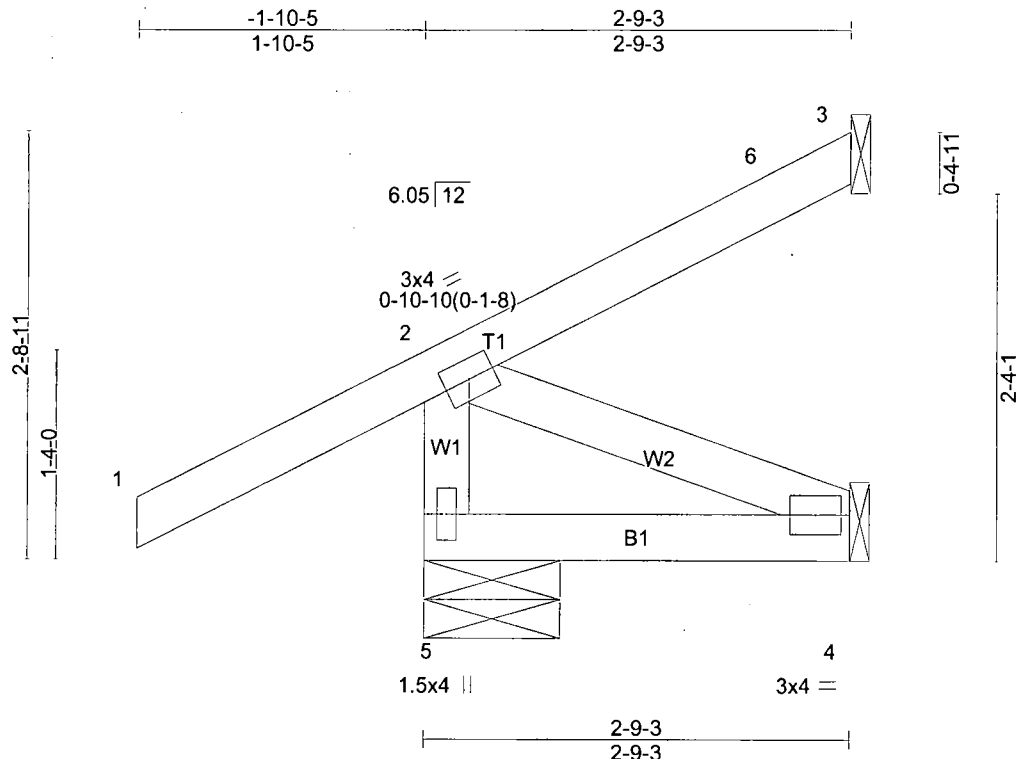
REACTIONS (lb/size) 5=321/0-10-10 (min. 0-1-8), 4=14/Mechanical, 3=-55/Mechanical
 Max Horz 5=73(LC 9)
 Max Uplift 5=-105(LC 9), 4=-48(LC 9), 3=-55(LC 1)
 Max Grav 5=321(LC 1), 4=28(LC 2), 3=54(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-5=-307/336

NOTES

- 1) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Corner(3) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 3 except (jt=lb) 5=105.
- 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



Scale = 1:15.0

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 30.0	Plates Increase 1.15	TC 0.48	Vert(LL) -0.00 4-5 >999 240	MT20	197/144
TCDL 7.0	Lumber Increase 1.15	BC 0.07	Vert(TL) -0.01 4-5 >999 180		
BCLL 0.0	Rep Stress Incr YES	WB 0.08	Horz(TL) -0.00 3 n/a n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002	(Matrix)			
				Weight: 13 lb	

<p>LUMBER</p> <p>TOP CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>BOT CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>WEBS 2 X 4 SPF No.1 or SPF No.2 *Except* W2: 2 X 4 SPF Stud</p>	<p>BRACING</p> <p>TOP CHORD Structural wood sheathing directly applied or 2-9-3 oc purlins, except end verticals.</p> <p>BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.</p> <div style="border: 1px solid black; padding: 5px; margin-top: 10px;"> <p>MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</p> </div>
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REACTIONS (lb/size) 5=328/0-10-10 (min. 0-1-8), 3=36/Mechanical, 4=26/Mechanical
 Max Horz 5=92(LC 9)
 Max Uplift 5=-91(LC 9), 3=-16(LC 6), 4=-23(LC 9)
 Max Grav 5=328(LC 1), 3=37(LC 7), 4=52(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-5=-301/325
 WEBS 2-4=0/259

- NOTES**
- 1) Wind: ASCE 7-05; 90mph; TC DL=4.2psf; BC DL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Corner(3) -1-10-5 to 2-4-9, Exterior(2) 2-4-9 to 2-8-7 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) Refer to girder(s) for truss to truss connections.
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3, 4.
 - 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

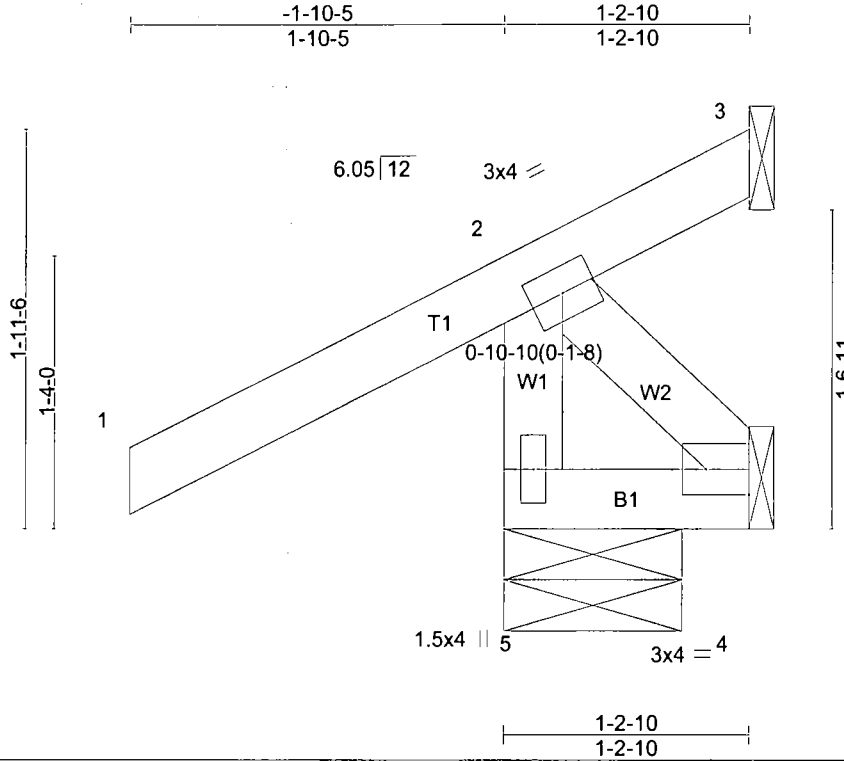


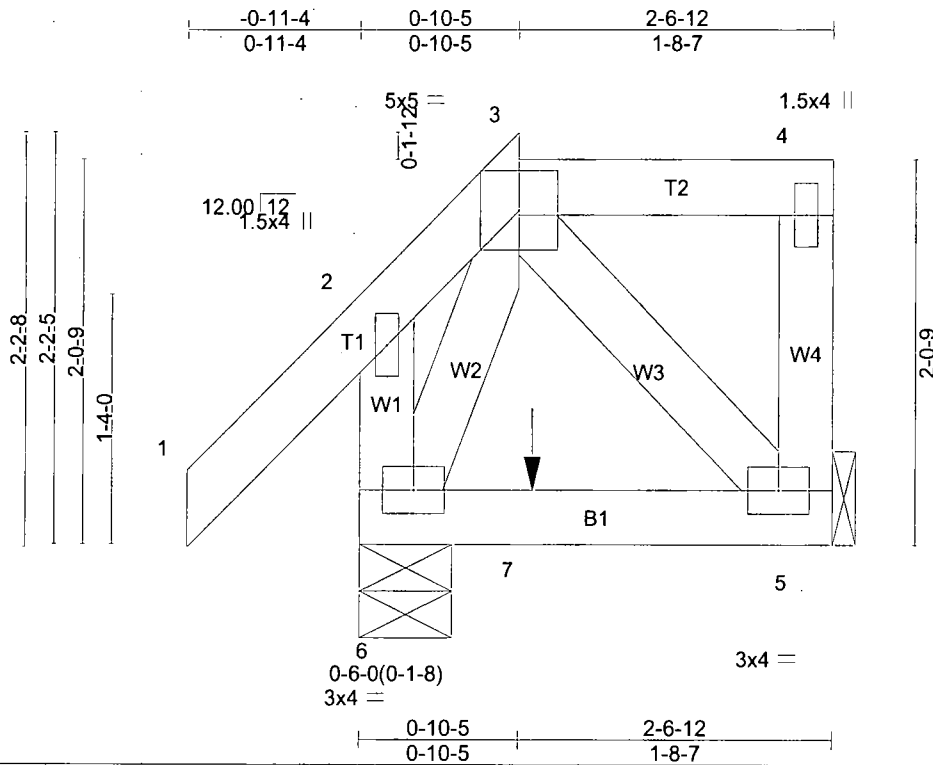
Plate Offsets (X,Y): [4:Edge,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 30.0	Plates Increase	1.15	TC 0.49	Vert(LL)	-0.00	5	>999	MT20	197/144
TCDL 7.0	Lumber Increase	1.15	BC 0.01	Vert(TL)	-0.00	5	>999		
BCLL 0.0	Rep Stress Incr	YES	WB 0.08	Horz(TL)	-0.00	3	n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002		(Matrix)						Weight: 8 lb

<p>LUMBER</p> <p>TOP CHORD 2 X 4 SPF No.1 or SPF No.2 BOT CHORD 2 X 4 SPF No.1 or SPF No.2 WEBS 2 X 4 SPF No.1 or SPF No.2 *Except* W2: 2 X 4 SPF Stud</p> <p>REACTIONS (lb/size) 5=339/0-10-10 (min. 0-1-8), 4=11/Mechanical, 3=-101/Mechanical Max Horz 5=67(LC 9) Max Uplift 5=-117(LC 9), 4=-62(LC 9), 3=-101(LC 1) Max Grav 5=339(LC 1), 4=21(LC 2), 3=84(LC 9)</p> <p>FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-5=-328/362</p>	<p>BRACING</p> <p>TOP CHORD Structural wood sheathing directly applied or 1-2-10 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.</p> <div style="border: 1px solid black; padding: 5px; font-size: x-small; margin-top: 5px;"> MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. </div>
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- NOTES**
- 1) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Corner(3) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) Refer to girder(s) for truss to truss connections.
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 5=117, 3=101.
 - 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d	PLATES	GRIP
TCLL 30.0	Plates Increase	1.15	TC 0.13	Vert(LL) -0.00	5-6 >999 240	MT20	197/144
TCDL 7.0	Lumber Increase	1.15	BC 0.08	Vert(TL) -0.01	5-6 >999 180		
BCLL 0.0	Rep Stress Incr	NO	WB 0.02	Horz(TL) -0.00	5 n/a n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002		(Matrix)				
							Weight: 15 lb

<p>LUMBER</p> <p>TOP CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>BOT CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>WEBS 2 X 4 SPF Stud *Except*</p> <p>W1: 2 X 4 SPF No.1 or SPF No.2</p>	<p>BRACING</p> <p>TOP CHORD Structural wood sheathing directly applied or 2-6-12 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.</p> <p>BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.</p> <p>JOINTS 1 Brace at Jt(s): 4</p> <div style="border: 1px solid black; padding: 2px; font-size: x-small;"> <p>MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</p> </div>
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REACTIONS (lb/size) 5=98/Mechanical, 6=226/0-6-0 (min. 0-1-8)

Max Horz 6=74(LC 7)

Max Uplift 5=-37(LC 4), 6=-45(LC 7)

Max Grav 5=107(LC 12), 6=226(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 6.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 31 lb down and 7 lb up at 0-11-3 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf)
- Vert: 1-2=-74, 2-3=-74, 3-4=-74, 5-6=-20
- Concentrated Loads (lb)
- Vert: 7=-31(F)

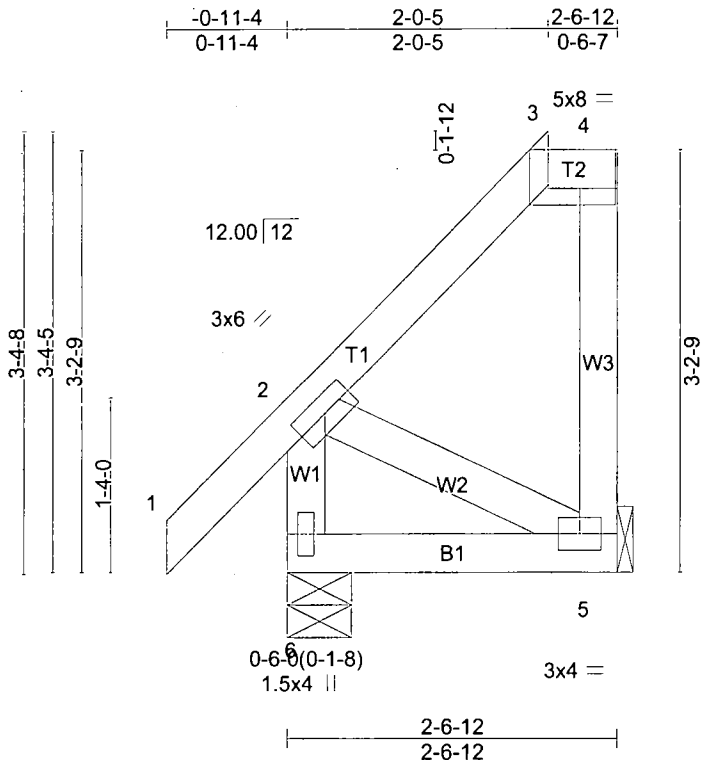


Plate Offsets (X,Y): [3:0-6-4,Edge]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 30.0	Plates Increase 1.15	TC 0.12	Vert(LL) -0.00 5-6 >999 240	MT20	197/144
TCDL 7.0	Lumber Increase 1.15	BC 0.03	Vert(TL) -0.00 5-6 >999 180		
BCLL 0.0	Rep Stress Incr YES	WB 0.04	Horz(TL) -0.00 5 n/a n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002	(Matrix)			Weight: 15 lb

<p>LUMBER</p> <p>TOP CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>BOT CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>WEBS 2 X 4 SPF Stud *Except*</p> <p style="margin-left: 20px;">W1: 2 X 4 SPF No.1 or SPF No.2</p>	<p>BRACING</p> <p>TOP CHORD Structural wood sheathing directly applied or 2-6-12 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.</p> <p>BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.</p> <p>JOINTS 1 Brace at Jt(s): 4</p>
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MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=88/Mechanical, 6=206/0-6-0 (min. 0-1-8)

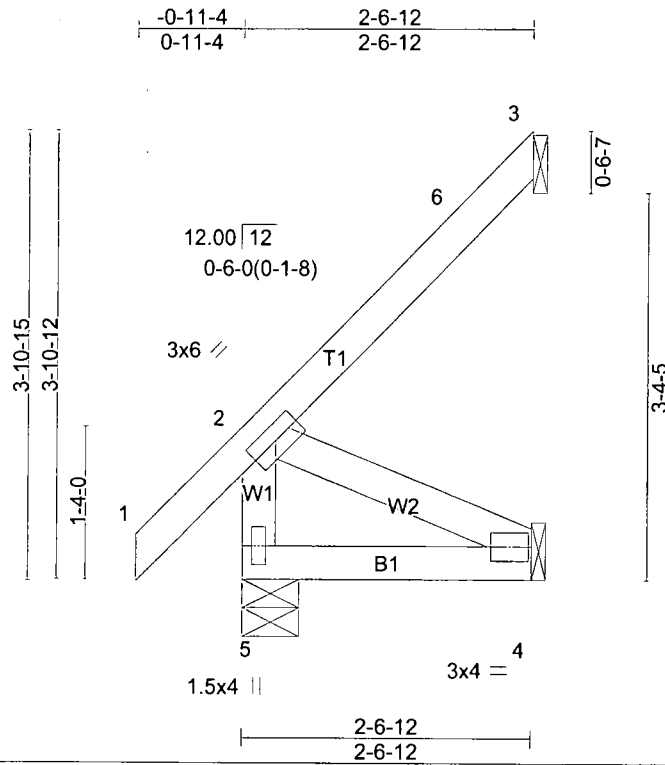
Max Horz 6=112(LC 9)

Max Uplift 5=-70(LC 9), 6=-3(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 90mph; TCCL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 6.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard



LOADING (psf)	SPACING	CSI	DEFL		PLATES	GRIP
TCLL 30.0	2-0-0	TC 0.12	in (loc) l/defl L/d		MT20	197/144
TCDL 7.0	Plates Increase 1.15	BC 0.06	Vert(LL) -0.00 4-5 >999 240			
BCLL 0.0	Lumber Increase 1.15	WB 0.06	Vert(TL) -0.01 4-5 >999 180			
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.00 3 n/a n/a			
	Code WISC/IRC06/TPI2002					Weight: 13 lb

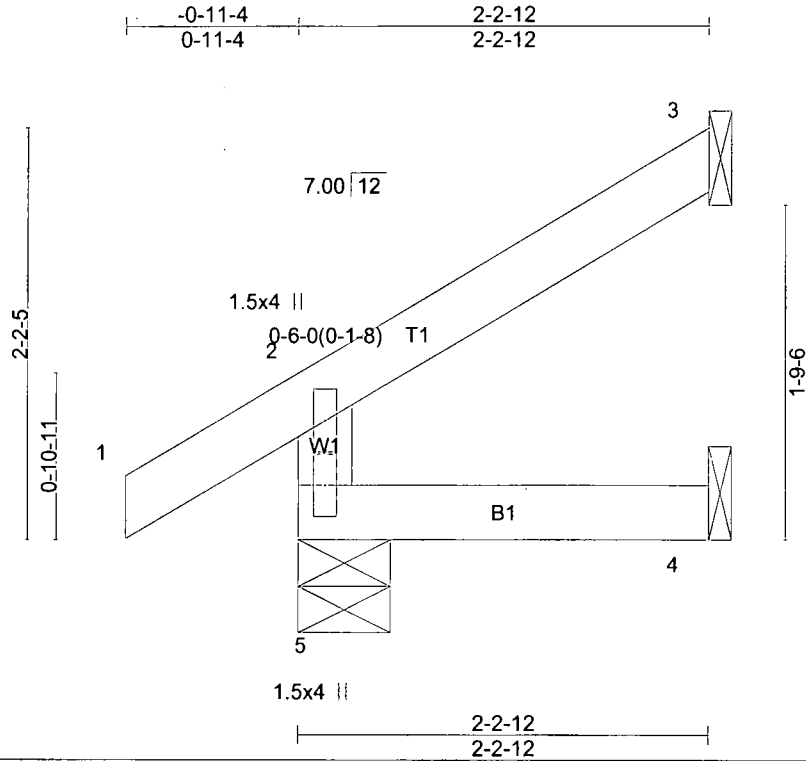
<p>LUMBER</p> <p>TOP CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>BOT CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>WEBS 2 X 4 SPF No.1 or SPF No.2 *Except* W2: 2 X 4 SPF Stud</p>	<p>BRACING</p> <p>TOP CHORD Structural wood sheathing directly applied or 2-6-12 oc purins, except end verticals.</p> <p>BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.</p> <div style="border: 1px solid black; padding: 5px; margin-top: 10px;"> <p>MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</p> </div>
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REACTIONS (lb/size) 5=210/0-6-0 (min. 0-1-8), 3=69/Mechanical, 4=24/Mechanical
 Max Horz 5=131(LC 9)
 Max Uplift 3=-45(LC 9), 4=-46(LC 9)
 Max Grav 5=210(LC 1), 3=69(LC 1), 4=48(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES**
- 1) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) -0-11-4 to 2-0-12, Interior(1) 2-0-12 to 2-6-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) Refer to girder(s) for truss to truss connections.
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.
 - 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



Scale = 1:12.5

LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 30.0	Plates Increase 1.15	TC 0.11	Vert(LL)	-0.00	5	>999	240	MT20	197/144
TCDL 7.0	Lumber Increase 1.15	BC 0.05	Vert(TL)	-0.00	4-5	>999	180		
BCLL 0.0	Rep Stress Incr YES	WB 0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002	(Matrix)							
									Weight: 7 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SPF No.1 or SPF No.2	TOP CHORD Structural wood sheathing directly applied or 2-2-12 oc purlins, except end verticals.
BOT CHORD 2 X 4 SPF No.1 or SPF No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SPF No.1 or SPF No.2	

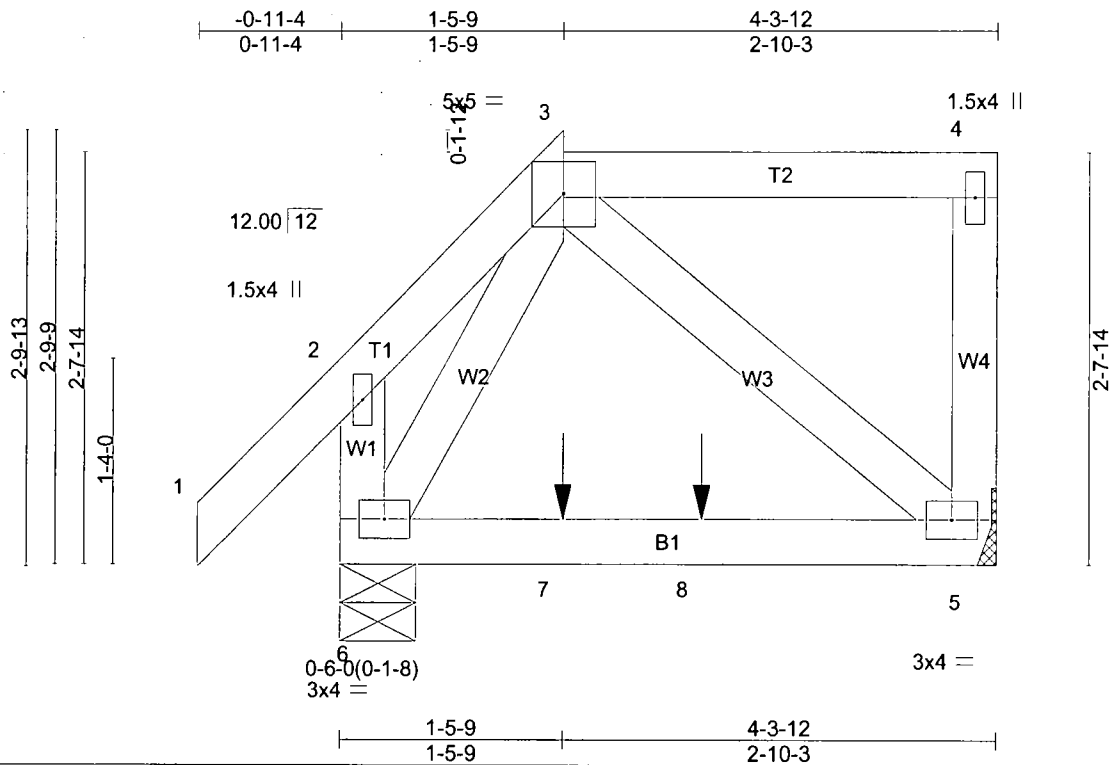
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=197/0-6-0 (min. 0-1-8), 3=56/Mechanical, 4=17/Mechanical
 Max Horz 5=70(LC 9)
 Max Uplift 5=-40(LC 9), 3=-32(LC 9)
 Max Grav 5=197(LC 1), 3=56(LC 1), 4=37(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES**
- 1) Wind: ASCE 7-05; 90mph; TCCL=4.2psf; BCCL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) Refer to girder(s) for truss to truss connections.
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.
 - 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 30.0	2-0-0	TC 0.18	in (loc) l/defl L/d	MT20	197/144
TCDL 7.0	Plates Increase 1.15	BC 0.14	Vert(LL) -0.01 5-6 >999 240		
BCLL 0.0	Lumber Increase 1.15	WB 0.02	Vert(TL) -0.04 5-6 >999 180		
BCDL 10.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.00 5 n/a n/a		
	Code WISC/IRC06/TPI2002			Weight: 22 lb	

LUMBER TOP CHORD 2 X 4 SPF No.1 or SPF No.2 BOT CHORD 2 X 4 SPF 2400F 2.0E WEBS 2 X 4 SPF Stud *Except* W1: 2 X 4 SPF No.1 or SPF No.2	BRACING TOP CHORD Structural wood sheathing directly applied or 4-3-12 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. JOINTS 1 Brace at Jt(s): 4
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MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=206/Mechanical, 6=313/0-6-0 (min. 0-1-8)
 Max Horz 6=94(LC 7)
 Max Uplift 5=-61(LC 4), 6=-54(LC 7)

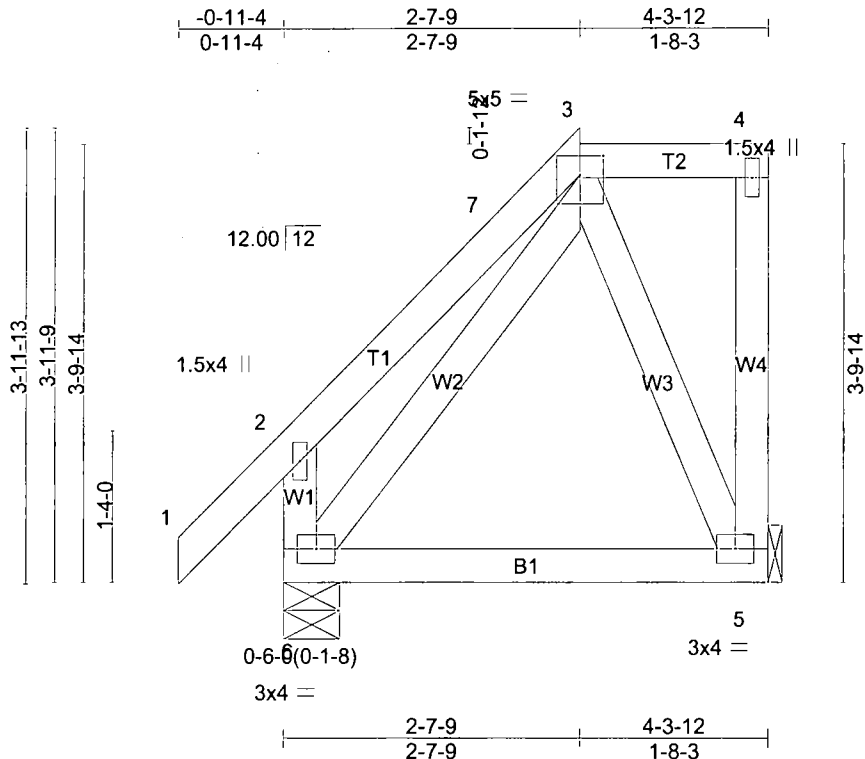
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 6.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 26 lb down and 6 lb up at 1-5-9, and 34 lb down and 8 lb up at 2-4-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (pf)
 Vert: 1-2=-74, 2-3=-74, 3-4=-74, 5-6=-20
 Concentrated Loads (lb)
 Vert: 7=-26(F) 8=-34(F)



Scale = 1:20.5

Plate Offsets (X,Y): [3:0-2-8,0-2-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d	PLATES	GRIP
TCLL 30.0	Plates Increase	1.15	TC 0.15	Vert(LL) -0.02	5-6 >999 240	MT20	197/144
TCDL 7.0	Lumber Increase	1.15	BC 0.18	Vert(TL) -0.04	5-6 >999 180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.08	Horz(TL) -0.00	5 n/a n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002		(Matrix)				
						Weight: 26 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SPF No.1 or SPF No.2	TOP CHORD Structural wood sheathing directly applied or 4-3-12 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.
BOT CHORD 2 X 4 SPF No.1 or SPF No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SPF Stud *Except*	JOINTS 1 Brace at Jt(s): 4
W1: 2 X 4 SPF No.1 or SPF No.2	

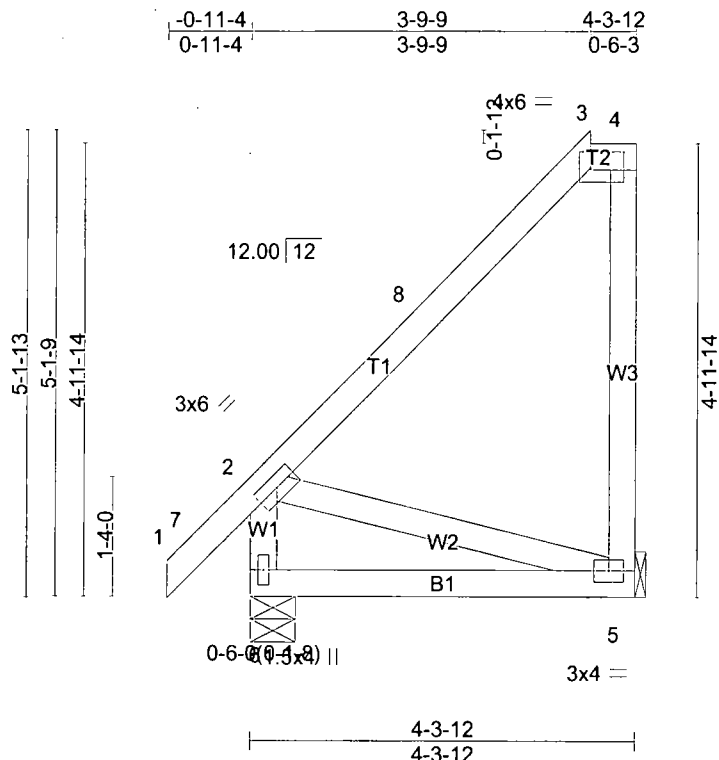
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=178/Mechanical, 6=280/0-6-0 (min. 0-1-8)
 Max Horz 6=132(LC 9)
 Max Uplift 5=-72(LC 9), 6=-22(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-6=-190/262

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) -0-11-4 to 2-0-12, Interior(1) 2-0-12 to 2-7-9, Exterior(2) 2-7-9 to 4-2-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 6.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard



Scale = 1:25.9

Plate Offsets (X,Y): [3:0-4-8,0-2-2]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d	PLATES	GRIP
TCLL 30.0	Plates Increase	1.15	TC 0.24	Vert(LL) -0.01	5-6 >999 240	MT20	197/144
TCDL 7.0	Lumber Increase	1.15	BC 0.10	Vert(TL) -0.02	5-6 >999 180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.08	Horz(TL) -0.00	5 n/a n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002		(Matrix)				
						Weight: 24 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SPF No.1 or SPF No.2	TOP CHORD Structural wood sheathing directly applied or 4-3-12 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.
BOT CHORD 2 X 4 SPF No.1 or SPF No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SPF Stud *Except*	JOINTS 1 Brace at Jt(s): 4
W1: 2 X 4 SPF No.1 or SPF No.2	

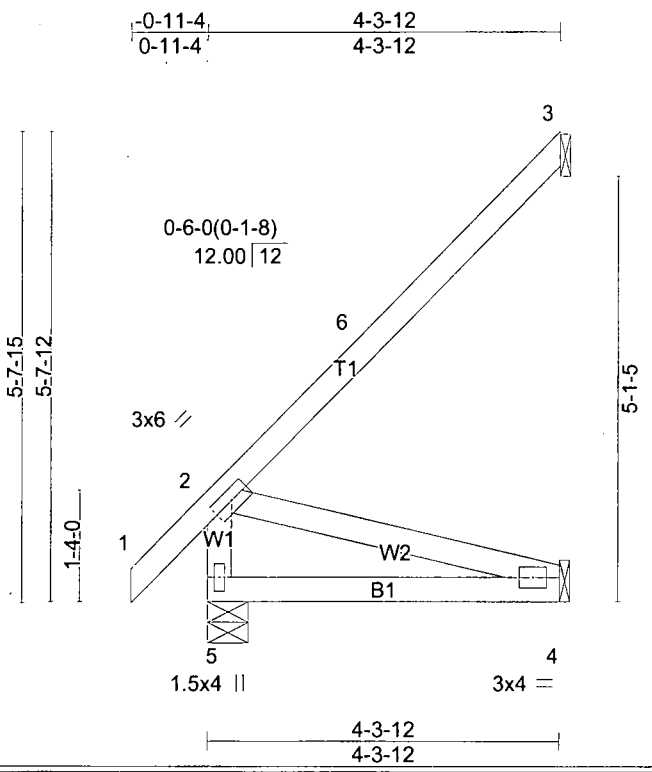
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=178/Mechanical, 6=280/0-6-0 (min. 0-1-8)
 Max Horz 6=170(LC 9)
 Max Uplift 5=-111(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 BOT CHORD 5-6=-256/93
 WEBS 2-5=-90/256

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) -0-11-4 to 2-0-12, Interior(1) 2-0-12 to 3-9-9, Exterior(2) 3-9-9 to 4-2-0 zone; cantilever left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 5=111.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard



LOADING (psf)	SPACING	CSI	DEFL		PLATES	GRIP
TCLL 30.0	2-0-0	TC 0.30	in (loc) l/defl L/d		MT20	197/144
TCDL 7.0	Plates Increase 1.15	BC 0.19	Vert(LL) -0.02 4-5 >999 240			
BCLL 0.0	Lumber Increase 1.15	WB 0.08	Vert(TL) -0.05 4-5 >999 180			
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.01 3 n/a n/a			
	Code WISC/IRC06/TPI2002					Weight: 19 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SPF No.1 or SPF No.2	TOP CHORD Structural wood sheathing directly applied or 4-3-12 oc purlins, except end verticals.
BOT CHORD 2 X 4 SPF No.1 or SPF No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SPF No.1 or SPF No.2 *Except* W2: 2 X 4 SPF Stud	

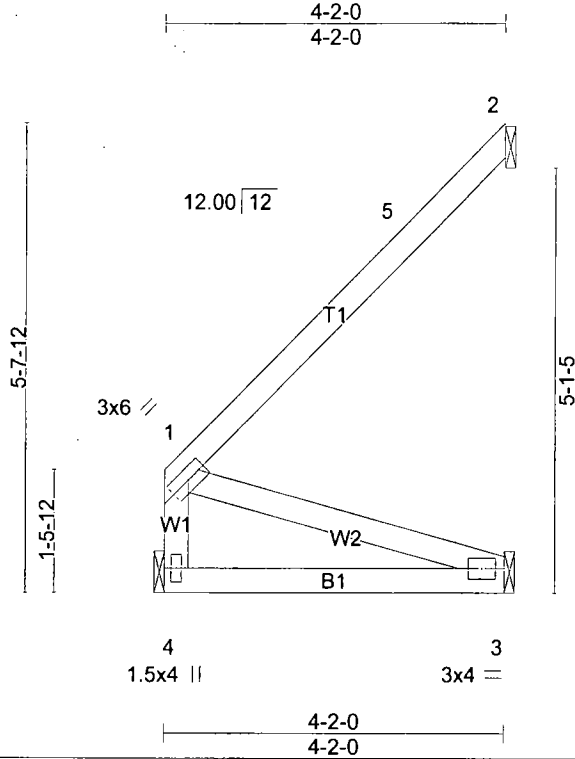
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=284/0-6-0 (min. 0-1-8), 3=141/Mechanical, 4=42/Mechanical
 Max Horz 5=188(LC 9)
 Max Uplift 3=104(LC 9), 4=-26(LC 9)
 Max Grav 5=284(LC 1), 3=141(LC 1), 4=83(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 WEBS 2-4=-46/255

- NOTES**
- 1) Wind: ASCE 7-05; 90mph; TC DL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) -0-11-4 to 2-0-12, Interior(1) 2-0-12 to 4-3-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) Refer to girder(s) for truss to truss connections.
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 3=104.
 - 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 30.0	2-0-0	TC 0.33	Vert(LL)	-0.02	3-4	>999	240	MT20
TCDL 7.0	Plates Increase 1.15	BC 0.18	Vert(TL)	-0.04	3-4	>999	180	197/144
BCLL 0.0	Lumber Increase 1.15	WB 0.06	Horz(TL)	-0.00	2	n/a	n/a	
BCDL 10.0	Rep Stress Incr YES	(Matrix)						
	Code WISC/IRC06/TPI2002							Weight: 18 lb

<p>LUMBER</p> <p>TOP CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>BOT CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>WEBS 2 X 4 SPF Stud</p>	<p>BRACING</p> <p>TOP CHORD Structural wood sheathing directly applied or 4-2-0 oc purlins, except end verticals.</p> <p>BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.</p>
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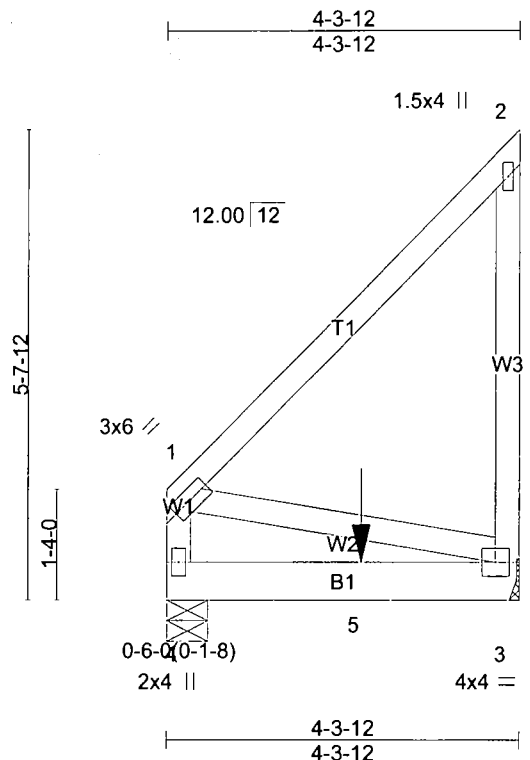
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 4=187/Mechanical, 2=146/Mechanical, 3=40/Mechanical
 Max Horz 4=130(LC 9)
 Max Uplift 2=113(LC 9), 3=17(LC 9)
 Max Grav 4=187(LC 1), 2=146(LC 1), 3=80(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES**
- 1) Wind: ASCE 7-05; 90mph; TC DL=4.2psf; BC DL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 4-1-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) Refer to girder(s) for truss to truss connections.
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3 except (jt=lb) 2=113.
 - 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



LOADING (psf)	SPACING 2-0-0	CSI	DEFL	PLATES	GRIP
TCLL 30.0	Plates Increase 1.15	TC 0.39	in (loc) l/defl L/d	MT20	197/144
TCDL 7.0	Lumber Increase 1.15	BC 0.39	Vert(LL) -0.02 3-4 >999 240		
BCLL 0.0	Rep Stress Incr NO	WB 0.04	Vert(TL) -0.05 3-4 >999 180		
BCDL 10.0	Code WISC/IRC06/TPI2002	(Matrix)	Horz(TL) -0.00 3 n/a n/a		
				Weight: 28 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SPF No.1 or SPF No.2	TOP CHORD Structural wood sheathing directly applied or 4-3-12 oc purlins, except end verticals.
BOT CHORD 2 X 6 SYP 2400F 2.0E	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SPF Stud *Except* W1: 2 X 4 SPF No.1 or SPF No.2	
	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=549/Mechanical, 4=478/0-6-0 (min. 0-1-8)
 Max Horz 4=132(LC 7)
 Max Uplift 3=-180(LC 7), 4=-18(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

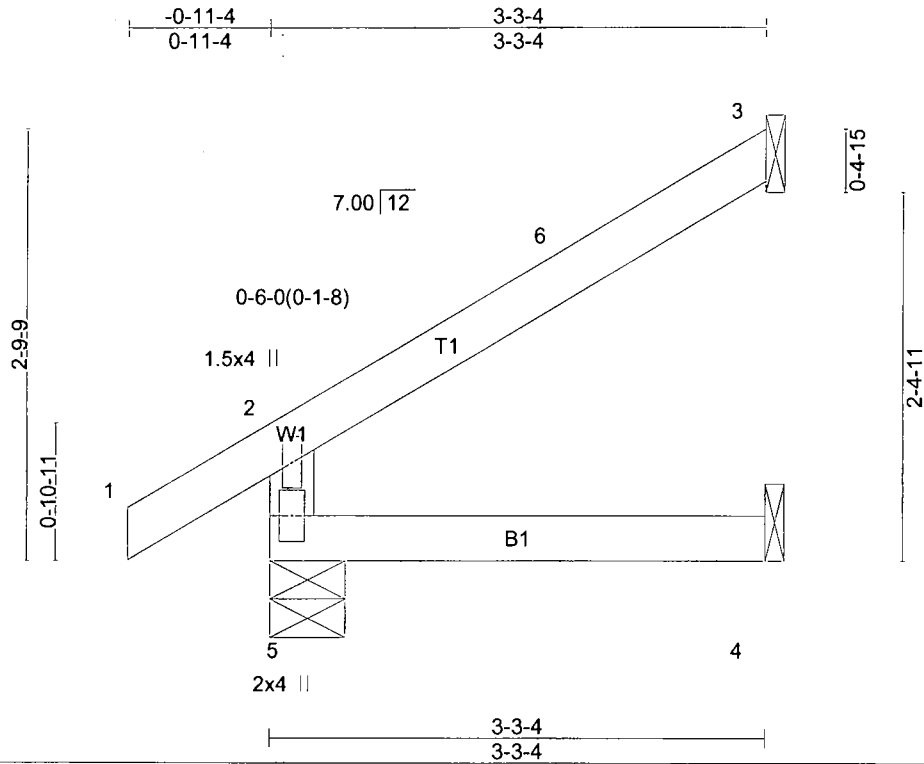
- NOTES**
- 1) Wind: ASCE 7-05; 90mph; TC DL=4.2psf; BC DL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) Refer to girder(s) for truss to truss connections.
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 3=180.
 - 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 649 lb down and 99 lb up at 2-4-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)
 Vert: 1-2=-74, 3-4=-20

Concentrated Loads (lb)
 Vert: 5=-649(F)



Scale = 1:15.3

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	PLATES	GRIP
TCLL 30.0	Plates Increase	1.15	TC 0.14	in (loc) l/defl L/d	MT20	197/144
TCDL 7.0	Lumber Increase	1.15	BC 0.10	Vert(LL) 0.01 4-5 >999 240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Vert(TL) -0.01 4-5 >999 180		
BCDL 10.0	Code WISC/IRC06/TPI2002		(Matrix)	Horz(TL) -0.01 3 n/a n/a		
						Weight: 10 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SPF No.1 or SPF No.2	TOP CHORD Structural wood sheathing directly applied or 3-3-4 oc purlins, except end verticals.
BOT CHORD 2 X 4 SPF No.1 or SPF No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SPF No.1 or SPF No.2	

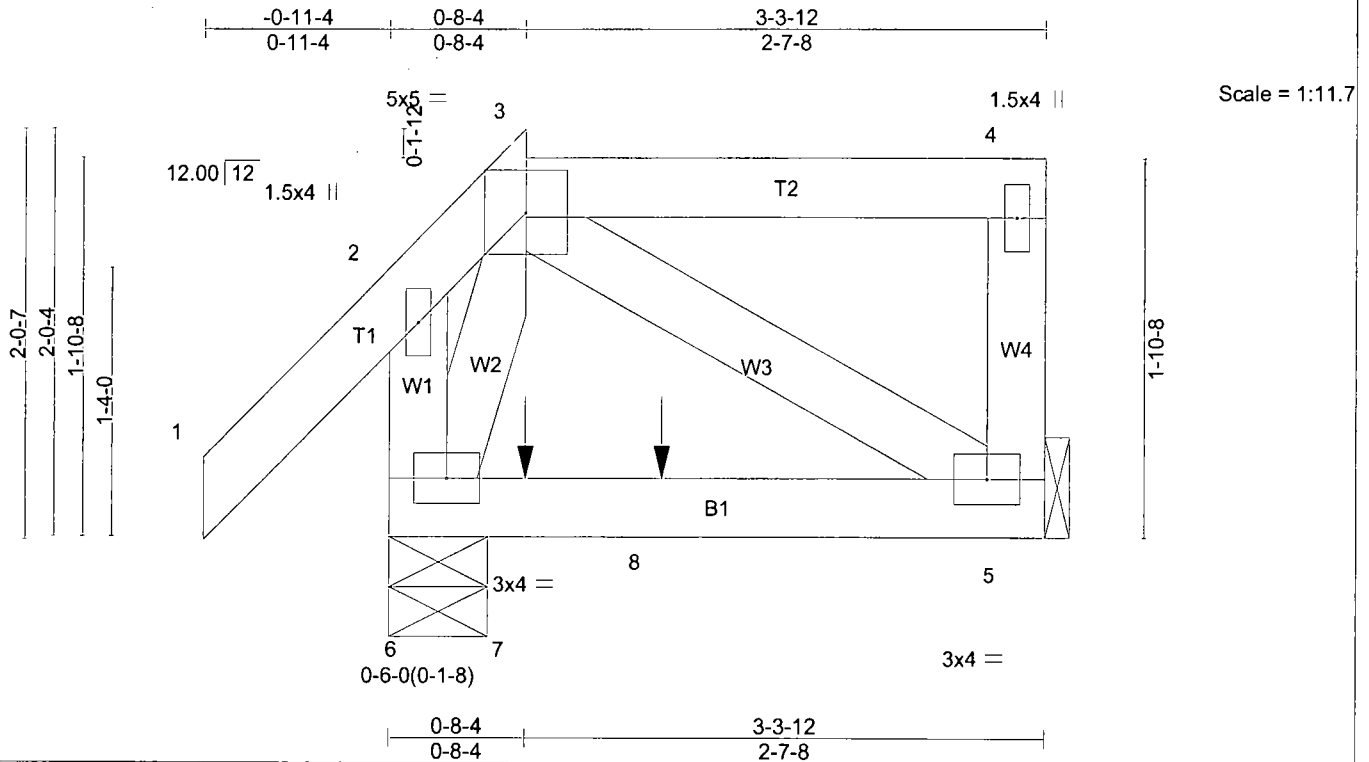
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=238/0-6-0 (min. 0-1-8), 3=96/Mechanical, 4=34/Mechanical
 Max Horz 5=90(LC 9)
 Max Uplift 5=-38(LC 9), 3=-50(LC 9)
 Max Grav 5=238(LC 1), 3=96(LC 1), 4=57(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES**
- 1) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) -0-11-4 to 2-0-12, Interior(1) 2-0-12 to 3-2-8 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) Refer to girder(s) for truss to truss connections.
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.
 - 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



LOADING (psf)	SPACING	CSI	DEFL		PLATES GRIP
TCLL 30.0	Plates Increase 1.15	TC 0.15	in (loc) l/defl L/d		MT20 197/144
TCDL 7.0	Lumber Increase 1.15	BC 0.12	Vert(LL) -0.01 5-6 >999 240		
BCLL 0.0	Rep Stress Incr NO	WB 0.02	Vert(TL) -0.01 5-6 >999 180		
BCDL 10.0	Code WISC/IRC06/TPI2002	(Matrix)	Horz(TL) -0.00 5 n/a n/a		Weight: 17 lb

<p>LUMBER</p> <p>TOP CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>BOT CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>WEBS 2 X 4 SPF Stud *Except*</p> <p style="padding-left: 20px;">W1: 2 X 4 SPF No.1 or SPF No.2</p>	<p>BRACING</p> <p>TOP CHORD Structural wood sheathing directly applied or 3-3-12 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.</p> <p>BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.</p> <p>JOINTS 1 Brace at Jt(s): 4</p> <div style="border: 1px solid black; padding: 5px; margin-top: 10px;"> <p>MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</p> </div>
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REACTIONS (lb/size) 5=135/Mechanical, 6=253/0-6-0 (min. 0-1-8)

Max Horz 6=69(LC 7)

Max Uplift 5=-41(LC 4), 6=-56(LC 7)

Max Grav 5=144(LC 12), 6=253(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 90mph; TC DL=4.2psf; BC DL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 6.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 - 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 11 lb down and 3 lb up at 0-8-4, and 13 lb down and 3 lb up at 1-4-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-2=-74, 2-3=-74, 3-4=-74, 5-6=-20

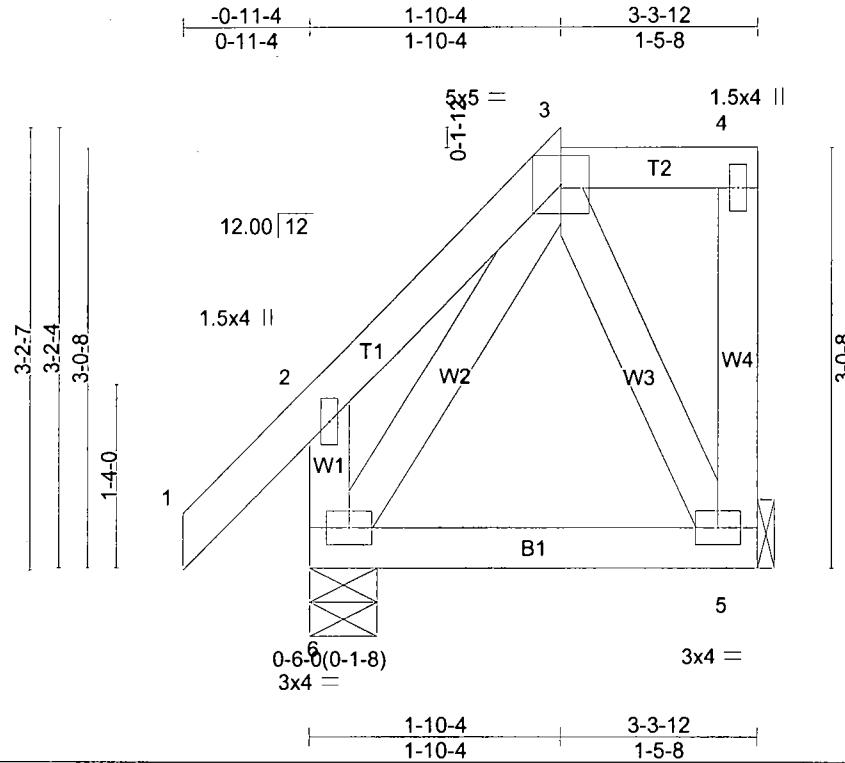
Concentrated Loads (lb)

Vert: 7=-11(F) 8=-13(F)

Job J1000359	Truss J12	Truss Type Half Hip Truss	Qty 2	Ply 1	KINGS WAY - PIERRI RES. #5302
					Job Reference (optional)

Richco Structures, Sheboygan Falls, WI, Heather Kraning

7.220 s Dec 29 2009 MiTek Industries, Inc. Wed Jan 27 08:39:18 2010 Page 1



Scale = 1:17.1

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 30.0	2-0-0	TC 0.15	in (loc) l/defl L/d	MT20	197/144
TCDL 7.0	Plates Increase 1.15	BC 0.10	Vert(LL) -0.00 5-6 >999 240		
BCLL 0.0	Lumber Increase 1.15	WB 0.05	Vert(TL) -0.01 5-6 >999 180		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.00 5 n/a n/a		
	Code WISC/IRC06/TPI2002			Weight: 21 lb	

LUMBER
 TOP CHORD 2 X 4 SPF No.1 or SPF No.2
 BOT CHORD 2 X 4 SPF No.1 or SPF No.2
 WEBS 2 X 4 SPF Stud *Except*
 W1: 2 X 4 SPF No.1 or SPF No.2

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-3-12 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 JOINTS 1 Brace at Jt(s): 4

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

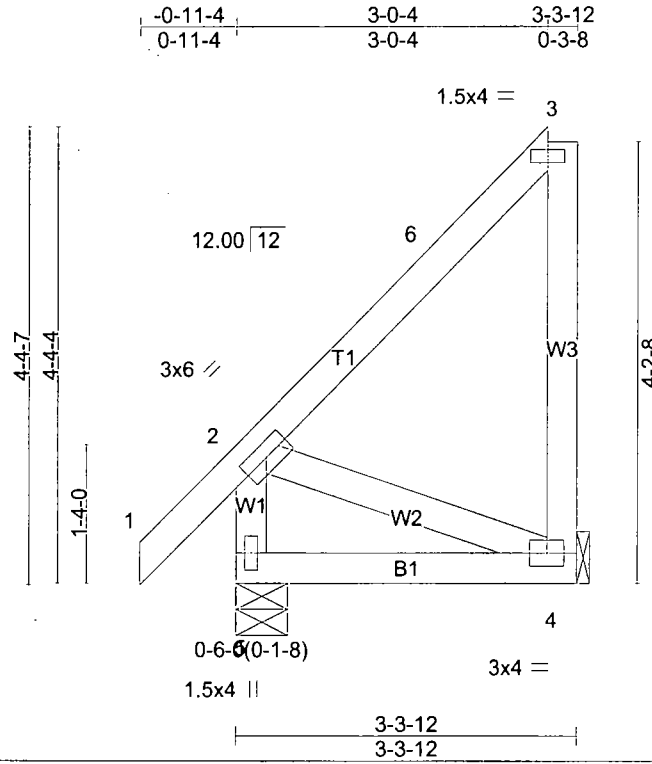
REACTIONS (lb/size) 5=128/Mechanical, 6=237/0-6-0 (min. 0-1-8)
 Max Horz 6=107(LC 9)
 Max Uplift 5=-55(LC 9), 6=-27(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) zone; cantilever left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 6.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
- 8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d	PLATES	GRIP
TCLL 30.0	Plates Increase	1.15	TC 0.14	Vert(LL) -0.00	4-5 >999 240	MT20	197/144
TCDL 7.0	Lumber Increase	1.15	BC 0.10	Vert(TL) -0.01	4-5 >999 180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.07	Horz(TL) -0.00	4 n/a n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002		(Matrix)				Weight: 19 lb

LUMBER TOP CHORD 2 X 4 SPF No.1 or SPF No.2 BOT CHORD 2 X 4 SPF No.1 or SPF No.2 WEBS 2 X 4 SPF Stud *Except* W1: 2 X 4 SPF No.1 or SPF No.2	BRACING TOP CHORD Structural wood sheathing directly applied or 3-3-12 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. <div style="border: 1px solid black; padding: 5px; margin-top: 10px;"> MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide. </div>
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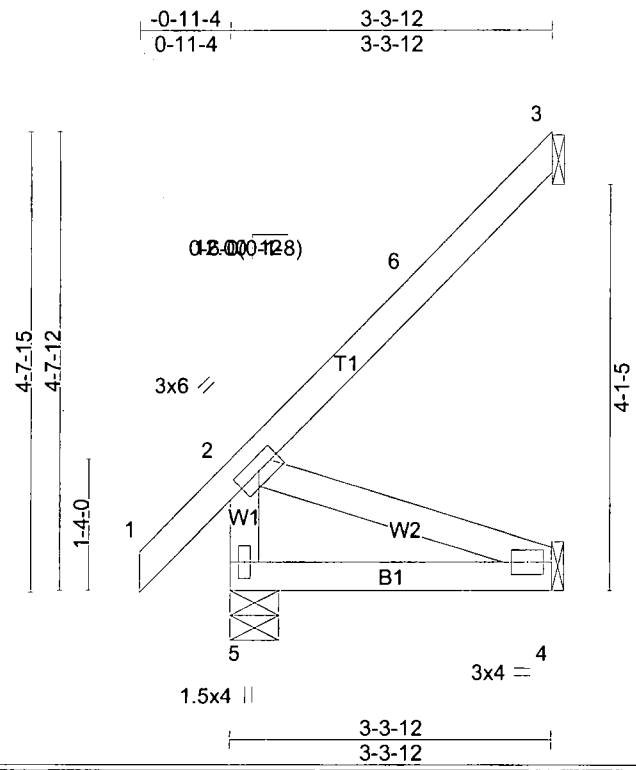
REACTIONS (lb/size) 4=128/Mechanical, 5=237/0-6-0 (min. 0-1-8)
 Max Horz 5=152(LC 9)
 Max Uplift 4=107(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- 1) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) -0-11-4 to 2-0-12, Interior(1) 2-0-12 to 3-2-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=107.
- 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



LOADING (psf)	SPACING	CSI	DEFL		PLATES	GRIP
TCLL 30.0	2-0-0	TC 0.16	in (loc) l/defl L/d		MT20	197/144
TCDL 7.0	Plates Increase 1.15	BC 0.11	Vert(LL) -0.01 4-5 >999 240			
BCLL 0.0	Lumber Increase 1.15	WB 0.07	Vert(TL) -0.02 4-5 >999 180			
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.00 3 n/a n/a			
	Code WISC/IRC06/TPI2002					Weight: 16 lb

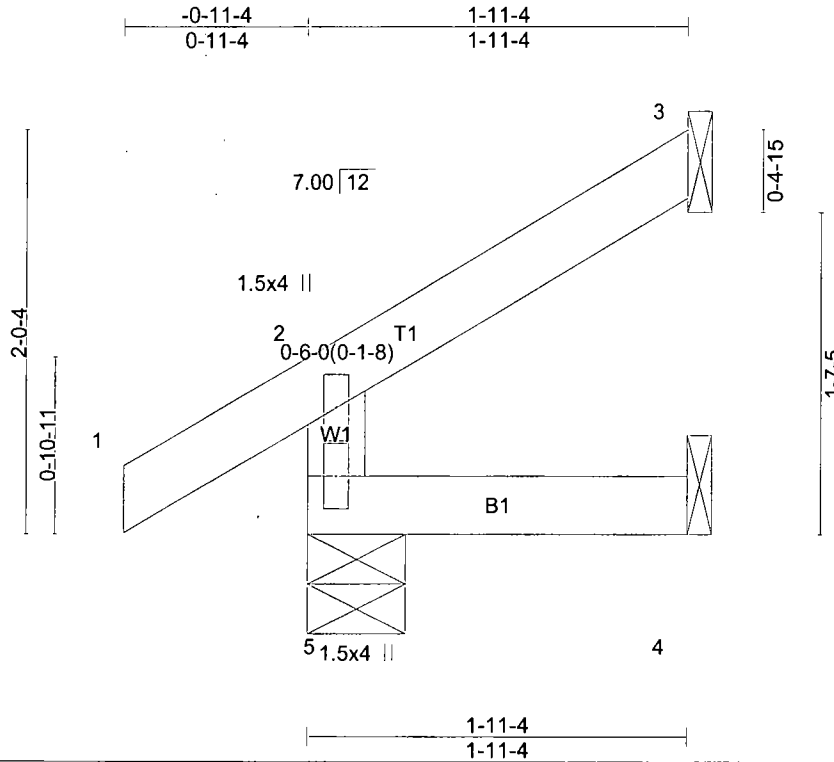
<p>LUMBER</p> <p>TOP CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>BOT CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>WEBS 2 X 4 SPF No.1 or SPF No.2 *Except* W2: 2 X 4 SPF Stud</p>	<p>BRACING</p> <p>TOP CHORD Structural wood sheathing directly applied or 3-3-12 oc purlins, except end verticals.</p> <p>BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.</p> <div style="border: 1px solid black; padding: 5px; margin-top: 10px;"> <p>MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</p> </div>
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REACTIONS (lb/size) 5=241/0-6-0 (min. 0-1-8), 3=101/Mechanical, 4=32/Mechanical
 Max Horz 5=155(LC 9)
 Max Uplift 3=-72(LC 9), 4=-36(LC 9)
 Max Grav 5=241(LC 1), 3=101(LC 1), 4=63(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES**
- 1) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) -0-11-4 to 2-0-12, Interior(1) 2-0-12 to 3-3-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) Refer to girder(s) for truss to truss connections.
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.
 - 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



Scale = 1:11.8

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	PLATES	GRIP
TCLL 30.0	Plates Increase	1.15	TC 0.11	in (loc) l/defl L/d	MT20	197/144
TCDL 7.0	Lumber Increase	1.15	BC 0.04	Vert(LL) -0.00 5 >999 240		
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Vert(TL) -0.00 4-5 >999 180		
BCDL 10.0	Code WISC/IRC06/TPI2002		(Matrix)	Horz(TL) -0.00 3 n/a n/a		
					Weight: 7 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SPF No.1 or SPF No.2	TOP CHORD Structural wood sheathing directly applied or 1-11-4 oc purlins, except end verticals.
BOT CHORD 2 X 4 SPF No.1 or SPF No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SPF No.1 or SPF No.2	

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=188/0-6-0 (min. 0-1-8), 4=13/Mechanical, 3=46/Mechanical
 Max Horz 5=65(LC 9)
 Max Uplift 5=-40(LC 9), 3=-27(LC 9)
 Max Grav 5=188(LC 1), 4=32(LC 2), 3=46(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES**
- 1) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) Refer to girder(s) for truss to truss connections.
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.
 - 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

-0-11-4 11-9-0 23-6-0 24-5-4
0-11-4 11-9-0 11-9-0 0-11-4

4x4 = Scale = 1:73.9

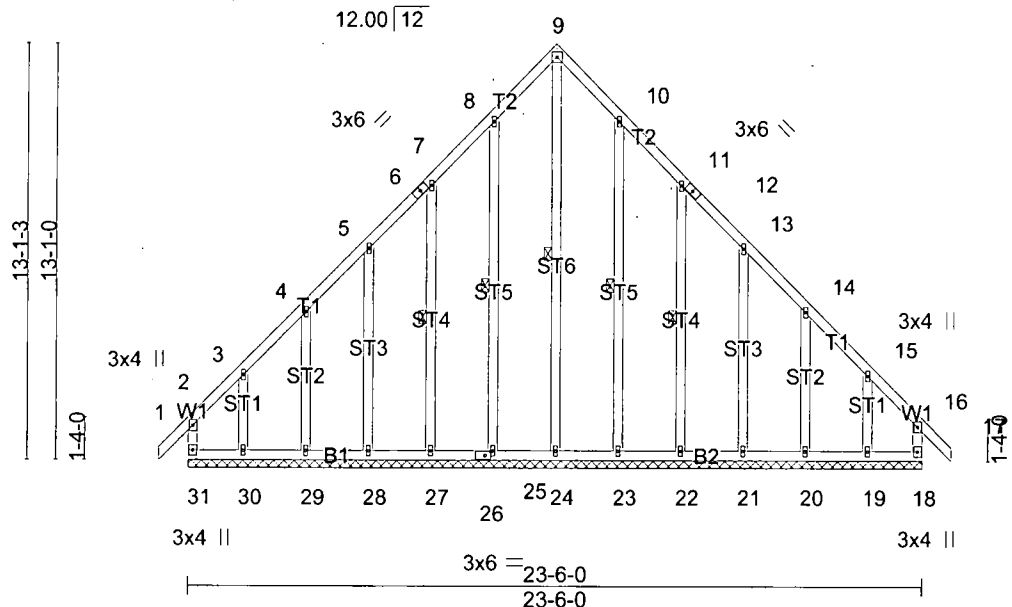


Plate Offsets (X,Y): [26:0-2-4,0-1-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 30.0	Plates Increase	1.15	TC 0.20	Vert(LL)	-0.01	17	n/r	MT20	197/144
TCDL 7.0	Lumber Increase	1.15	BC 0.17	Vert(TL)	-0.01	17	n/r		
BCLL 0.0	Rep Stress Incr	YES	WB 0.44	Horz(TL)	0.01	18	n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002		(Matrix)						
								Weight: 159 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SPF No.1 or SPF No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2 X 4 SPF No.1 or SPF No.2	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2 X 4 SPF No.1 or SPF No.2	WEBS 1 Row at midpt 9-24, 8-25, 7-27, 10-23, 11-22
OTHERS 2 X 4 SPF Stud *Except* ST6,ST5,ST4: 2 X 4 SPF No.1 or SPF No.2	

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 23-6-0.
 (lb) - Max Horz 31=-299(LC 7)
 Max Uplift All uplift 100 lb or less at joint(s) 25, 28, 29, 23, 21, 20 except 31=-281(LC 7), 18=-246(LC 8), 27=-103(LC 9), 30=-188(LC 6), 22=-103(LC 9), 19=-176(LC 5)
 Max Grav All reactions 250 lb or less at joint(s) 18, 25, 27, 28, 29, 23, 22, 21, 20, 19 except 31=270(LC 8), 24=529(LC 9), 30=262(LC 7)

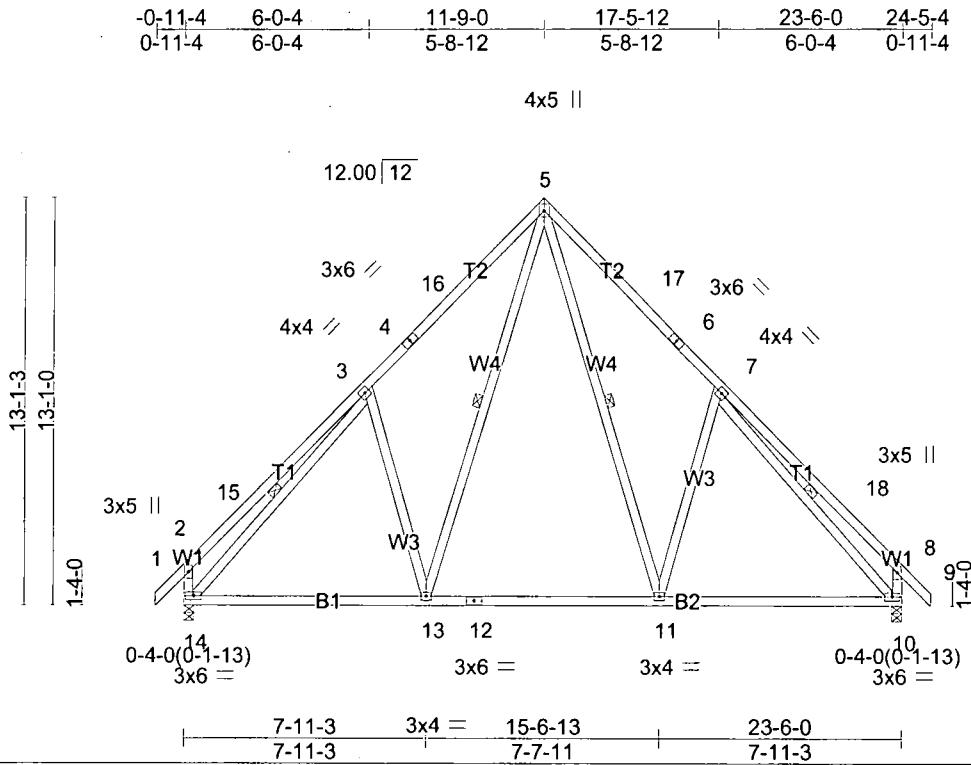
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-250/280, 5-6=-113/269, 6-7=-102/275, 7-8=-83/396, 8-9=-59/486, 9-10=-59/486, 10-11=-56/396, 11-12=-76/275, 12-13=-86/269
 WEBS 9-24=-597/17

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Corner(3) -0-11-4 to 2-0-12, Exterior(2) 2-0-12 to 11-9-0, Corner(3) 11-9-0 to 14-9-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
 - 4) All plates are 1.5x4 MT20 unless otherwise indicated.
 - 5) Gable requires continuous bottom chord bearing.
 - 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - 7) Gable studs spaced at 2-0-0 oc.
 - 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 25, 28, 29, 23, 21, 20 except (jt=lb) 31=281, 18=246, 27=103, 30=188, 22=103, 19=176.
 - 10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job J1000359	Truss T02	Truss Type Common Truss	Qty 2	Ply 1	KINGS WAY - PIERRI RES. #5302
					Job Reference (optional)

Richco Structures, Sheboygan Falls, WI, Heather Kranning 7.220 s Dec 29 2009 MiTek Industries, Inc. Wed Jan 27 08:39:26 2010 Page 1



Scale = 1:75.5

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 30.0	2-0-0	TC 0.40	in (loc) l/defl L/d	MT20	197/144
TCDL 7.0	Plates Increase 1.15	BC 0.41	Vert(LL) -0.09 10-11 >999 240		
BCLL 0.0	Lumber Increase 1.15	WB 0.35	Vert(TL) -0.22 10-11 >999 180		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.03 10 n/a n/a		
	Code WISC/IRC06/TPI2002			Weight: 136 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SPF No.1 or SPF No.2	TOP CHORD Structural wood sheathing directly applied or 5-3-0 oc purlins, except end verticals.
BOT CHORD 2 X 4 SPF No.1 or SPF No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 SPF No.1 or SPF No.2 *Except* W3: 2 X 4 SPF Stud	WEBS 1 Row at midpt 5-11, 5-13, 3-14, 7-10

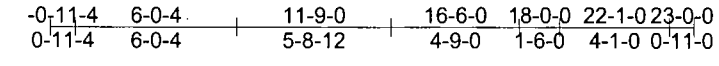
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 14=1171/0-4-0 (min. 0-1-13), 10=1171/0-4-0 (min. 0-1-13)
 Max Horz 14=-299(LC 7)
 Max Uplift 14=-188(LC 9), 10=-188(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-15=-408/275, 3-15=-177/298, 3-4=-1056/369, 4-16=-911/382, 5-16=-891/399,
 5-17=-891/399, 6-17=-911/382, 6-7=-1056/369, 7-18=-177/298, 8-18=-408/275,
 2-14=-473/332, 8-10=-473/332
BOT CHORD 13-14=-66/708, 12-13=-25/499, 11-12=-25/499, 10-11=-15/708
WEBS 5-11=-209/488, 7-11=-320/309, 5-13=-209/488, 3-13=-320/309, 3-14=-815/0,
 7-10=-815/0

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05: 90mph; TCCL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) 0-11-4 to 2-0-12, Interior(1) 2-0-12 to 11-9-0, Exterior(2) 11-9-0 to 14-9-0 zone, cantilever left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 14=188, 10=188.
 - 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



Scale = 1:85.7

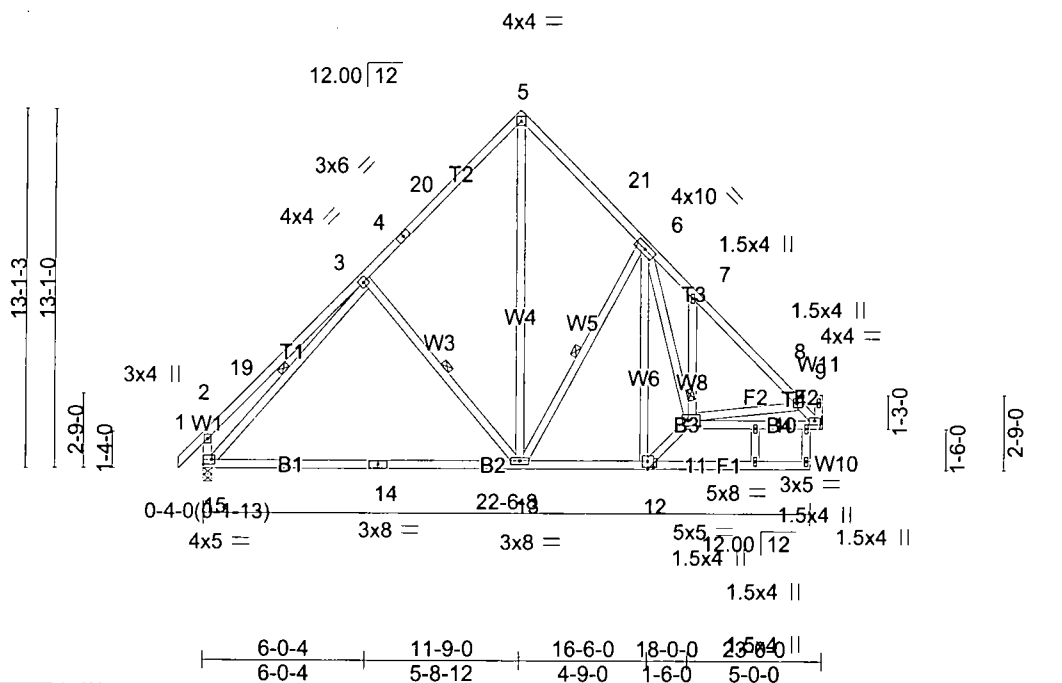


Plate Offsets (X,Y): [11:0-5-4,0-2-4], [12:0-0-12,0-2-8], [15:Edge,0-2-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL		PLATES	GRIP
TCLL 30.0	Plates Increase	1.15	TC 0.52	Vert(LL) -0.45 13-15	>604 240	MT20	197/144
TCDL 7.0	Lumber Increase	1.15	BC 1.00	Vert(TL) -1.13 13-15	>240 180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.74	Horz(TL) 0.05 10	n/a n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002		(Matrix)				Weight: 149 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SPF No.1 or SPF No.2	TOP CHORD Structural wood sheathing directly applied or 4-11-12 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 8-9.
BOT CHORD 2 X 4 SPF No.1 or SPF No.2	BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 2 X 4 SPF Stud *Except* W4,W5,W6,W1,W3,W2: 2 X 4 SPF No.1 or SPF No.2	WEBS 1 Row at midpt 6-13, 3-13, 3-15
	JOINTS 1 Brace at Jt(s): 9, 11

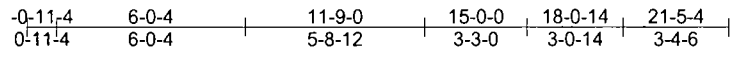
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 10=1065/Mechanical, 15=1149/0-4-0 (min. 0-1-13)
Max Horz 15=295(LC 8)
Max Uplift 10=-144(LC 9), 15=-181(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-19=-475/219, 3-19=-317/241, 3-4=-873/261, 4-20=-725/274, 5-20=-704/292,
5-21=-704/300, 6-21=-826/282, 6-7=-1193/332, 7-8=-1270/219, 2-15=-518/290
BOT CHORD 14-15=-114/671, 13-14=-114/671, 12-13=-45/623, 11-12=-54/866, 10-11=-178/884
WEBS 5-13=-258/635, 6-13=-289/223, 6-12=-578/29, 6-11=-158/883, 8-10=-1317/306,
3-13=-292/266, 3-15=-680/53

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) 0-11-4 to 2-0-12, Interior(1) 2-0-12 to 11-9-0, Exterior(2) 11-9-0 to 14-9-0, Interior(1) 22-1-0 to 22-10-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=144, 15=181.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard



6x6 = Scale = 1:73.5

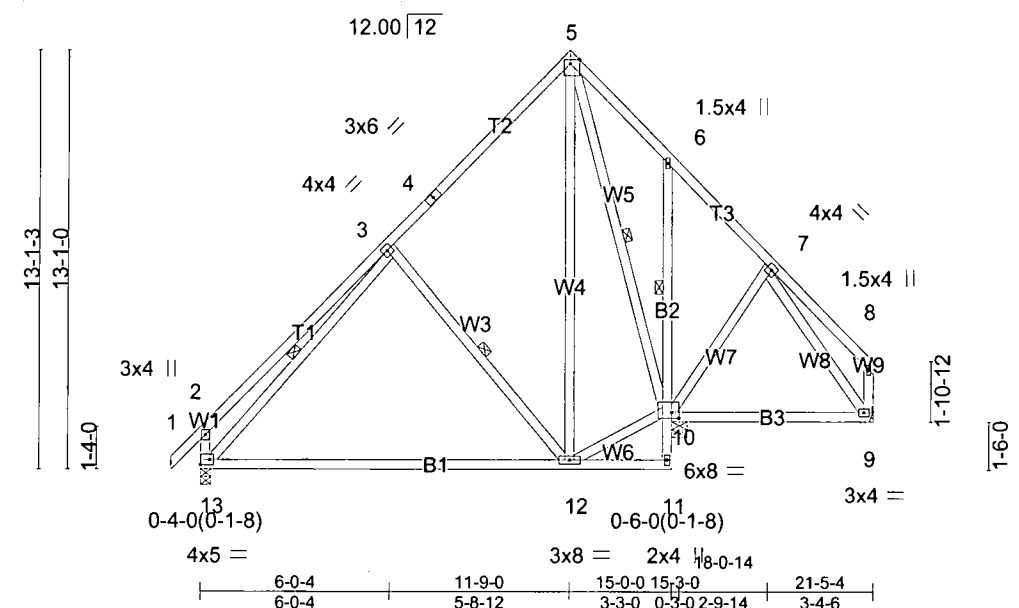


Plate Offsets (X,Y): [5:0-3-8,Edge], [10:0-2-12,0-2-4], [13:Edge,0-2-0]

LOADING (psf)	SPACING	CSI	DEFL		PLATES	GRIP
TCLL 30.0	2-0-0	TC 0.49	in (loc) l/defl L/d		MT20	197/144
TCDL 7.0	Plates Increase 1.15	BC 0.62	Vert(LL) -0.34 12-13 >521 240			
BCLL 0.0	Lumber Increase 1.15	WB 0.38	Vert(TL) -0.85 12-13 >208 180			
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 9 n/a n/a			
	Code WISC/IRC06/TPI2002					Weight: 137 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SPF 1650F 1.5E	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2 X 4 SPF 2100F 1.8E	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
WEBS 2 X 4 SPF No.1 or SPF No.2 *Except* W6,W7,W8: 2 X 4 SPF Stud	1 Row at midpt 6-10
	1 Row at midpt 5-10, 3-12, 3-13
	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 10=851/0-6-0 (min. 0-1-8), 9=400/Mechanical, 13=817/0-4-0 (min. 0-1-8)
 Max Horz 13=284(LC 6)
 Max Uplift 10=-213(LC 7), 13=-103(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-440/202, 3-4=-448/135, 4-5=-275/166, 2-13=-491/238
 BOT CHORD 12-13=-129/394, 6-10=-253/169
 WEBS 5-12=-134/404, 10-12=-49/299, 5-10=-392/10, 3-12=-313/270, 3-13=-285/128

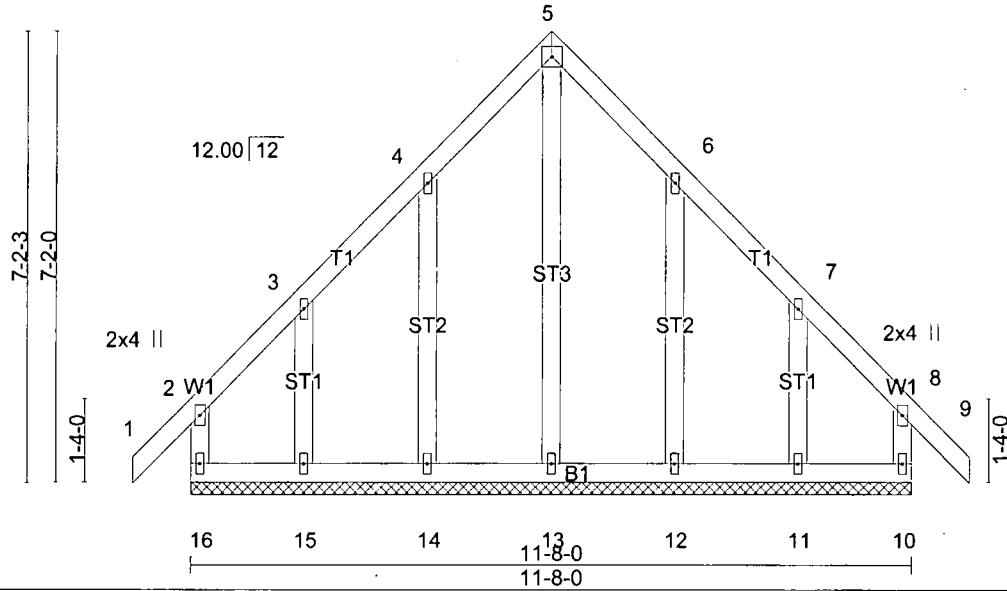
- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) Refer to girder(s) for truss to truss connections.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=213, 13=103.
 - 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



4x4 =

Scale = 1:37.4



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d	PLATES	GRIP
TCLL 30.0	Plates Increase	1.15	TC 0.12	Vert(LL) -0.01	9 n/r 120	MT20	197/144
TCDL 7.0	Lumber Increase	1.15	BC 0.10	Vert(TL) -0.01	9 n/r 90		
BCLL 0.0	Rep Stress Incr	YES	WB 0.24	Horz(TL) 0.00	10 n/a n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002		(Matrix)				Weight: 62 lb

LUMBER

TOP CHORD 2 X 4 SPF No.1 or SPF No.2
 BOT CHORD 2 X 4 SPF No.1 or SPF No.2
 WEBS 2 X 4 SPF No.1 or SPF No.2
 OTHERS 2 X 4 SPF Stud

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS

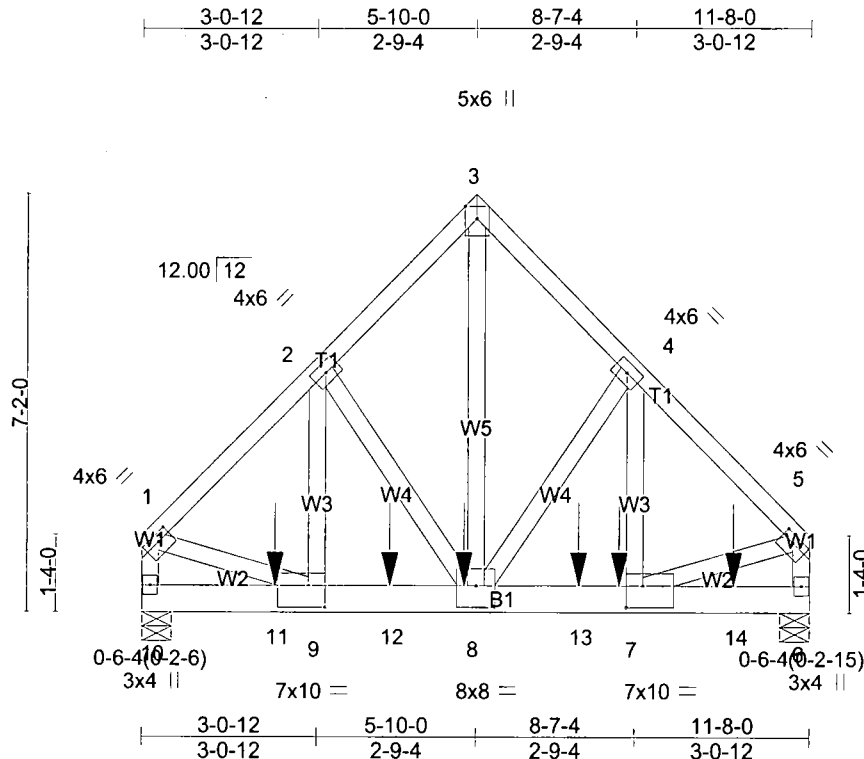
All bearings 11-8-0.
 (lb) - Max Horz 16=148(LC 8)
 Max Uplift All uplift 100 lb or less at joint(s) 14, 12 except 16=-115(LC 7), 10=-101(LC 8), 15=-120(LC 9), 11=-120(LC 9)
 Max Grav All reactions 250 lb or less at joint(s) 16, 10, 13, 14, 15, 12, 11

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 WEBS 5-13=-256/0

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Corner(3) -0-11-4 to 1-10-0, Exterior(2) 1-10-0 to 5-10-0, Corner(3) 5-10-0 to 8-10-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) All plates are 1.5x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14, 12 except (jt=lb) 16=115, 10=101, 15=120, 11=120.
- 10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



Scale = 1:40.3

Plate Offsets (X,Y): [1:0-2-12,0-1-8], [5:0-2-12,0-1-8], [7:0-3-8,0-4-8], [8:0-4-0,0-4-8], [9:0-3-8,0-4-8]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 30.0	2-0-0	TC 0.38	in (loc) l/def L/d	MT20	197/144
TCDL 7.0	Plates Increase 1.15	BC 0.37	Vert(LL) -0.06 7-8 >999 240		
BCLL 0.0	Lumber Increase 1.15	WB 0.98	Vert(TL) -0.10 7-8 >999 180		
BCDL 10.0	Rep Stress Incr NO	(Matrix)	Horz(TL) 0.01 6 n/a n/a		
	Code WISC/IRC06/TPI2002				Weight: 161 lb

LUMBER
TOP CHORD 2 X 4 SPF No.1 or SPF No.2
BOT CHORD 2 X 6 SYP 2400F 2.0E
WEBS 2 X 4 SPF Stud *Except*
W5,W1: 2 X 4 SPF No.1 or SPF No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-10-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

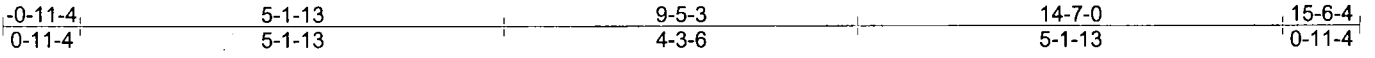
REACTIONS (lb/size) 10=5665/0-6-4 (min. 0-2-6), 6=7051/0-6-4 (min. 0-2-15)
Max Horz 10=-145(LC 5)
Max Uplift 10=-782(LC 7), 6=-974(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-5462/773, 2-3=-4584/729, 3-4=-4582/729, 4-5=-6160/870, 1-10=-5294/747, 5-6=-5965/840
BOT CHORD 9-12=-532/3819, 8-12=-532/3819, 8-13=-569/4311, 7-13=-569/4311
WEBS 3-8=-971/6192, 4-8=-1954/361, 4-7=-356/2550, 2-8=-1095/242, 2-9=-188/1349, 1-9=-532/4056, 5-7=-604/4579

- NOTES**
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-5-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05: 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=782, 6=974.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1941 lb down and 269 lb up at 2-4-0, 1941 lb down and 269 lb up at 4-4-0, 1941 lb down and 269 lb up at 5-7-8, 1941 lb down and 269 lb up at 7-7-8, and 1941 lb down and 269 lb up at 8-4-0, and 1941 lb down and 269 lb up at 10-4-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- Regular: Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-3=-74, 3-5=-74, 6-10=-20
Concentrated Loads (lb)
Vert: 8=-1941(B) 7=-1941(B) 11=-1941(B) 12=-1941(B) 13=-1941(B) 14=-1941(B)



Scale = 1:28.0

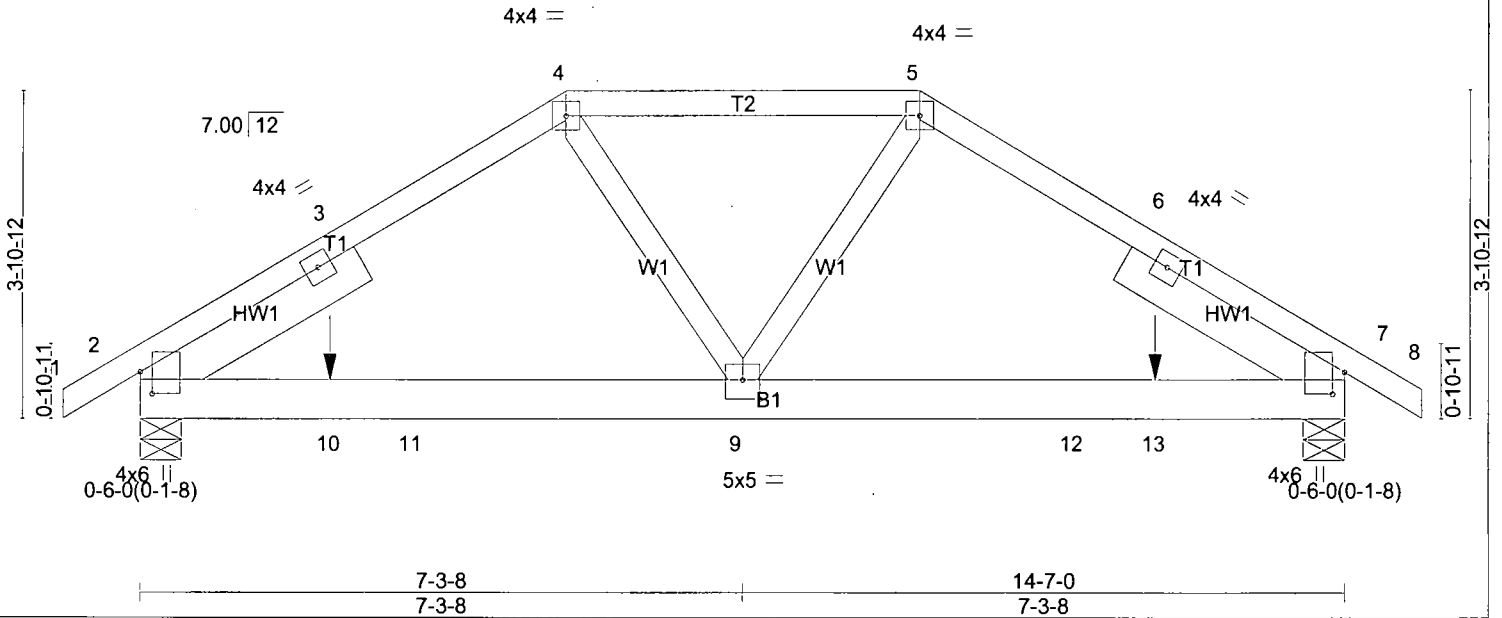


Plate Offsets (X,Y): [2:0-3-2,0-1-12], [7:0-3-2,0-1-12], [9:0-2-8,0-2-12]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 30.0	Plates Increase 1.15	TC 0.60	in (loc) l/defl L/d	MT20	197/144
TCDL 7.0	Lumber Increase 1.15	BC 0.40	Vert(LL) -0.05 7-9 >999 240		
BCLL 0.0	Rep Stress Incr NO	WB 0.23	Vert(TL) -0.11 7-9 >999 180		
BCDL 10.0	Code WISC/IRC06/TPI2002	(Matrix)	Horz(TL) 0.01 7 n/a n/a		
				Weight: 75 lb	

<p>LUMBER</p> <p>TOP CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>BOT CHORD 2 X 6 SYP 2400F 2.0E</p> <p>WEBS 2 X 4 SPF Stud</p> <p>SLIDER Left 2 X 6 SPF 1650F 1.5E 3-0-6, Right 2 X 6 SPF 1650F 1.5E 3-0-6</p>	<p>BRACING</p> <p>TOP CHORD Structural wood sheathing directly applied or 4-4-12 oc purlins, except</p> <p>BOT CHORD 2-0-0 oc purlins (4-2-12 max.): 4-5. Rigid ceiling directly applied or 10-0-0 oc bracing.</p>
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MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1241/0-6-0 (min. 0-1-8), 7=1241/0-6-0 (min. 0-1-8)
 Max Horz 2=80(LC 6)
 Max Uplift 2=-192(LC 7), 7=-192(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1461/220, 3-4=-1346/230, 4-5=-1362/226, 5-6=-1346/230, 6-7=-1461/220

BOT CHORD 2-10=-167/1110, 10-11=-167/1110, 9-11=-167/1110, 9-12=-153/1110, 12-13=-153/1110, 7-13=-153/1110

WEBS 4-9=-41/545, 5-9=-41/545

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=192, 7=192.
 - 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 7) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 - 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 226 lb down and 54 lb up at 2-3-8, and 226 lb down and 54 lb up at 12-3-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

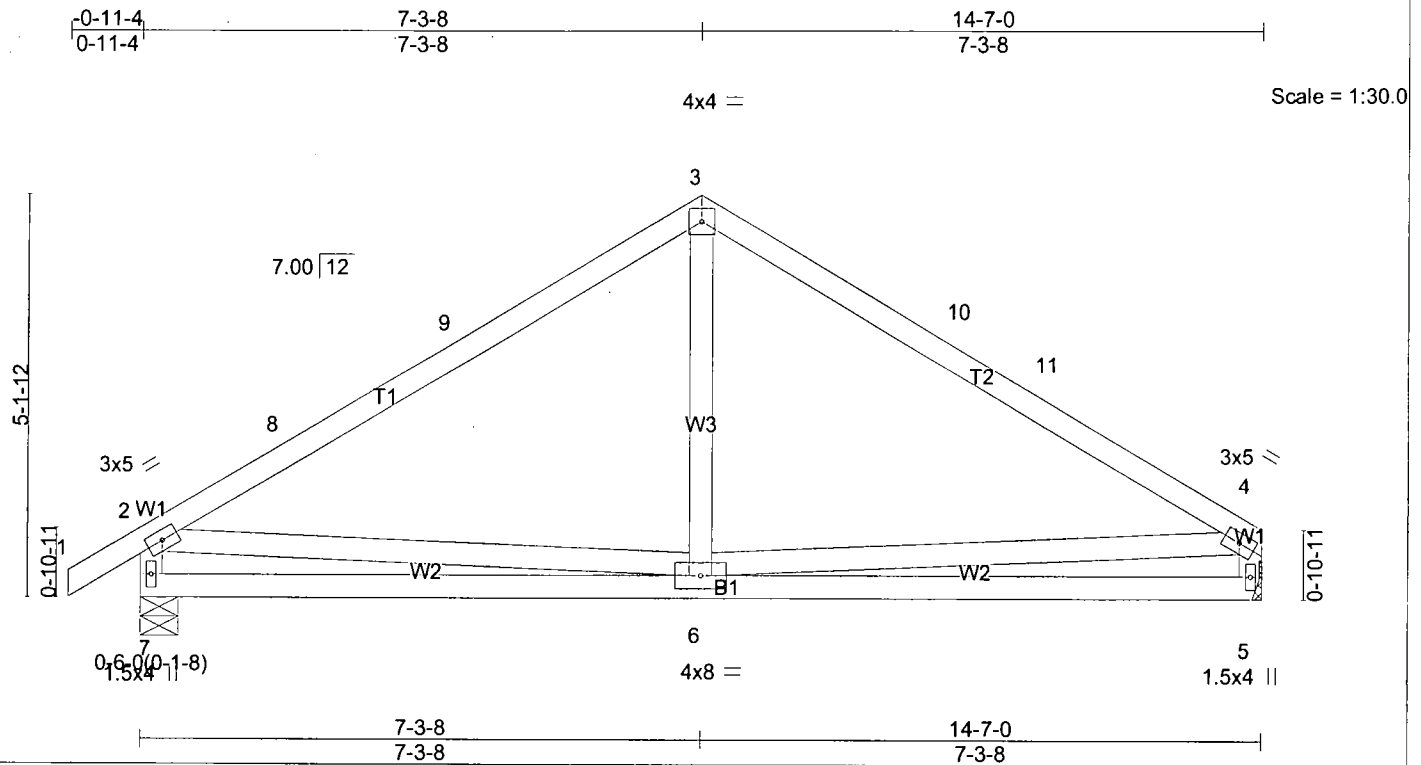
1) Regular: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-4=-74, 4-5=-74, 5-8=-74, 2-11=-20, 11-12=-85(F=-65), 7-12=-20

Concentrated Loads (lb)

Vert: 10=-226(F) 13=-226(F)



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 30.0	2-0-0	TC 0.76	in (loc) l/defl L/d	MT20	197/144
TCDL 7.0	Plates Increase 1.15	BC 0.55	Vert(LL) -0.07 5-6 >999 240		
BCLL 0.0	Lumber Increase 1.15	WB 0.22	Vert(TL) -0.18 5-6 >977 180		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) -0.00 5 n/a n/a		
	Code WISC/IRC06/TPI2002			Weight: 59 lb	

<p>LUMBER</p> <p>TOP CHORD 2 X 4 SPF 2100F 1.8E</p> <p>BOT CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>WEBS 2 X 4 SPF Stud *Except*</p> <p>W1: 2 X 4 SPF No.1 or SPF No.2</p>	<p>BRACING</p> <p>TOP CHORD Structural wood sheathing directly applied or 5-4-6 oc purlins, except end verticals.</p> <p>BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.</p> <div style="border: 1px solid black; padding: 5px; margin-top: 10px;"> <p>MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</p> </div>
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REACTIONS (lb/size) 7=755/0-6-0 (min. 0-1-8), 5=669/Mechanical
 Max Horz 7=105(LC 8)
 Max Uplift 7=-134(LC 9), 5=-87(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-8=-709/88, 8-9=-599/96, 3-9=-581/107, 3-10=-578/107, 10-11=-599/94, 4-11=-706/93, 2-7=-701/207, 4-5=-614/154

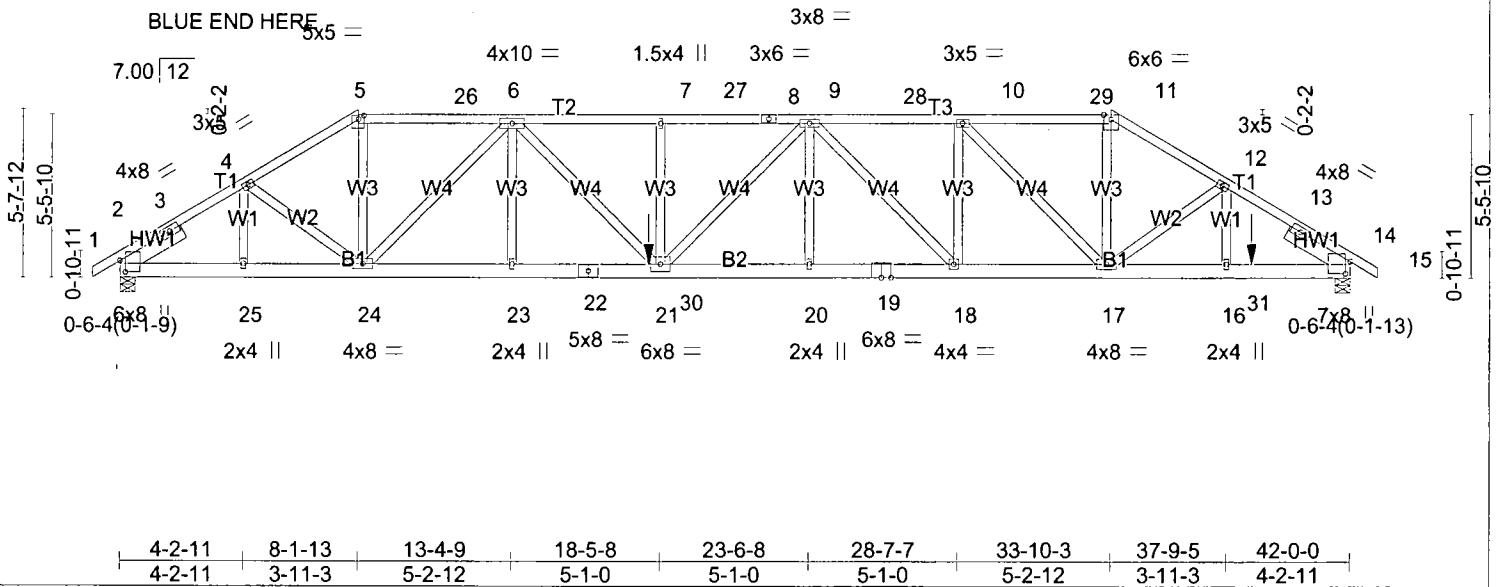
WEBS 3-6=0/294, 2-6=0/520, 4-6=-3/520

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) -0-11-4 to 2-0-12, Interior(1) 2-0-12 to 7-3-8, Exterior(2) 7-3-8 to 10-3-8 zone; cantilever left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) Refer to girder(s) for truss to truss connections.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (jt=lb) 7=134.
 - 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

0-11-4	4-2-11	8-1-13	13-4-9	18-5-8	23-6-8	28-7-7	33-10-3	37-9-5	42-0-0	42-11-4
0-11-4	4-2-11	3-11-3	5-2-12	5-1-0	5-1-0	5-1-0	5-2-12	3-11-3	4-2-11	0-11-4

Scale = 1:78.8



4-2-11	8-1-13	13-4-9	18-5-8	23-6-8	28-7-7	33-10-3	37-9-5	42-0-0
4-2-11	3-11-3	5-2-12	5-1-0	5-1-0	5-1-0	5-2-12	3-11-3	4-2-11

Plate Offsets (X,Y): [2:0-4-10,0-2-4], [5:0-2-4,0-1-7], [14:0-4-14,0-2-0]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 30.0	Plates Increase 1.15	TC 0.88	Vert(LL)	-0.36	20-21	>999	MT20	197/144
TCDL 7.0	Lumber Increase 1.15	BC 0.38	Vert(TL)	-0.66	20-21	>761		
BCLL 0.0	Rep Stress Incr NO	WB 0.93	Horz(TL)	0.13	14	n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002	(Matrix)						

Weight: 495 lb

LUMBER
 TOP CHORD 2 X 4 SPF 1650F 1.5E *Except*
 T2,T3: 2 X 4 SPF No.1 or SPF No.2
 BOT CHORD 2 X 6 SYP 2400F 2.0E
 WEBS 2 X 4 SPF Stud
 SLIDER Left 2 X 6 SPF 1650F 1.5E 2-4-14,
 Right 2 X 6 SPF 1650F 1.5E 2-4-14

BRACING
 TOP CHORD Structural wood sheathing directly applied or 3-8-11 oc purlins, except
 2-0-0 oc purlins (3-5-5 max.): 5-11.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=3723/0-6-4 (min. 0-1-9), 14=4432/0-6-4 (min. 0-1-13)
 Max Horz 2=118(LC 6)
 Max Uplift 2=-518(LC 7), 14=-612(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-5786/760, 3-4=-5705/771, 4-5=-5975/838, 5-26=-5266/769, 6-26=-5269/769,
 6-27=-10010/1394, 7-27=-10010/1394, 7-8=-10010/1394, 8-9=-10010/1394,
 9-28=-8365/1180, 10-28=-8365/1180, 10-29=-5995/865, 11-29=-5992/865,
 11-12=-6787/945, 12-13=-6803/913, 13-14=-6880/903
 BOT CHORD 2-25=-574/4724, 24-25=-574/4724, 23-24=-962/7983, 22-23=-962/7983, 22-30=-962/7983,
 21-30=-962/7983, 20-21=-1188/9710, 19-20=-1188/9710, 18-19=-1188/9710,
 17-18=-1011/8365, 16-17=-694/5636, 16-31=-694/5636, 14-31=-694/5636
 WEBS 4-25=-268/87, 4-24=-105/760, 5-24=-308/2588, 6-24=-3916/515, 6-23=0/266,
 6-21=-383/2935, 7-21=-354/119, 9-21=-87/492, 9-20=-18/661, 9-18=-1968/251,
 10-18=-181/1957, 10-17=-3430/449, 11-17=-359/2978, 12-17=-71/516

- NOTES**
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 6 - 2 rows at 0-6-0 oc.
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 90mph; TCCL=4.2psf; BCCL=6.0psf; h=25ft; B=45ft; L=42ft; eave=5ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=518, 14=612.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1686 lb down and 238 lb up at 18-0-12, and 313 lb down and 44 lb up at 38-7-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

Job J1000359	Truss T09G	Truss Type Hip Truss	Qty 1	Ply 2	KINGS WAY - PIERRI RES. #5302 Job Reference (optional)
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Richco Structures, Sheboygan Falls , WI, Heather Kraning

7.220 s Dec 29 2009 MiTek Industries, Inc. Wed Jan 27 08:39:38 2010 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-74, 5-11=-74, 11-15=-74, 2-30=-20, 16-30=-125(F=-105), 14-16=-20

Concentrated Loads (lb)

Vert: 30=-1686(F) 31=-313(F)

-0-11-4 4-2-11 | 8-1-13 | 13-4-9 | 18-5-8 | 23-6-8 | 27-4-4 28-7-7 | 33-10-3 | 37-9-5 | 42-0-0 42-11-4
 0-11-4 4-2-11 | 3-11-3 | 5-2-12 | 5-1-0 | 5-1-0 | 3-9-12 1-3-3 | 5-2-12 | 3-11-3 | 4-2-11 0-11-4

Scale = 1:78.8

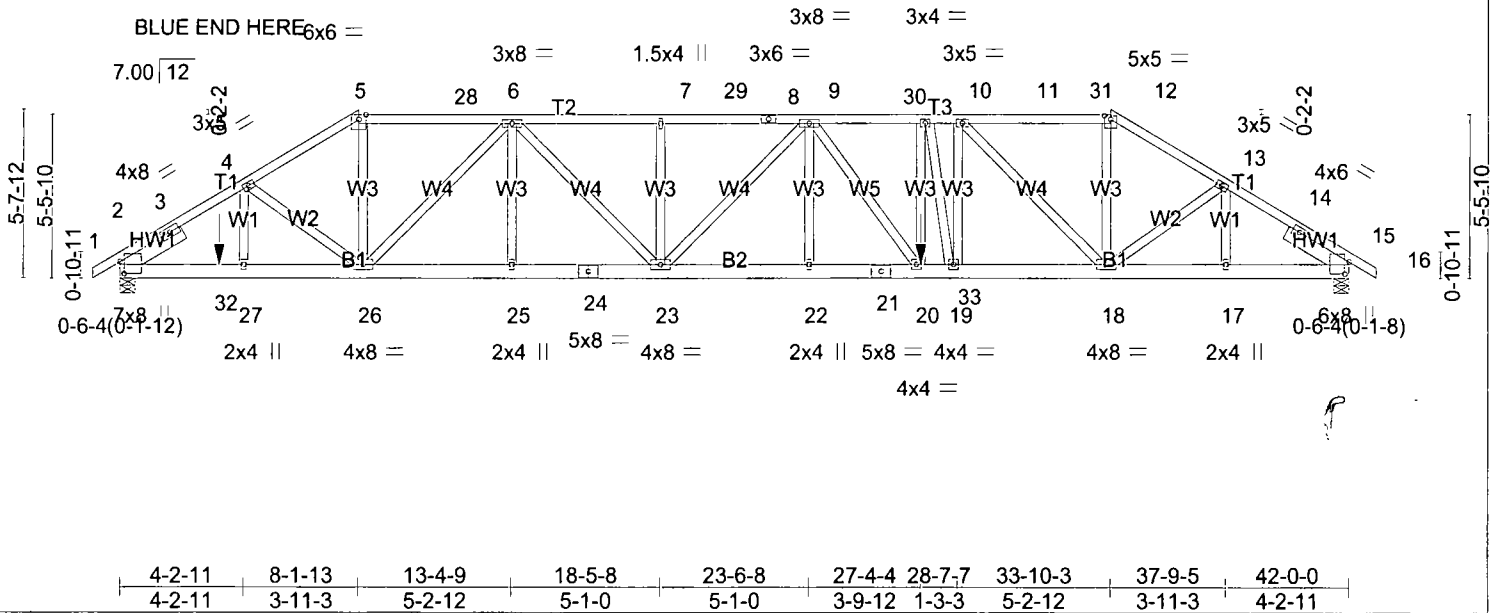


Plate Offsets (X,Y): [2:0-4-14,0-1-12], [12:0-2-8,0-1-7], [15:0-4-14,0-1-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d	PLATES	GRIP
TCLL 30.0	Plates Increase	1.15	TC 0.96	Vert(LL) -0.32 22-23	>999 240	MT20	197/144
TCDL 7.0	Lumber Increase	1.15	BC 0.37	Vert(TL) -0.59 22-23	>852 180		
BCLL 0.0	Rep Stress Incr	NO	WB 0.80	Horz(TL) 0.12 15	n/a n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002		(Matrix)				Weight: 515 lb

LUMBER TOP CHORD 2 X 4 SPF No.1 or SPF No.2 BOT CHORD 2 X 6 SYP 2400F 2.0E WEBS 2 X 4 SPF Stud SLIDER Left 2 X 6 SPF 1650F 1.5E 2-4-14, Right 2 X 6 SPF 1650F 1.5E 2-4-14	BRACING TOP CHORD Structural wood sheathing directly applied or 2-5-14 oc purlins, except 2-0-0 oc purlins (3-11-11 max.): 5-12. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
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REACTIONS (lb/size) 2=4235/0-6-4 (min. 0-1-12), 15=3331/0-6-4 (min. 0-1-8)
 Max Horz 2=-118(LC 5)
 Max Uplift 2=-586(LC 7), 15=-467(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD
 2-3=-6542/859, 3-4=-6465/871, 4-5=-6306/881, 5-28=-5563/808, 6-28=-5566/808,
 6-29=-8452/1189, 7-29=-8452/1189, 7-8=-8452/1189, 8-9=-8452/1189, 9-30=-7318/1041,
 10-30=-7318/1041, 10-11=-7010/1002, 11-31=-4640/686, 12-31=-4637/686,
 12-13=-5271/745, 13-14=-5055/686, 14-15=-5137/675

BOT CHORD
 2-32=-658/5360, 27-32=-658/5360, 26-27=-658/5360, 25-26=-910/7587, 24-25=-910/7587,
 23-24=-910/7587, 22-23=-1002/8294, 21-22=-1002/8294, 21-33=-1002/8294,
 20-33=-1002/8294, 19-20=-867/7276, 18-19=-827/6957, 17-18=-504/4188,
 15-17=-504/4188

WEBS
 4-27=0/265, 4-26=-45/334, 5-26=-329/2746, 6-26=-2935/386, 6-25=-24/687,
 6-23=-156/1290, 7-23=-364/121, 9-23=-57/290, 9-22=-38/761, 11-19=-176/1864,
 11-18=-3357/442, 12-18=-262/2246, 13-18=-90/655, 10-20=-180/1479, 9-20=-1689/218,
 10-19=-1449/183

- NOTES**
- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
 Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
 Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 - 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - 3) Unbalanced roof live loads have been considered for this design.
 - 4) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=42ft; eave=5ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 5) Provide adequate drainage to prevent water ponding.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=586, 15=467.
 - 8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 9) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 - 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 313 lb down and 44 lb up at 3-4-8, and 478 lb down and 67 lb up at 27-4-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- Continued on page 2

Job	Truss	Truss Type	Qty	Ply	KINGS WAY - PIERRI RES. #5302
J1000359	T09GA	Hip Truss	1	2	Job Reference (optional)

Richco Structures, Sheboygan Falls, WI, Heather Kraning

7.220 s Dec 29 2009 MiTek Industries, Inc. Wed Jan 27 08:39:40 2010 Page 2

LOAD CASE(S) Standard

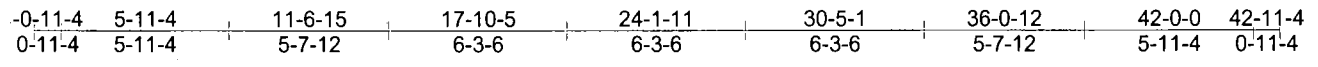
1) Regular: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-74, 5-12=-74, 12-16=-74, 2-32=-20, 32-33=-137(F=-117), 15-33=-20

Concentrated Loads (lb)

Vert: 20=-478(F) 32=-313(F)



Scale = 1:81.2

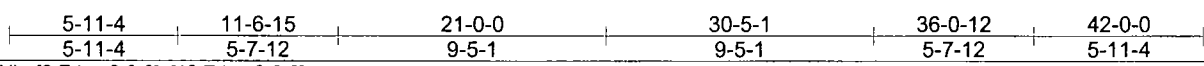
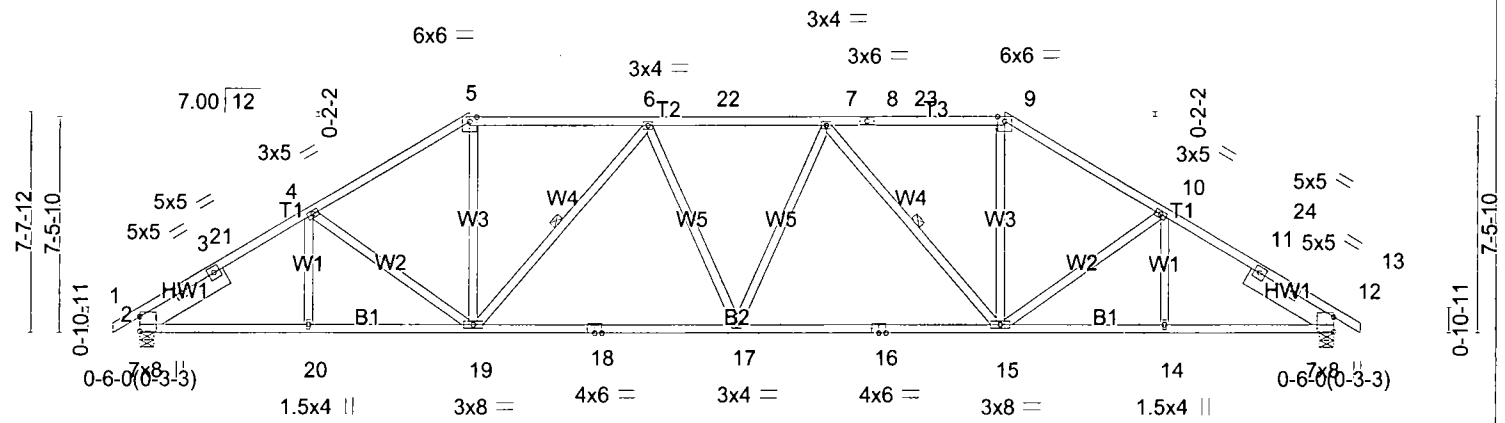


Plate Offsets (X,Y): [2:Edge,0-0-0], [12:Edge,0-0-0]									
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 30.0	Plates Increase	1.15	TC 0.94	Vert(LL)	-0.26	17	>999	MT20	197/144
TCDL 7.0	Lumber Increase	1.15	BC 0.89	Vert(TL)	-0.62	15-17	>807		
BCLL 0.0	Rep Stress Incr	YES	WB 0.39	Horz(TL)	0.21	12	n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002		(Matrix)						
								Weight: 202 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SPF 1650F 1.5E *Except* T3,T2: 2 X 4 SPF No.1 or SPF No.2	TOP CHORD Structural wood sheathing directly applied, except 2-0-0 oc purlins (3-0-11 max.): 5-9.
BOT CHORD 2 X 4 SPF No.1 or SPF No.2	BOT CHORD Rigid ceiling directly applied or 9-2-5 oc bracing.
WEBS 2 X 4 SPF Stud *Except* W4,W5: 2 X 4 SPF No.1 or SPF No.2	WEBS 1 Row at midpt 6-19, 7-15
SLIDER Left 2 X 8 SYP M 23 3-5-13, Right 2 X 8 SYP M 23 3-5-13	

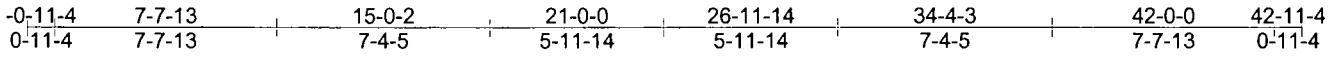
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=2043/0-6-0 (min. 0-3-3), 12=2043/0-6-0 (min. 0-3-3)
 Max Horz 2=-165(LC 7)
 Max Uplift 2=-298(LC 9), 12=-298(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-3127/529, 3-21=-2998/538, 4-21=-2982/551, 4-5=-2729/560, 5-22=-2331/534,
 6-22=-2331/533, 6-7=-2889/619, 7-23=-2329/532, 8-23=-2329/532, 8-9=-2329/534,
 9-10=-2729/560, 10-24=-2982/551, 11-24=-2998/538, 11-12=-3127/529
 BOT CHORD 2-20=-385/2487, 19-20=-385/2487, 18-19=-393/2845, 17-18=-393/2845, 16-17=-400/2848,
 15-16=-400/2848, 14-15=-379/2486, 12-14=-379/2486
 WEBS 5-19=-113/893, 6-19=-928/188, 7-15=-931/189, 9-15=-113/896

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=42ft; eave=5ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) 0-11-4 to 3-3-2, Interior(1) 3-3-2 to 11-6-15, Exterior(2) 11-6-15 to 36-0-12, Interior(1) 36-0-12 to 42-11-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=298, 12=298.
 - 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 7) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard



Scale = 1:79.3

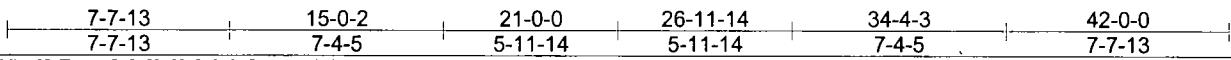
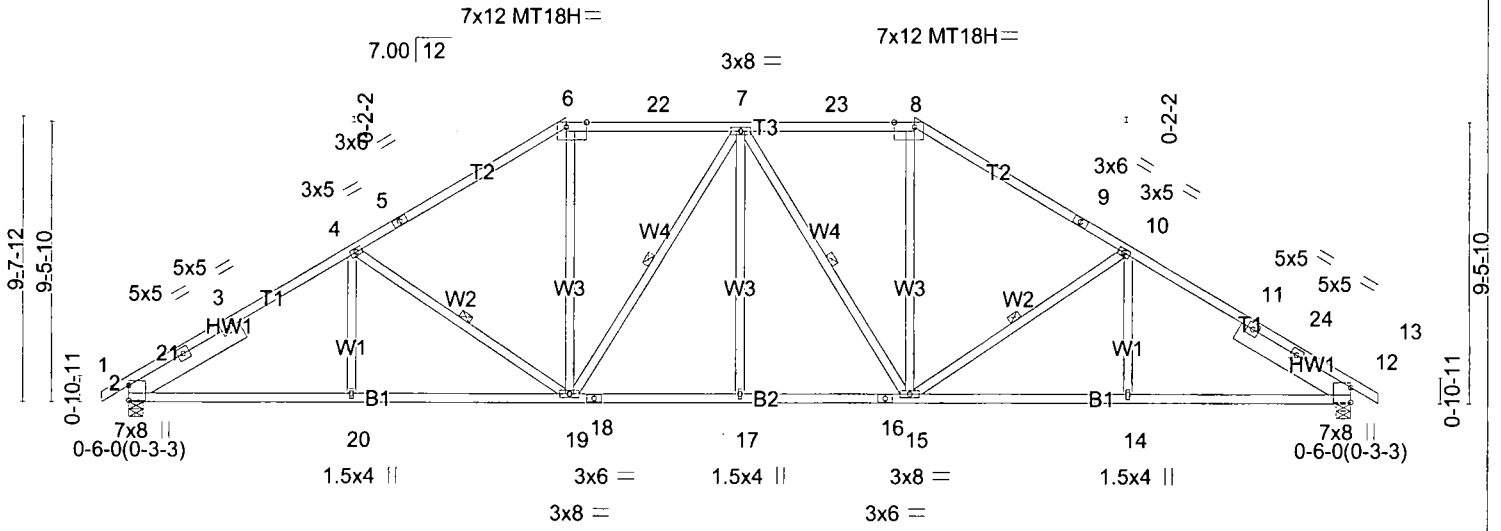


Plate Offsets (X,Y): [2:Edge,0-0-0], [6:0-8-6,Edge], [8:0-8-6,Edge], [12:Edge,0-0-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 30.0	Plates Increase	1.15	TC 0.80	Vert(LL)	-0.20	17	>999	MT20	197/144
TCDL 7.0	Lumber Increase	1.15	BC 0.72	Vert(TL)	-0.42	19-20	>999	MT18H	197/144
BCLL 0.0	Rep Stress Incr	YES	WB 0.31	Horz(TL)	0.20	12	n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002		(Matrix)						
								Weight: 217 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SPF 1650F 1.5E *Except* T3: 2 X 4 SPF No.1 or SPF No.2	TOP CHORD Structural wood sheathing directly applied or 2-7-4 oc purlins, except 2-0-0 oc purlins (3-8-9 max.): 6-8.
BOT CHORD 2 X 4 SPF No.1 or SPF No.2	BOT CHORD Rigid ceiling directly applied or 9-9-2 oc bracing.
WEBS 2 X 4 SPF No.1 or SPF No.2 *Except* W1: 2 X 4 SPF Stud	WEBS 1 Row at midpt 4-19, 7-19, 7-15, 10-15
SLIDER Left 2 X 8 SYP M 23 4-5-11, Right 2 X 8 SYP M 23 4-5-11	

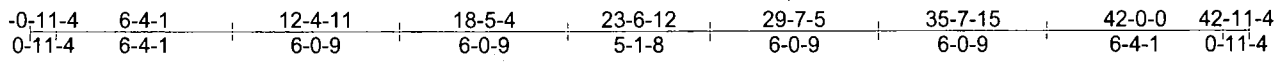
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=2043/0-6-0 (min. 0-3-3), 12=2043/0-6-0 (min. 0-3-3)
 Max Horz 2=-211(LC 7)
 Max Uplift 2=-298(LC 9), 12=-298(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-21=-3106/509, 3-21=-2972/513, 3-4=-2974/534, 4-5=-2531/512, 5-6=-2390/539,
 6-22=-2088/527, 7-22=-2091/526, 7-23=-2091/526, 8-23=-2088/527, 8-9=-2390/539,
 9-10=-2531/512, 10-11=-2974/534, 11-24=-2972/513, 12-24=-3106/509
 BOT CHORD 2-20=-360/2497, 19-20=-360/2497, 18-19=-252/2284, 17-18=-252/2284, 16-17=-252/2284,
 15-16=-252/2284, 14-15=-353/2497, 12-14=-353/2497
 WEBS 4-20=0/316, 4-19=-535/180, 6-19=-89/732, 7-19=-570/93, 7-15=-570/93, 8-15=-89/732,
 10-15=-535/180, 10-14=0/316

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=42ft; eave=5ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) 0-11-4 to 3-3-2, Interior(1) 3-3-2 to 15-0-2, Exterior(2) 15-0-2 to 32-11-3, Interior(1) 32-11-3 to 42-11-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - All plates are MT20 plates unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=298, 12=298.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard



Scale = 1:82.9

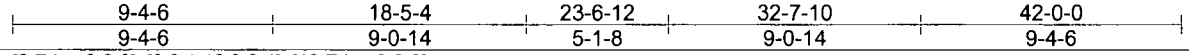
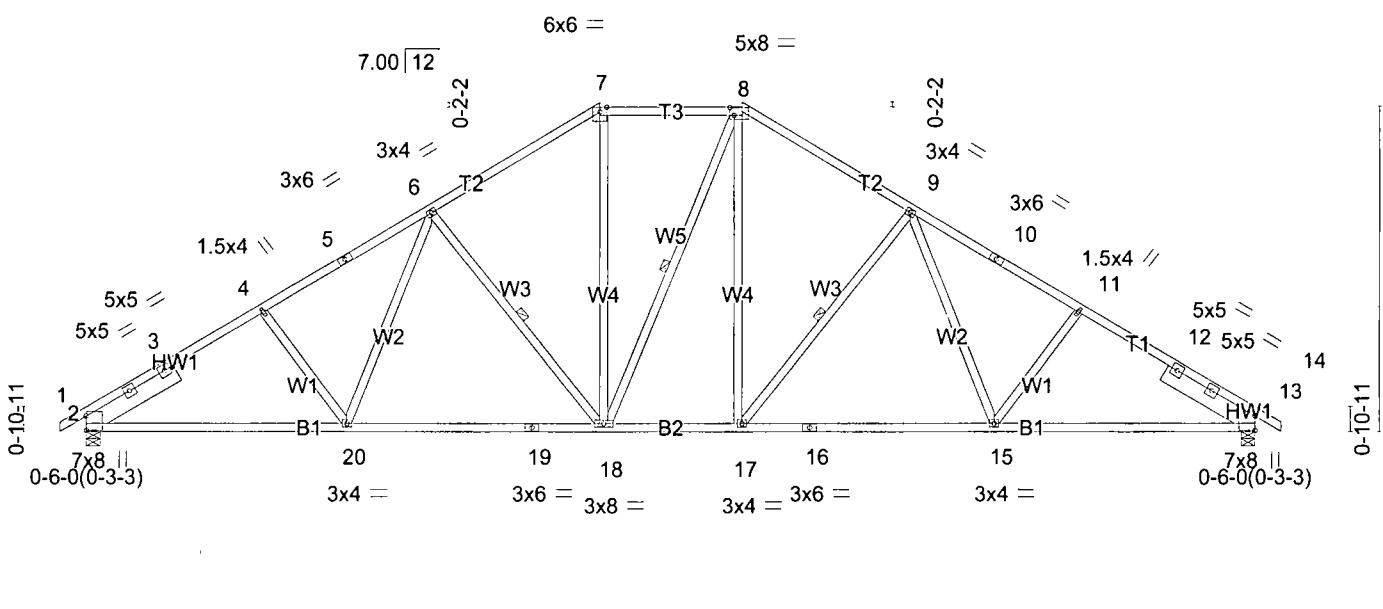


Plate Offsets (X,Y): [2:Edge,0-0-0], [8:0-1-12,0-3-4], [13:Edge,0-0-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 30.0	Plates Increase	1.15	TC 0.87	Vert(LL)	-0.21 15-17	>999	240	MT20	197/144
TCDL 7.0	Lumber Increase	1.15	BC 0.80	Vert(TL)	-0.56 15-17	>897	180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.32	Horz(TL)	0.19 13	n/a	n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002		(Matrix)						
									Weight: 218 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SPF No.1 or SPF No.2 *Except* T1: 2 X 4 SPF 1650F 1.5E	TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except 2-0-0 oc purlins (3-11-14 max.): 7-8.
BOT CHORD 2 X 4 SPF No.1 or SPF No.2	BOT CHORD Rigid ceiling directly applied or 9-8-5 oc bracing.
WEBS 2 X 4 SPF No.1 or SPF No.2 *Except* W1: 2 X 4 SPF Stud	WEBS 1 Row at midpt 6-18, 8-18, 9-17
SLIDER Left 2 X 8 SYP M 23 3-8-12, Right 2 X 8 SYP M 23 3-8-12	

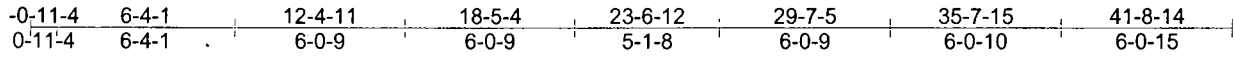
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=2043/0-6-0 (min. 0-3-3), 13=2043/0-6-0 (min. 0-3-3)
Max Horz 2=-256(LC 7)
Max Uplift 2=-298(LC 9), 13=-298(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-3108/507, 3-4=-2972/527, 4-5=-2810/521, 5-6=-2720/535, 6-7=-2224/523, 7-8=-1830/505, 8-9=-2223/523, 9-10=-2720/535, 10-11=-2810/521, 11-12=-2972/527, 12-13=-3109/507
BOT CHORD 2-20=-357/2472, 19-20=-265/2236, 18-19=-265/2236, 17-18=-116/1829, 16-17=-256/2236, 15-16=-256/2236, 13-15=-348/2472
WEBS 6-20=-9/401, 6-18=-685/228, 7-18=-103/656, 8-18=-275/278, 8-17=-100/669, 9-17=-686/229, 9-15=-9/401

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=42ft; eave=5ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) 0-11-4 to 3-3-2, Interior(1) 3-3-2 to 18-5-4, Exterior(2) 18-5-4 to 23-6-12, Interior(1) 29-7-5 to 42-11-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=298, 13=298.
 - "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard



Scale = 1:81.9

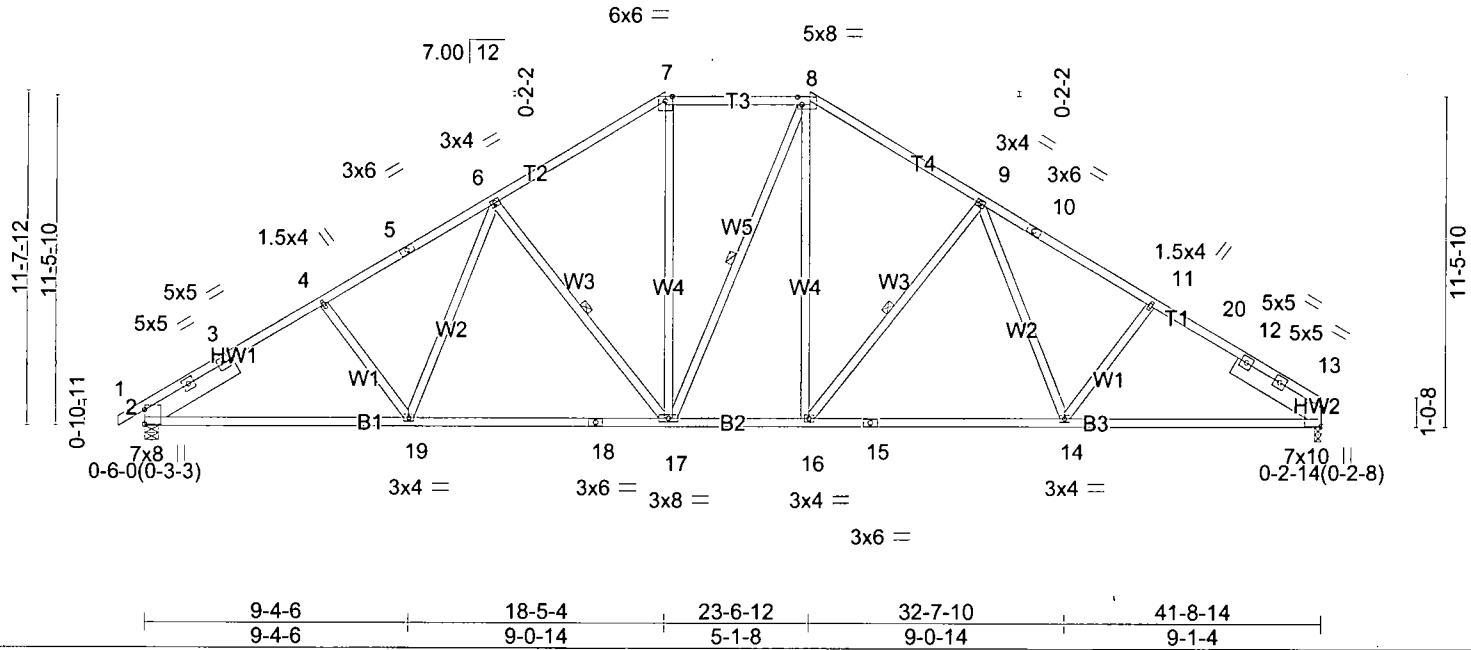


Plate Offsets (X,Y): [2:Edge,0-0-0], [8:0-1-12,0-3-4], [13:0-7-15,0-0-3]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc) l/defl L/d	PLATES	GRIP
TCLL 30.0	Plates Increase	1.15	TC 0.81	Vert(LL) -0.19 14-16	>999 240	MT20	197/144
TCDL 7.0	Lumber Increase	1.15	BC 0.80	Vert(TL) -0.51 17-19	>975 180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.32	Horz(TL) 0.17 13	n/a n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002		(Matrix)				Weight: 216 lb

LUMBER	BRACING
TOP CHORD 2 X 4 SPF No.1 or SPF No.2 *Except* T1: 2 X 4 SPF 2100F 1.8E	TOP CHORD Structural wood sheathing directly applied or 2-5-1 oc purlins, except 2-0-0 oc purlins (4-0-2 max.); 7-8.
BOT CHORD 2 X 4 SPF No.1 or SPF No.2 *Except* B3: 2 X 4 SPF 2100F 1.8E	BOT CHORD Rigid ceiling directly applied or 9-9-9 oc bracing.
WEBS 2 X 4 SPF No.1 or SPF No.2 *Except* W1: 2 X 4 SPF Stud	WEBS 1 Row at midpt 6-17, 8-17, 9-16
SLIDER Left 2 X 8 SYP M 23 3-8-12, Right 2 X 8 SYP M 23 3-7-13	

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=2032/0-6-0 (min. 0-3-3), 13=1961/0-2-14 (min. 0-2-8)
 Max Horz 2=-257(LC 7)
 Max Uplift 2=-297(LC 9), 13=-257(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-3089/504, 3-4=-2952/524, 4-5=-2790/519, 5-6=-2700/532, 6-7=-2202/521,
 7-8=-1811/505, 8-9=-2195/523, 9-10=-2545/533, 10-11=-2728/519, 11-20=-2830/524,
 12-20=-2878/515, 12-13=-3015/503
 BOT CHORD 2-19=-349/2456, 18-19=-257/2218, 17-18=-257/2218, 16-17=-112/1805, 15-16=-249/2192,
 14-15=-249/2192, 13-14=-331/2365
 WEBS 6-19=-10/402, 6-17=-685/228, 7-17=-102/644, 8-17=-263/287, 8-16=-98/646,
 9-16=-658/223, 9-14=-7/364

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=42ft; eave=5ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) -0-11-4 to 3-2-13, Interior(1) 3-2-13 to 18-5-4, Exterior(2) 18-5-4 to 23-6-12, Interior(1) 29-7-5 to 41-8-14 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 13.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=297, 13=257.
 - 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

LOAD CASE(S) Standard



Scale = 1:80.5

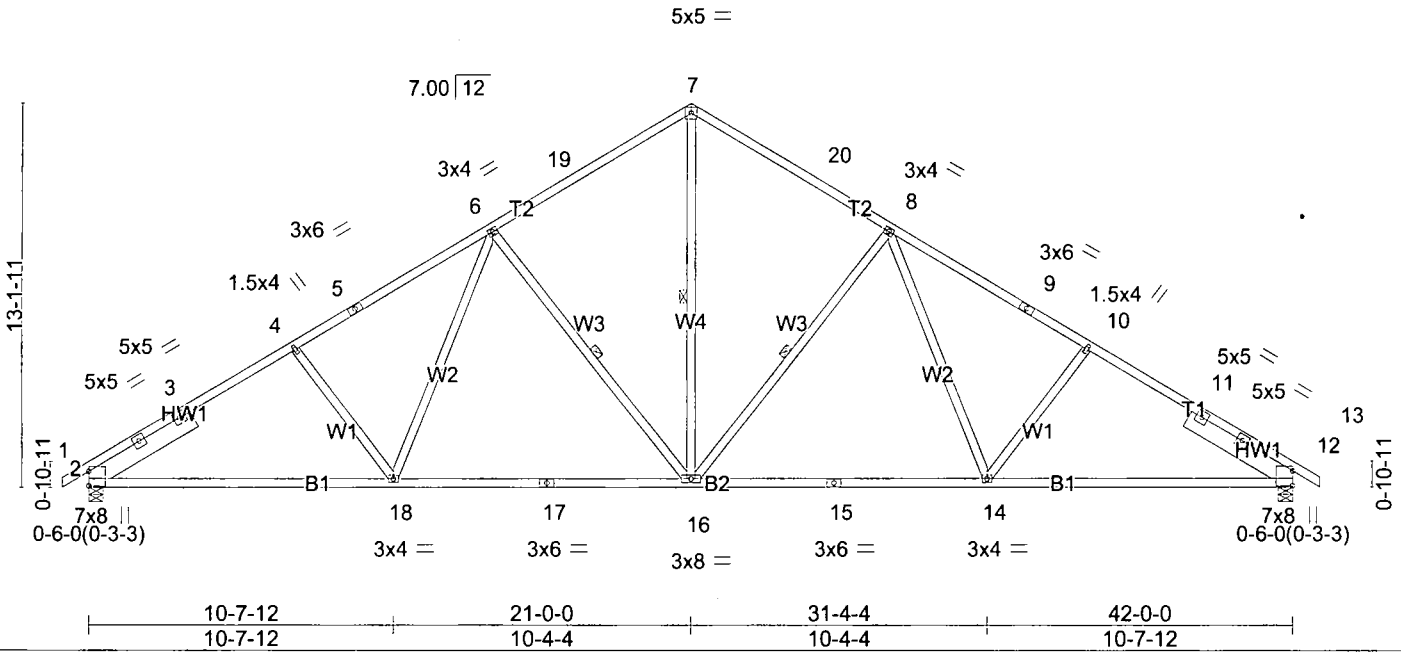


Plate Offsets (X,Y): [2:Edge,0-0-0], [12:Edge,0-0-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL		PLATES	GRIP
TCLL 30.0	Plates Increase	1.15	TC 0.76	Vert(LL) -0.29 12-14	>999 240	MT20	197/144
TCDL 7.0	Lumber Increase	1.15	BC 0.91	Vert(TL) -0.77 12-14	>657 180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.48	Horz(TL) 0.19 12	n/a n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002		(Matrix)				Weight: 205 lb

<p>LUMBER</p> <p>TOP CHORD 2 X 4 SPF No.1 or SPF No.2 *Except* T1: 2 X 4 SPF 1650F 1.5E</p> <p>BOT CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>WEBS 2 X 4 SPF No.1 or SPF No.2 *Except* W1: 2 X 4 SPF Stud</p> <p>SLIDER Left 2 X 8 SYP M 23 4-2-11, Right 2 X 8 SYP M 23 4-2-11</p>	<p>BRACING</p> <p>TOP CHORD Structural wood sheathing directly applied or 2-6-9 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.</p> <p>BOT CHORD 1 Row at midpt 7-16, 8-16, 6-16</p> <p>WEBS</p> <div style="border: 1px solid black; padding: 5px; margin-top: 10px;"> <p>MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</p> </div>
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REACTIONS (lb/size) 2=2043/0-6-0 (min. 0-3-3), 12=2043/0-6-0 (min. 0-3-3)
 Max Horz 2=-292(LC 7)
 Max Uplift 2=-298(LC 9), 12=-298(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

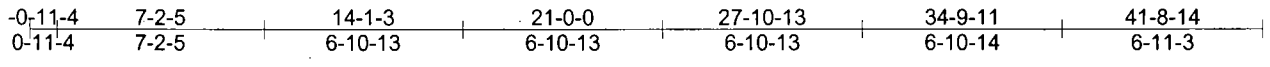
TOP CHORD 2-3=-3084/462, 3-4=-2946/484, 4-5=-2779/464, 5-6=-2667/490, 6-19=-2042/437,
 7-19=-1897/463, 7-20=-1897/463, 8-20=-2042/437, 8-9=-2667/490, 9-10=-2779/464,
 10-11=-2946/484, 11-12=-3084/462

BOT CHORD 2-18=-298/2468, 17-18=-172/2133, 16-17=-172/2133, 15-16=-172/2133, 14-15=-172/2133,
 12-14=-298/2468

WEBS 7-16=-277/1464, 8-16=-829/257, 8-14=-30/505, 10-14=-313/202, 6-16=-829/257,
 6-18=-30/505, 4-18=-313/202

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=42ft; eave=5ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) 0-11-4 to 3-3-2, Interior(1) 3-3-2 to 21-0-0, Exterior(2) 21-0-0 to 25-2-6 zone; cantilever left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=298, 12=298.
 - 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



Scale = 1:79.7

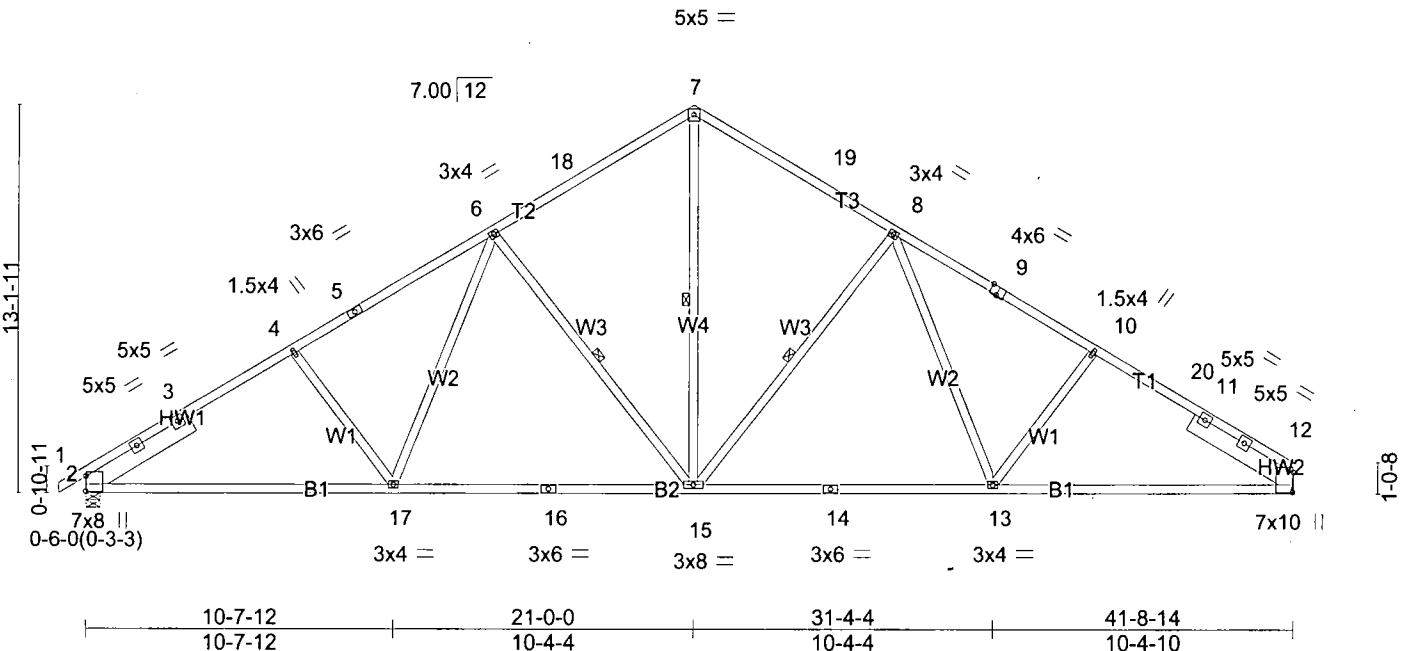


Plate Offsets (X,Y): [2:Edge,0-0-0], [9:0-3-0,Edge], [12:0-7-15,0-0-3]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 30.0	Plates Increase 1.15	TC 1.00	in (loc) l/defl L/d	MT20	197/144
TCDL 7.0	Lumber Increase 1.15	BC 0.91	Vert(LL) -0.29 2-17 >999 240		
BCLL 0.0	Rep Stress Incr YES	WB 0.48	Vert(TL) -0.77 2-17 >654 180		
BCDL 10.0	Code WISC/IRC06/TPI2002	(Matrix)	Horz(TL) 0.18 12 n/a n/a		
				Weight: 203 lb	

<p>LUMBER</p> <p>TOP CHORD 2 X 4 SPF No.1 or SPF No.2 *Except* T1: 2 X 4 SPF 2100F 1.8E</p> <p>BOT CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>WEBS 2 X 4 SPF No.1 or SPF No.2 *Except* W1: 2 X 4 SPF Stud</p> <p>SLIDER Left 2 X 8 SYP M 23 4-2-11, Right 2 X 8 SYP M 23 4-1-12</p>	<p>BRACING</p> <p>TOP CHORD Structural wood sheathing directly applied.</p> <p>BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.</p> <p>WEBS 1 Row at midpt 6-15, 7-15, 8-15</p> <div style="border: 1px solid black; padding: 5px; margin-top: 10px;"> <p>MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</p> </div>
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REACTIONS (lb/size) 2=2032/0-6-0 (min. 0-3-3), 12=1961/Mechanical
 Max Horz 2=-293(LC 7)
 Max Uplift 2=-297(LC 9), 12=-257(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-3065/460, 3-4=-2928/482, 4-5=-2760/462, 5-6=-2647/488, 6-18=-2020/433,
 7-18=-1875/460, 7-19=-1875/462, 8-19=-2020/435, 8-9=-2582/488, 9-10=-2682/472,
 10-20=-2832/481, 11-20=-2864/468, 11-12=-3016/458

BOT CHORD 2-17=-296/2453, 16-17=-166/2114, 15-16=-166/2114, 14-15=-167/2097, 13-14=-167/2097,
 12-13=-288/2373

WEBS 4-17=-316/203, 6-17=-31/507, 6-15=-829/257, 7-15=-276/1443, 8-15=-805/254,
 8-13=-28/456, 10-13=-262/198

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=42ft; eave=5ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) 0-11-4 to 3-2-13, Interior(1) 3-2-13 to 21-0-0, Exterior(2) 21-0-0 to 25-2-1 zone; cantilever left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) Refer to girder(s) for truss to truss connections.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=297, 12=257.
 - 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

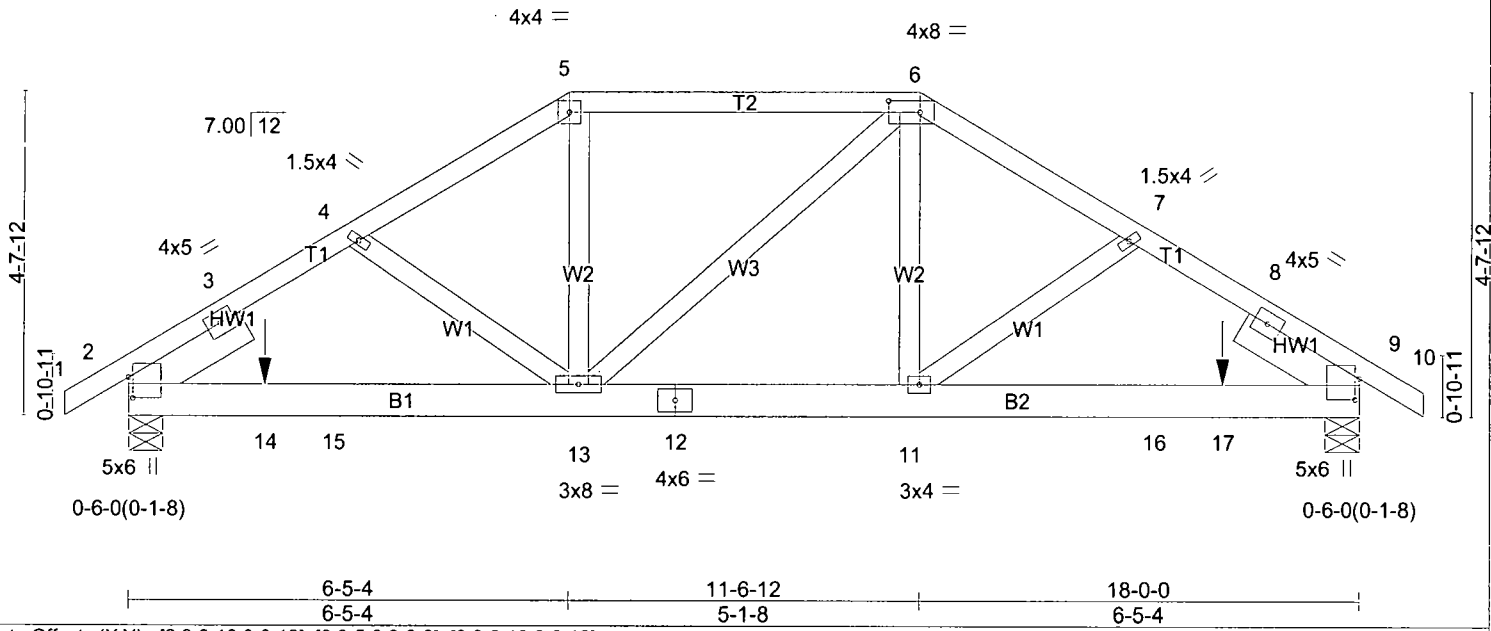
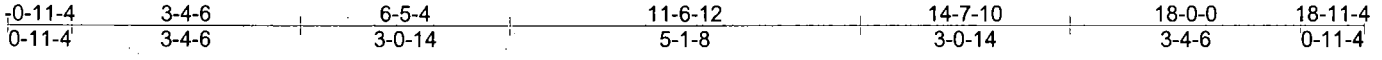


Plate Offsets (X,Y): [2:0-3-10,0-0-12], [6:0-5-8,0-2-0], [9:0-3-10,0-0-12]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 30.0	Plates Increase	1.15	TC 0.96	Vert(LL)	-0.07	9-11	>999	240	MT20	197/144
TCDL 7.0	Lumber Increase	1.15	BC 0.39	Vert(TL)	-0.16	9-11	>999	180		
BCLL 0.0	Rep Stress Incr	NO	WB 0.30	Horz(TL)	0.03	9	n/a	n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002		(Matrix)							
								Weight: 99 lb		

LUMBER
 TOP CHORD 2 X 4 SPF No.1 or SPF No.2
 BOT CHORD 2 X 6 SYP 2400F 2.0E
 WEBS 2 X 4 SPF Stud
 SLIDER Left 2 X 6 SPF 1650F 1.5E 1-11-0,
 Right 2 X 6 SPF 1650F 1.5E 1-11-0

BRACING
 TOP CHORD Structural wood sheathing directly applied or 2-0-7 oc purlins, except 2-0-0 oc purlins (2-7-13 max.): 5-6.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

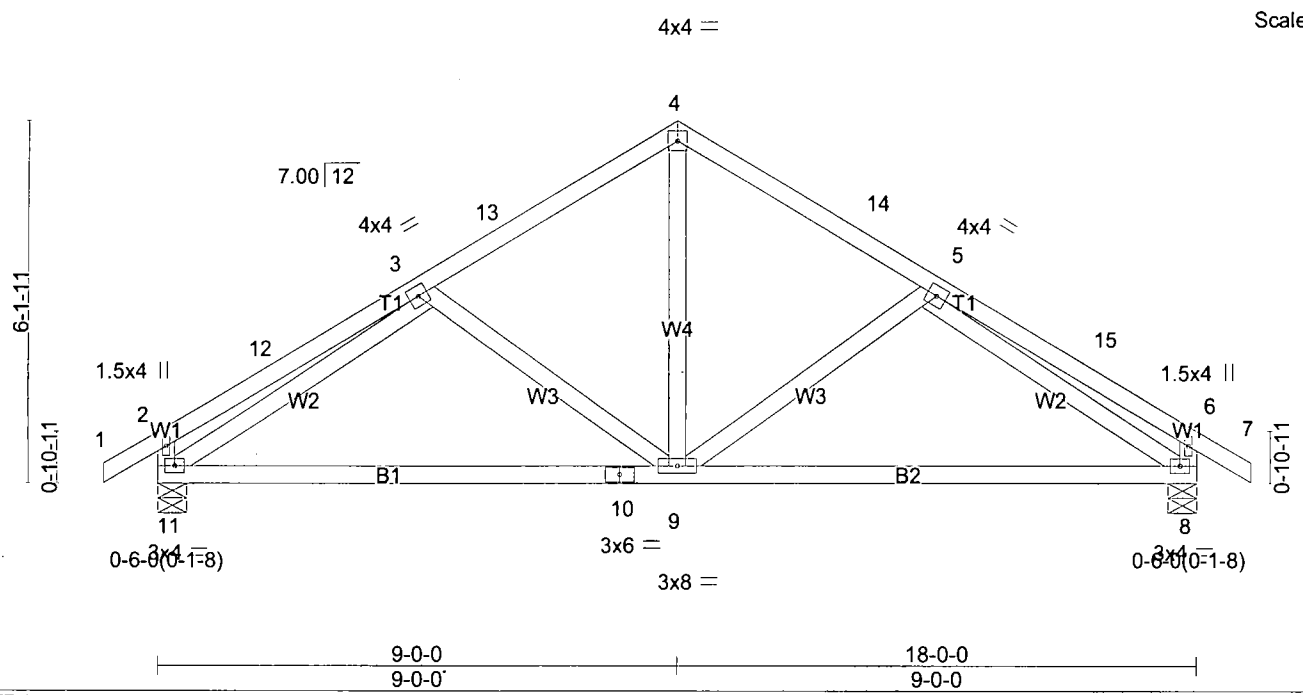
REACTIONS (lb/size) 2=1732/0-6-0 (min. 0-1-8), 9=1732/0-6-0 (min. 0-1-8)
 Max Horz 2=97(LC 6)
 Max Uplift 2=-262(LC 4), 9=-262(LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-2158/358, 3-4=-2090/364, 4-5=-2039/364, 5-6=-1750/333, 6-7=-2038/365,
 7-8=-2089/364, 8-9=-2157/358
 BOT CHORD 2-14=-294/1678, 14-15=-294/1678, 13-15=-294/1678, 12-13=-254/1749, 11-12=-254/1749,
 11-16=-258/1677, 16-17=-258/1677, 9-17=-258/1677
 WEBS 5-13=-76/685, 6-11=-85/690

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights); cantilever left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=262, 9=262.
 - 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
 - 7) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 - 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 253 lb down and 60 lb up at 2-0-0, and 253 lb down and 60 lb up at 16-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Regular: Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: 1-5=-74, 5-6=-74, 6-10=-74, 2-15=-20, 15-16=-114(F=-94), 9-16=-20
 Concentrated Loads (lb)
 Vert: 14=-253(F) 17=-253(F)



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 30.0	2-0-0	TC 0.36	in (loc) l/def L/d	MT20	197/144
TCDL 7.0	Plates Increase 1.15	BC 0.93	Vert(LL) -0.16 8-9 >999 240		
BCLL 0.0	Lumber Increase 1.15	WB 0.68	Vert(TL) -0.41 8-9 >522 180		
BCDL 10.0	Code WISC/IRC06/TPI2002	(Matrix)	Horz(TL) 0.03 8 n/a n/a		
				Weight: 78 lb	

<p>LUMBER</p> <p>TOP CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>BOT CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>WEBS 2 X 4 SPF Stud *Except*</p> <p style="margin-left: 20px;">W1: 2 X 4 SPF No.1 or SPF No.2</p>	<p>BRACING</p> <p>TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purtins, except end verticals.</p> <p>BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.</p> <div style="border: 1px solid black; padding: 5px; margin-top: 10px;"> <p>MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</p> </div>
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REACTIONS (lb/size) 11=912/0-6-0 (min. 0-1-8), 8=912/0-6-0 (min. 0-1-8)

Max Horz 11=-117(LC 7)

Max Uplift 11=-154(LC 9), 8=-154(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

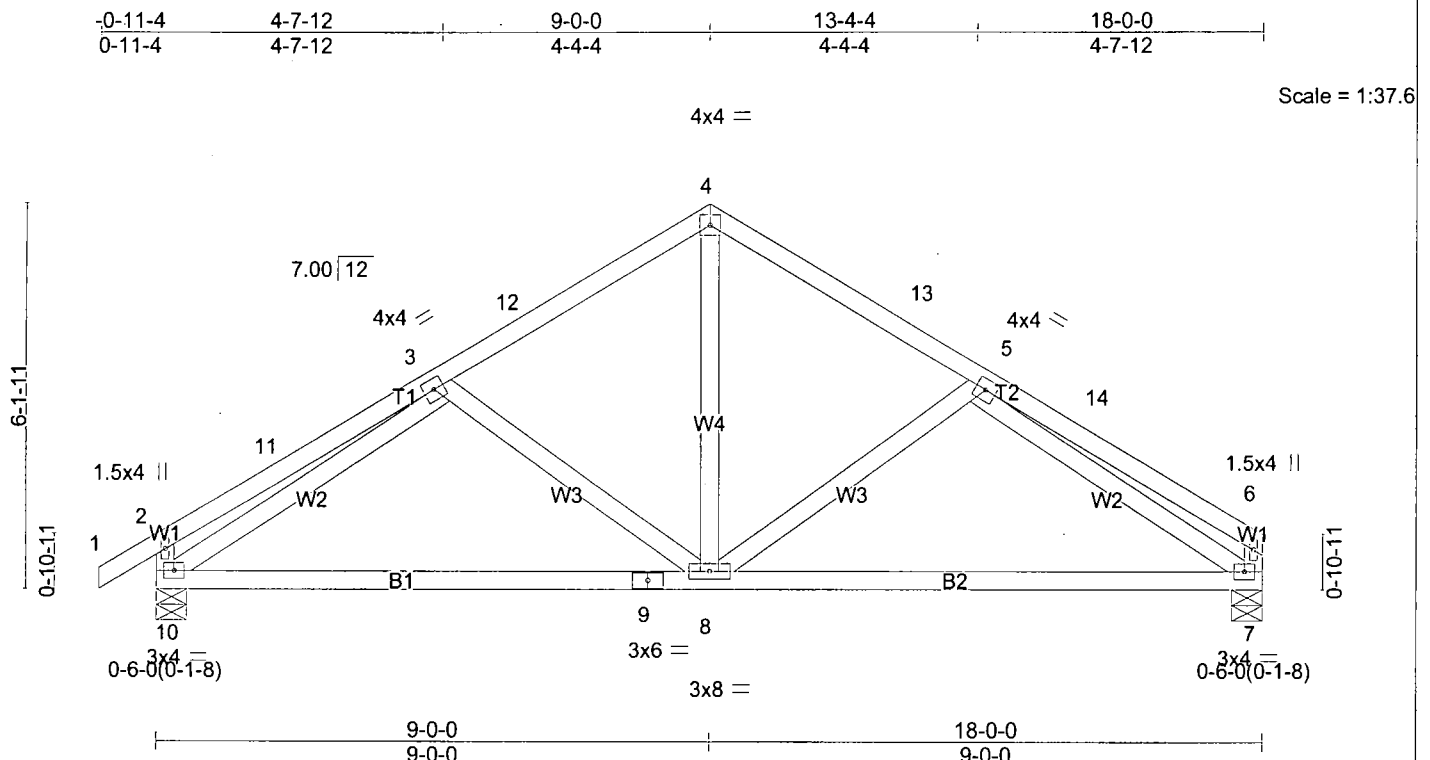
TOP CHORD 3-13=-862/153, 4-13=-761/168, 4-14=-761/168, 5-14=-862/153

BOT CHORD 10-11=-109/856, 9-10=-109/856, 8-9=-118/856

WEBS 4-9=-63/505, 5-9=-272/174, 3-9=-272/174, 3-11=-1063/146, 5-8=-1063/146

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) 0-11-4 to 2-0-12, Interior(1) 2-0-12 to 9-0-0, Exterior(2) 9-0-0 to 12-0-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 11=154, 8=154.
 - 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



Scale = 1:37.6

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 30.0	2-0-0	TC 0.41	in (loc) l/defl L/d	MT20	197/144
TCDL 7.0	Plates Increase 1.15	BC 0.93	Vert(LL) -0.16 7-8 >999 240		
BCLL 0.0	Lumber Increase 1.15	WB 0.69	Vert(TL) -0.41 7-8 >522 180		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.03 7 n/a n/a		
	Code WISC/IRC06/TPI2002			Weight: 77 lb	

<p>LUMBER</p> <p>TOP CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>BOT CHORD 2 X 4 SPF No.1 or SPF No.2</p> <p>WEBS 2 X 4 SPF Stud *Except*</p> <p>W1: 2 X 4 SPF No.1 or SPF No.2</p>	<p>BRACING</p> <p>TOP CHORD Structural wood sheathing directly applied or 5-11-9 oc purlins, except end verticals.</p> <p>BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.</p> <div style="border: 1px solid black; padding: 5px; margin-top: 10px;"> <p>MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</p> </div>
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REACTIONS (lb/size) 10=915/0-6-0 (min. 0-1-8), 7=830/0-6-0 (min. 0-1-8)

Max Horz 10=128(LC 8)

Max Uplift 10=-155(LC 9), 7=-109(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

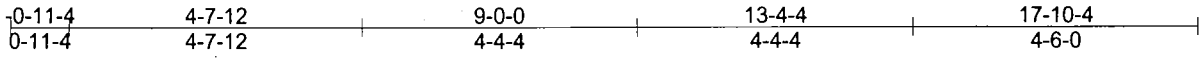
TOP CHORD 3-12=-866/153, 4-12=-765/168, 4-13=-765/174, 5-13=-867/158

BOT CHORD 9-10=-138/860, 8-9=-138/860, 7-8=-140/870

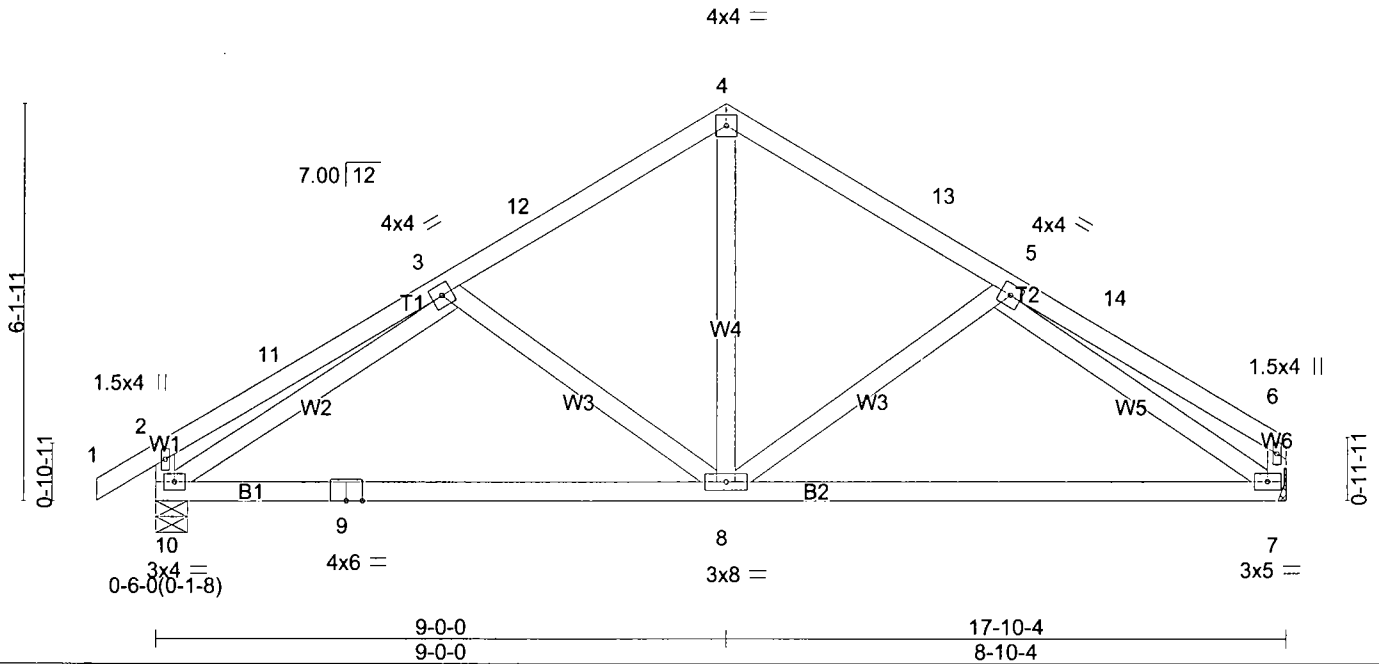
WEBS 4-8=-70/511, 5-8=-283/176, 3-8=-271/173, 3-10=-1067/150, 5-7=-1079/174

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) -0-11-4 to 2-0-12, Interior(1) 2-0-12 to 9-0-0, Exterior(2) 9-0-0 to 12-0-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=155, 7=109.
 - 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



Scale = 1:36.5



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 30.0	Plates Increase 1.15	TC 0.39	Vert(LL) -0.16 8-10 >999 240	MT20	197/144
TCDL 7.0	Lumber Increase 1.15	BC 0.91	Vert(TL) -0.41 8-10 >508 180		
BCLL 0.0	Rep Stress Incr YES	WB 0.67	Horz(TL) 0.03 7 n/a n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002	(Matrix)			Weight: 77 lb

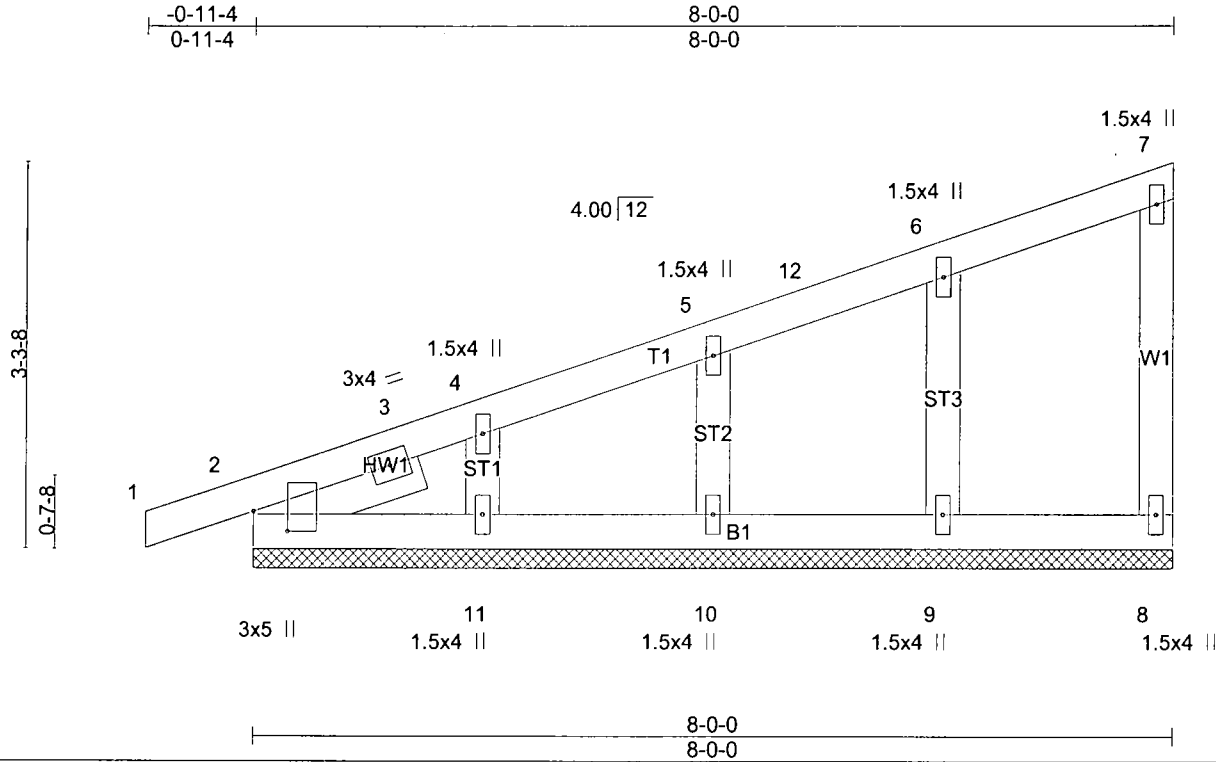
<p>LUMBER TOP CHORD 2 X 4 SPF No.1 or SPF No.2 BOT CHORD 2 X 4 SPF No.1 or SPF No.2 WEBS 2 X 4 SPF Stud *Except* W1,W6: 2 X 4 SPF No.1 or SPF No.2</p>	<p>BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.</p> <div style="border: 1px solid black; padding: 5px; margin-top: 10px;"> <p>MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.</p> </div>
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REACTIONS (lb/size) 10=908/0-6-0 (min. 0-1-8), 7=823/Mechanical
 Max Horz 10=127(LC 8)
 Max Uplift 10=-154(LC 9), 7=-108(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 3-12=-854/151, 4-12=-752/167, 4-13=-752/172, 5-13=-854/157
 BOT CHORD 9-10=-140/850, 8-9=-140/850, 7-8=-137/841
 WEBS 3-8=-272/174, 4-8=-68/497, 5-8=-265/171, 3-10=-1055/149, 5-7=-1056/172

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) -0-11-4 to 2-0-12, Interior(1) 2-0-12 to 9-0-0, Exterior(2) 9-0-0 to 12-0-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) Refer to girder(s) for truss to truss connections.
 - 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 10=154, 7=108.
 - 6) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard



Scale = 1:20.1

Plate Offsets (X,Y): [2-0-2-1,0-3-9]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 30.0	2-0-0	TC 0.07	Vert(LL)	0.00	1	n/r	120	MT20
TCDL 7.0	Plates Increase 1.15	BC 0.04	Vert(TL)	-0.00	1	n/r	90	197/144
BCLL 0.0	Lumber Increase 1.15	WB 0.06	Horz(TL)	0.00		n/a	n/a	
BCDL 10.0	Rep Stress Incr YES	(Matrix)						
	Code WISC/IRC06/TPI2002							Weight: 30 lb

LUMBER
 TOP CHORD 2 X 4 SPF No.1 or SPF No.2
 BOT CHORD 2 X 4 SPF No.1 or SPF No.2
 WEBS 2 X 4 SPF Stud
 OTHERS 2 X 4 SPF Stud
 SLIDER Left 2 X 4 SPF No.1 No.2 1-6-3

BRACING
 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 8-0-0.
 (lb) - Max Horz 2=96(LC 9)
 Max Uplift All uplift 100 lb or less at joint(s) 8, 2, 9, 10, 11
 Max Grav All reactions 250 lb or less at joint(s) 8, 2, 9, 10, 11

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- 1) Wind: ASCE 7-05; 90mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Corner(3) -0-11-4 to 2-0-0, Exterior(2) 2-0-0 to 7-10-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 2, 9, 10, 11.
- 7) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

WARNING! BCSI-B1 SUMMARY SHEET - GUIDE FOR HANDLING, INSTALLING, RESTRAINING AND BRACING OF TRUSSES

Spans over 60' may require complex permanent bracing. Please always consult a Professional Engineer.

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses*** for more detailed information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members*** for more information. All other permanent bracing design is the responsibility of the Building Designer.

The consequences of improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious personal injury or death.

El resultado de un manejo, levantamiento, instalación, restricción y arriostre incorrecto puede ser la caída de la estructura o aún peor, heridos o muertos.

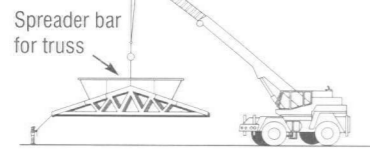
Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding. Empaques y placas de metal tienen bordes afilados. Lleve guantes y lentes protectores cuando corte los empaques.

HANDLING — MANEJO

Avoid lateral bending. — Evite la flexión lateral.

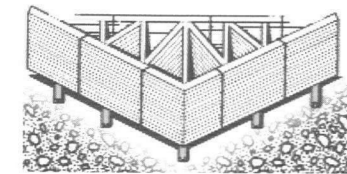


Use special care in windy weather or near power lines and airports. Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.



Use proper rigging and hoisting equipment. Use equipo apropiado para levantar e improvisar.

The contractor is responsible for properly receiving, unloading and storing the trusses at the jobsite. El contratista tiene la responsabilidad de recibir, descargar y almacenar adecuadamente los trusses en la obra.



If trusses are to be stored horizontally, place blocking of sufficient height beneath the stack of trusses at 8' to 10' on center.

For trusses stored for more than one week, cover bundles to prevent moisture gain but allow for ventilation.

Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses*** for more detailed information pertaining to handling and jobsite storage of trusses.

Si los trusses estarán guardados horizontalmente, ponga bloqueando de altura suficiente detrás de la pila de los trusses a 8 hasta 10 pies en el centro.

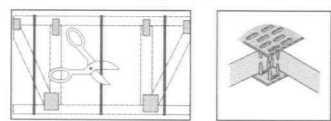
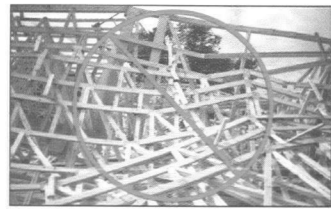
Para trusses guardados por más de una semana, cubra los paquetes para prevenir aumento de humedad pero permita ventilación.

Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses*** for more detailed information pertaining to handling and jobsite storage of trusses.

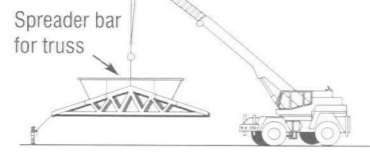
NOTAS GENERALES

Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arriostre diagonal temporales. Use las recomendaciones de manejo, instalación, restricción y arriostre temporal de los trusses. Vea el folleto BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arriostre de los Trusses de Madera Conectados con Placas de Metal*** para información más detallada.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen BCSI-B3 - Restricción/Arriostre Permanente de Cuerdas y Miembros Secundarios*** para más información. El resto de los diseños de arriostres permanentes son la responsabilidad del Diseñador del Edificio.



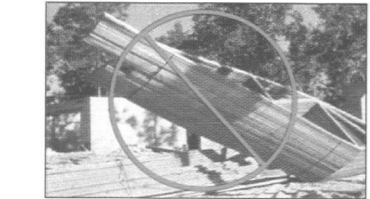
Use special care in windy weather or near power lines and airports. Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.



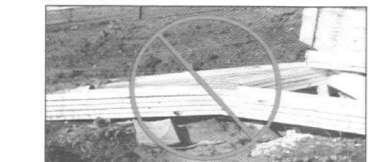
Use proper rigging and hoisting equipment. Use equipo apropiado para levantar e improvisar.



Do not store unbraced bundles upright. No almacene verticalmente los trusses sueltos.

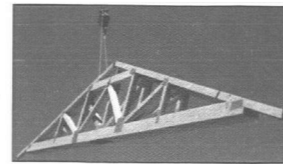


Do not store on uneven ground. No almacene en tierra desigual.



HOISTING RECOMMENDATIONS FOR TRUSS BUNDLES

Warning! Don't overload the crane. ¡Advertencia! ¡No sobrecargue la grúa! Never use banding alone to lift a bundle. Do not lift a group of individually banded bundles. Nunca use sólo los empaques para levantar un paquete. No levante un grupo de paquetes empaqueados individualmente.



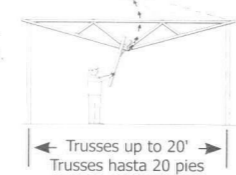
A single lift point may be used for bundles with trusses up to 45'. Two lift points may be used for bundles with trusses up to 60'. Use at least 3 lift points for bundles with trusses greater than 60'.

Warning! Do not over load supporting structure with truss bundle. ¡Advertencia! No sobrecargue la estructura apoyada con el paquete de trusses.

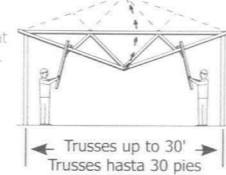
Place truss bundles in stable position. Puse paquetes de trusses en una posición estable.

INSTALLATION OF SINGLE TRUSSES BY HAND

Trusses 20' or less, support at peak.



Trusses 30' or less, support at quarter points.



HOISTING OF SINGLE TRUSSES — LEVANTAMIENTO DE TRUSSES INDIVIDUALES

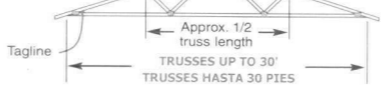
Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points. Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalado y el truss está asegurado en los soportes.

Warning! Using a single pick-point at the peak can damage the truss. ¡Advertencia! El uso de un solo lugar en el pico para levantar puede hacer daño al truss.



HOISTING RECOMMENDATIONS FOR SINGLE TRUSSES

Trusses up to 30' TRUSSES UP TO 30' TRUSSES HASTA 30 PIES



Trusses up to 60' TRUSSES UP TO 60' TRUSSES HASTA 60 PIES



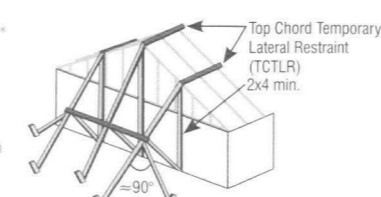
Trusses up to and over 60' TRUSSES HASTA Y SOBRE 60 PIES



TEMPORARY RESTRAINT & BRACING

Refer to BCSI-B2 Summary Sheet - Truss Installation & Temporary Restraint/Bracing*** for more information. Vea el resumen BCSI-B2 - Instalación de Trusses y Restricción/Arriostre Temporal*** para más información.

Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).



Coloque los arriostres de tierra para el primer truss directamente en línea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la tabla en la próxima columna).



Do not walk on unbraced trusses. No camine en trusses sueltos.



STEPS TO SETTING TRUSSES

1) Install ground bracing. 2) Set first truss and attach securely to ground bracing. 3) Set next 4 trusses with short member temporary lateral restraint (see below). 4) Install top chord diagonal bracing (see below). 5) Install web member plane diagonal bracing to stabilize the first five trusses (see below). 6) Install bottom chord temporary lateral restraint and diagonal bracing (see below). 7) Repeat process on groups of four trusses until all trusses are set.

1) Instale los arriostres de tierra. 2) Instale el primero truss y ate seguramente al arriostre de tierra. 3) Instale los próximos 4 trusses con restricción lateral temporal de miembro corto (vea abajo). 4) Instale el arriostre diagonal de la cuerda superior (vea abajo). 5) Instale arriostre diagonal para los planos de los miembros secundarios para estabilice los primeros cinco trusses (vea abajo). 6) Instale la restricción lateral temporal y arriostre diagonal para la cuerda inferior (vea abajo). 7) Repita este procedimiento en grupos de cuatro trusses hasta que todos los trusses estén instalados.

Refer to BCSI-B2 Summary Sheet - Truss Installation & Temporary Restraint/Bracing*** for more information.

Vea el resumen BCSI-B2 - Instalación de Trusses y Restricción/Arriostre Temporal*** para más información.

RESTRAINT/BRACING FOR ALL PLANES OF TRUSSES

This restraint & bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses.

Este método de restricción y arriostre es para todo trusses excepto trusses de cuerdas paralelas 3x2 y 4x2.

1) TOP CHORD — CUERDA SUPERIOR

Truss Span Longitud de Tramo	Top Chord Temporary Lateral Restraint (TCLR) Spacing Espaciamiento del Arriostre Temporal de la Cuerda Superior
Up to 30' Hasta 30 pies	10' o.c. max. 10 pies máximo
30' to 45' 30 a 45 pies	8' o.c. max. 8 pies máximo
45' to 60' 45 a 60 pies	6' o.c. max. 6 pies máximo
60' to 80'* 60 a 80 pies*	4' o.c. max. 4 pies máximo

*Consult a Professional Engineer for trusses longer than 60'. *Consulte a un Ingeniero Profesional para trusses más de 60 pies.

See BCSI-B2*** for TCLR options. Vea el BCSI-B2*** para las opciones de TCLR.

Refer to BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members*** for Gable End Frame restraint/bracing/reinforcement information.

Para información sobre restricción/arriostre/refuerzo para Armazones Hastiales vea el resumen BCSI-B3 - Restricción/Arriostre Permanente de Cuerdas y Miembros Secundarios.***

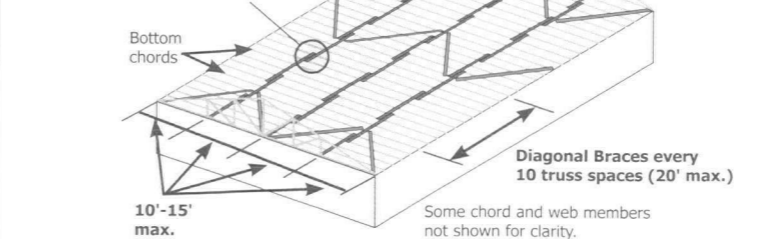
2) WEB MEMBER PLANE — PLANO DE LOS MIEMBROS SECUNDARIOS

LATERAL RESTRAINT & DIAGONAL BRACING ARE VERY IMPORTANT! LATERAL Y EL ARRIOSTRE DIAGONAL SON MUY IMPORTANTES!

10'-15' max. Same spacing as bottom chord Lateral Restraint

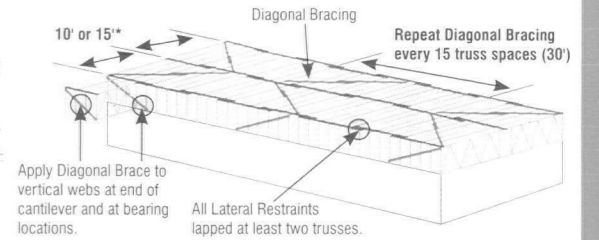
3) BOTTOM CHORD — CUERDA INFERIOR

Lateral Restraints - 2x4x12' or greater lapped over two trusses.



RESTRAINT & BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES

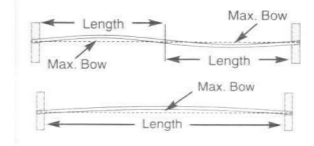
Refer to BCSI-B7 Summary Sheet - Temporary & Permanent Restraint/Bracing for Parallel Chord Trusses*** for more information.



Vea el resumen BCSI-B7 - Restricción/Arriostre Temporal y Permanente para Trusses de Cuerdas Paralelas*** para más información.

INSTALLING — INSTALACIÓN

Tolerances for Out-of-Plane. Tolerancias para Fuera-de-Plano.



Tolerances for Out-of-Plumb. Tolerancias para Fuera-de-Plomada.

Out of Plumb		Out of Plane	
D/50	D (ft.)	Max. Bow	Truss Length
1/4"	1'	3/4"	12.5'
1/2"	2'	7/8"	14.6'
3/4"	3'	1"	16.7'
1"	4'	1-1/8"	18.8'
1-1/4"	5'	1-1/4"	20.8'
1-1/2"	6'	1-3/8"	22.9'
1-3/4"	7'	1-1/2"	25.0'
2"	≥8'	1-3/4"	29.2'
		2"	≥33.3'

CONSTRUCTION LOADING — CARGA DE CONSTRUCCIÓN

Do not proceed with construction until all lateral restraint and bracing is securely and properly in place.

No proceda con la construcción hasta que todas las restricciones laterales y los arriostres estén colocados en forma apropiada y segura.

Do not exceed maximum stack heights. Refer to BCSI-B4 Summary Sheet - Construction Loading*** for more information.

No exceda las alturas máximas de montón. Vea el resumen BCSI-B4 Carga de Construcción*** para más información.



Do not overload small groups or single trusses. No sobrecargue pequeños grupos o trusses individuales.

Never stack materials near a peak. Nunca amontone los materiales cerca de un pico.

Place loads over as many trusses as possible. Coloque las cargas sobre tantos trusses como sea posible.

Position loads over load bearing walls. Coloque las cargas sobre las paredes soportantes.

ALTERATIONS — ALTERACIONES

Refer to BCSI-B5 Summary Sheet - Truss Damage, Jobsite Modifications & Installation Errors.*** Vea el resumen BCSI-B5 Daños de Trusses, Modificaciones en la Obra y Errores de Instalación.***

Do not cut, alter, or drill any structural member of a truss unless specifically permitted by the Truss Design Drawing.

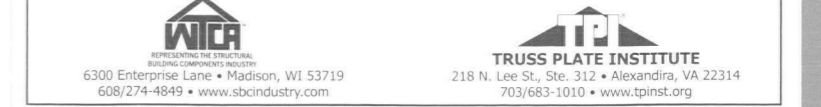
No corte, altere o perforo ningún miembro estructural de un truss, a menos que esté específicamente permitido en el Dibujo del Diseño del Truss.

Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.

Trusses que se han sobrecargado durante la construcción o han sido alterados sin la autorización previa del Fabricante de Trusses, pueden hacer nulo y sin efecto la garantía limitada del Fabricante de Trusses.

Contact the Component Manufacturer for more information or consult a Professional Engineer for assistance. To view a non-printing PDF of this document, visit www.sbindustry.com/b1.

NOTE: The Truss Manufacturer and Truss Designer rely on the presumption that the Contractor and crane operator (if applicable) are professionals with the capability to undertake the work they have agreed to do on any given project. If the Contractor believes it needs assistance in some aspect of the construction project, it should seek assistance from a competent party. The methods and procedures outlined in this document are intended to ensure that the overall construction techniques employed will put the trusses into place SAFELY. These recommendations for handling, installing, restraining and bracing trusses are based upon the collective experience of leading personnel involved with truss design, manufacture and installation, but must, due to the nature of responsibilities involved, be presented only as a GUIDE for use by a qualified Building Designer or Contractor. It is not intended that these recommendations be interpreted as superior to the Building Designer's design specification for handling, installing, restraining and bracing trusses and it does not preclude the use of other equivalent methods for restraining/bracing and providing stability for the walls, columns, floors, roofs and all the interrelated structural building components as determined by the Contractor. Thus, WTCA and TPI expressly disclaim any responsibility for damages arising from the use, application, or reliance on the recommendations and information contained herein.





Richco Structures
 Division of Richardson Industries, Inc.
 P.O. Box 904 Monroe St
 Sheboygan Falls, WI 53085
 Ph: 800-845-8910 HA 800-472-9662 DP
 Fax: 920-467-5916

Fre ? Jim T.
SHIPPING TICKET

ATTN: **JOHN H. GARY** 1-414-333-2401

QUOTE # B1000359 ORDER # J1000359

0-4 ENDS	KINGS WAY HOMES INC. 700 N. PILGRIM PARKWAY ELM GROVE, WI 53122 (262) 797-3600		Acct# KIWA00 County 40-MILWAUKEE-WI		SHIP TO	PIERRI RES. - #PIE 5302 1000 CHURCHILL LANE FOX POINT, WI	
	(262) 797-3610		<i>P-41 2/14</i>				
Job Name:	PIERRI - #PIE 5302		Quoted:	01/20/10	Salesman	Bob Happach	
Model:	BELMONT II (ROOF)		Revised:		Lot		
P.O.#		Plan Date:	Orderd:	01/20/10	Designer	AJR	
Contractor:	<i>EWP</i>		Bld Plt:	HVN	Estimator	AJR	
SCH DELIVERY 02/12/10			MAXIMUM TRUSS HEIGHT 13-01-11				
Delivery Instructions				Special Notes			

ROOF TRUSSES

LOADING INFORMATION

TCLL-TCDL-BCLL-BCDL	STRESS INCR.
30.0,7.0,0.0,10.0	1.15

ROOF TRUSS SPACING: 24.0 IN. O.C. (TYP.)

PROFILE	QTY	PITCH		TYPE ID	BASE SPAN	O/A SPAN	LUMBER		OVERHANG		CANTILEVER		HEELS		CHECKED BY	QTY SHIPPED
		PLY	TOP				BOT	TOP	BOT	LEFT	RIGHT	LEFT	RIGHT	LEFT		
	2	6.05	0.00	Jack-Open CJ01	01-06-11	01-06-11	2 X 4	2 X 4	01-10-05				02-01-07	01-04-00		
	2	6.05	0.00	Jack-Open CJ02	02-09-03	02-09-03	2 X 4	2 X 4	01-10-05				00-04-11	01-04-00		
	2	6.05	0.00	Jack-Open CJ03	01-02-10	01-02-10	2 X 4	2 X 4	01-10-05				01-11-06	01-04-00		
	2	12.00	0.00	Half Hip Truss J01G	02-06-12	02-06-12	2 X 4	2 X 4	00-11-04				02-00-09	01-04-00		
	2	12.00	0.00	Half Hip Truss J02	02-06-12	02-06-12	2 X 4	2 X 4	00-11-04				03-02-09	01-04-00		
	2	12.00	0.00	Jack-Open J03	02-06-12	02-06-12	2 X 4	2 X 4	00-11-04				00-06-07	01-04-00		
	2	7.00	0.00	Jack-Open J04	02-02-12	02-02-12	2 X 4	2 X 4	00-11-04				02-02-05	00-10-11		
	2	12.00	0.00	Half Hip Truss J05G	04-03-12	04-03-12	2 X 4	2 X 4	00-11-04				02-07-14	01-04-00		
	2	12.00	0.00	Half Hip Truss J06	04-03-12	04-03-12	2 X 4	2 X 4	00-11-04				03-09-14	01-04-00		
	2	12.00	0.00	Half Hip Truss J07	04-03-12	04-03-12	2 X 4	2 X 4	00-11-04				04-11-14	01-04-00		
	16	12.00	0.00	Jack-Open J08	04-03-12	04-03-12	2 X 4	2 X 4	00-11-04				05-07-12	01-04-00		
	1	12.00	0.00	Jack-Open J08A	04-02-00	04-02-00	2 X 4	2 X 4					05-07-12	01-05-12		
	1	12.00	0.00	Monopitch J09G	04-03-12	04-03-12	2 X 4	2 X 6					05-07-12	01-04-00		
	2	7.00	0.00	Jack-Open J10	03-03-04	03-03-04	2 X 4	2 X 4	00-11-04				00-04-15	00-10-11		
	2	12.00	0.00	Half Hip Truss J11G	03-03-12	03-03-12	2 X 4	2 X 4	00-11-04				01-10-08	01-04-00		
	2	12.00	0.00	Half Hip Truss J12	03-03-12	03-03-12	2 X 4	2 X 4	00-11-04				03-00-08	01-04-00		
	2	12.00	0.00	Half Hip Truss J13	03-03-12	03-03-12	2 X 4	2 X 4	00-11-04				04-02-08	01-04-00		
	2	12.00	0.00	Jack-Open J14	03-03-12	03-03-12	2 X 4	2 X 4	00-11-04				04-07-12	01-04-00		

CAN SHIP 2/22/10
OR LATER

CALLED ON 2/17/10

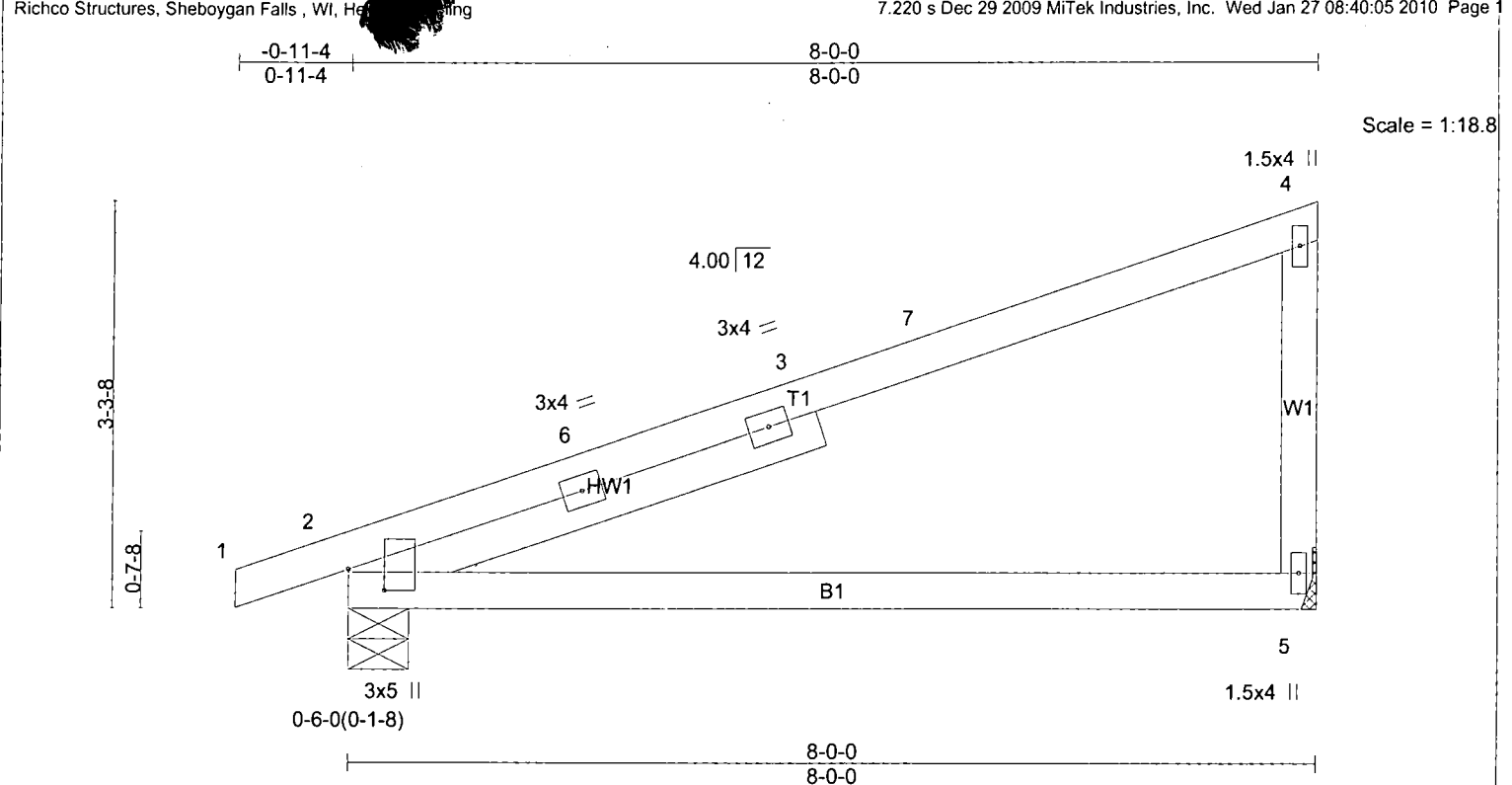


Plate Offsets (X,Y): [2:0-2-1,0-3-9]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 30.0	Plates Increase 1.15	TC 1.00	Vert(LL) -0.18	2-5	>531	240	MT20	197/144
TCDL 7.0	Lumber Increase 1.15	BC 0.42	Vert(TL) -0.44	2-5	>212	180		
BCLL 0.0	Rep Stress Incr YES	WB 0.00	Horz(TL) 0.00		n/a	n/a		
BCDL 10.0	Code WISC/IRC06/TPI2002	(Matrix)						
								Weight: 28 lb

LUMBER
 TOP CHORD 2 X 4 SPF 1650F 1.5E
 BOT CHORD 2 X 4 SPF 2100F 1.8E
 WEBS 2 X 4 SPF Stud
 SLIDER Left 2 X 4 SPF No.1 No.2 4-0-14

BRACING
 TOP CHORD Structural wood sheathing directly applied, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=365/Mechanical, 2=443/0-6-0 (min. 0-1-8)
 Max Horz 2=96(LC 9)
 Max Uplift 5=-64(LC 9), 2=-71(LC 9)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 4-5=-286/183

- NOTES**
- 1) Wind: ASCE 7-05; 90mph; TC DL=4.2psf; BC DL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; enclosed; MWFRS (all heights) and C-C Exterior(2) -0-11-4 to 2-0-12, Interior(1) 2-0-12 to 7-10-4 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 3) Refer to girder(s) for truss to truss connections.
 - 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 2.
 - 5) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

① Fristop (Myi Bortoluzzi)

② Hanger - Hanger

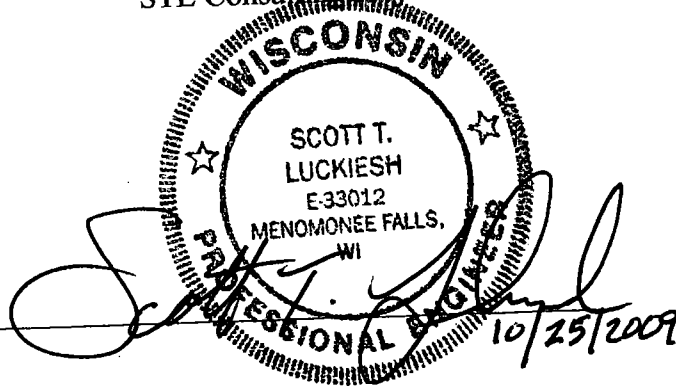
③

*Structural
Calculations for:*

Pierrri Residence
Fox Point, WI

Kings Way Homes, L.L.C.
Client # PIE5302
Model: The Belmont II

STL Consulting Project No. 09021



KINGS WAY HOMES

Pierri Residence - The Belmont II

DESIGN CRITERIA

Roof Loading

Dead Load	22	psf
Live Load	30	psf (drift loading as required per Wisconsin Code)

Floor Loading

Dead Load (typical)	10	psf
Tile Flooring	20	psf
Partitions	10	psf
Live Load (typical)	40	psf
Bathrooms	50	psf

Soil (Assumed)

Lateral Pressure	50	psf/ft
Bearing Capacity	3000	psf

Wind

MWFRS	20	psf
Components and Cladding	20	psf

Wood (2005 National Design Specifications for Wood Construction):

No.1/No.2 or Better S-P-F	Stud Grade S-P-F
Fb = 875 psi	Fb = 675 psi
Ft = 450 psi	Ft = 350 psi
Fv = 135 psi	Fv = 135 psi
Fc perp = 425 psi	Fc perp = 425 psi
Fc para = 1150 psi	Fc para = 725 psi
E = 1.4E6 psi	E = 1.2E6 psi

Engineered Lumber:

2.0E VERSA-LAM LVL Beams and Headers	1.7E VERSA-LAM Columns and Studs
E = 2.0E6 psi	E = 1.7E6 psi
Fb = 3100 psi	Fb = 2650 psi
Fv = 285 psi	Fv = 285 psi

Masonry

f _m	1350	psi
----------------	------	-----

Concrete

f _c	3000	psi
f _y (reinforcement)	60,000	psi

Structural Steel

ASTM A36 F _y =	36,000	psi
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CUSTOMER INFORMATION

Customer Name: PEDRO & LAURA PIERRI
 Job Number: PIE5302
 Lot Number: PIE5302

JOB SITE INFORMATION

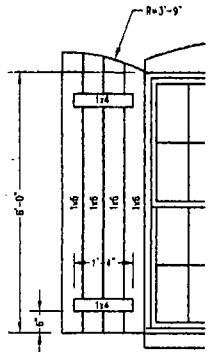
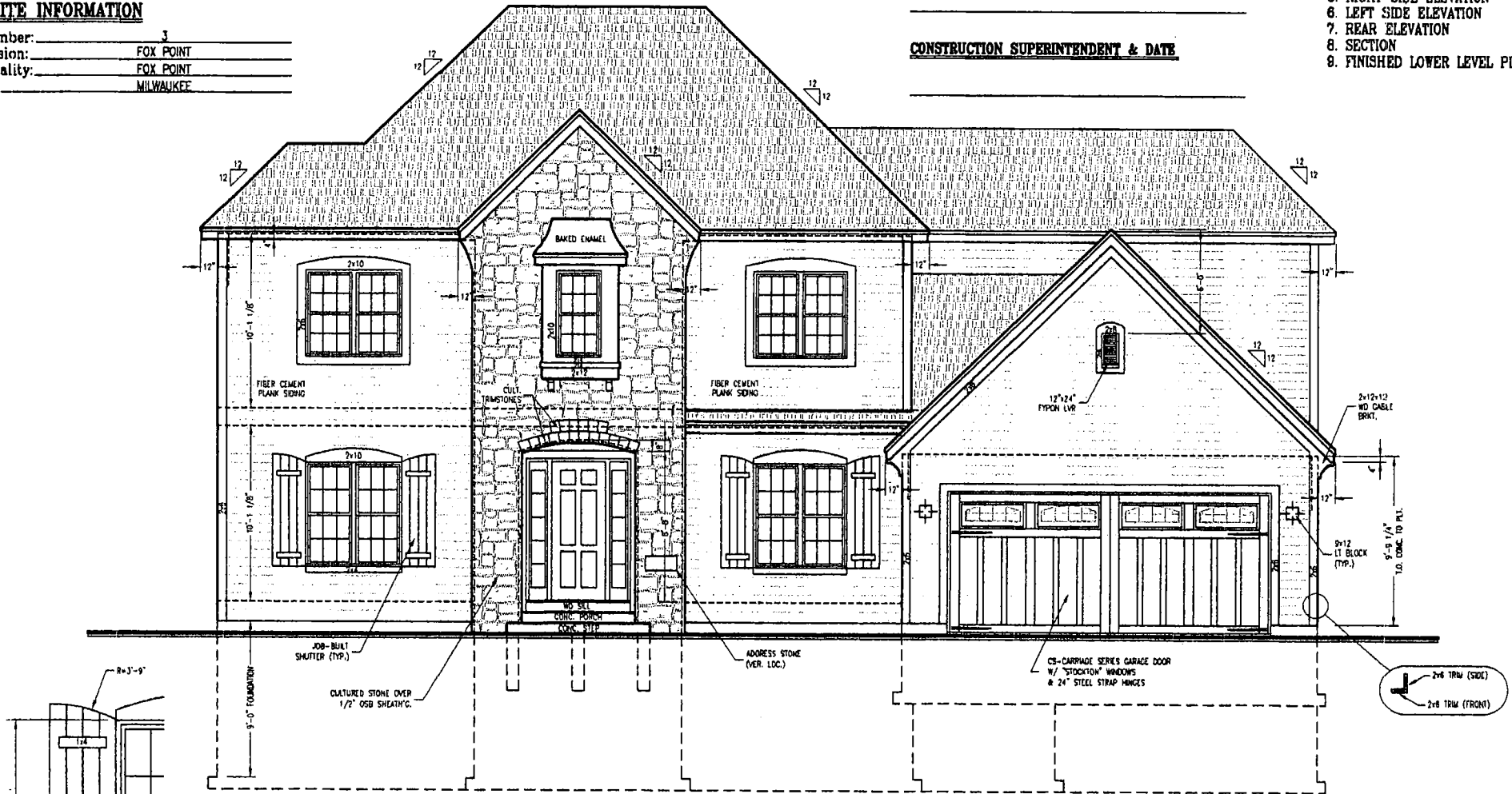
Lot Number: 3
 Subdivision: FOX POINT
 Municipality: FOX POINT
 County: MILWAUKEE

CUSTOMER SIGNATURE & DATE

CONSTRUCTION SUPERINTENDENT & DATE

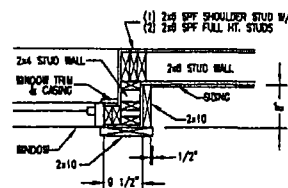
SHEET INDEX

1. FRONT ELEVATION
2. FIRST FLOOR PLAN
3. SECOND FLOOR PLAN
4. BASEMENT PLAN
5. RIGHT SIDE ELEVATION
6. LEFT SIDE ELEVATION
7. REAR ELEVATION
8. SECTION
9. FINISHED LOWER LEVEL PLAN



JOB-BUILT SHUTTER

SCALE: 1/2" = 1'-0"



BOX WINDOW DTL.

SCALE: 3/4" = 1'-0"

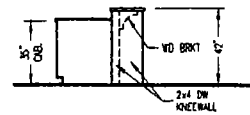
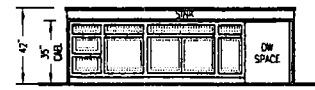
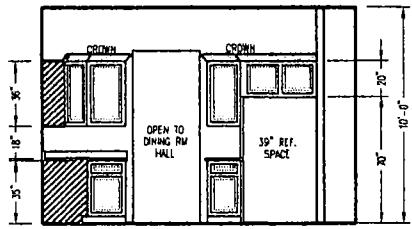
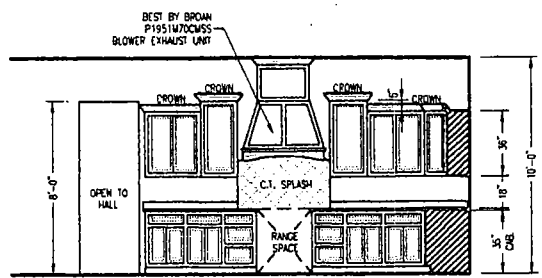
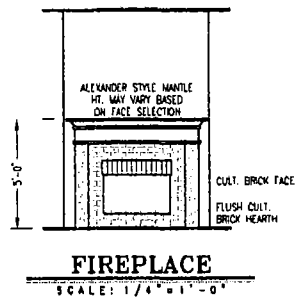
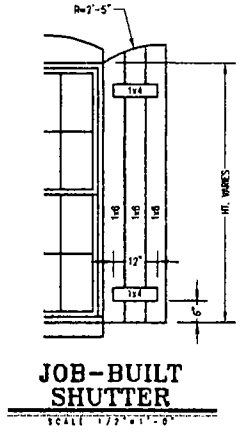
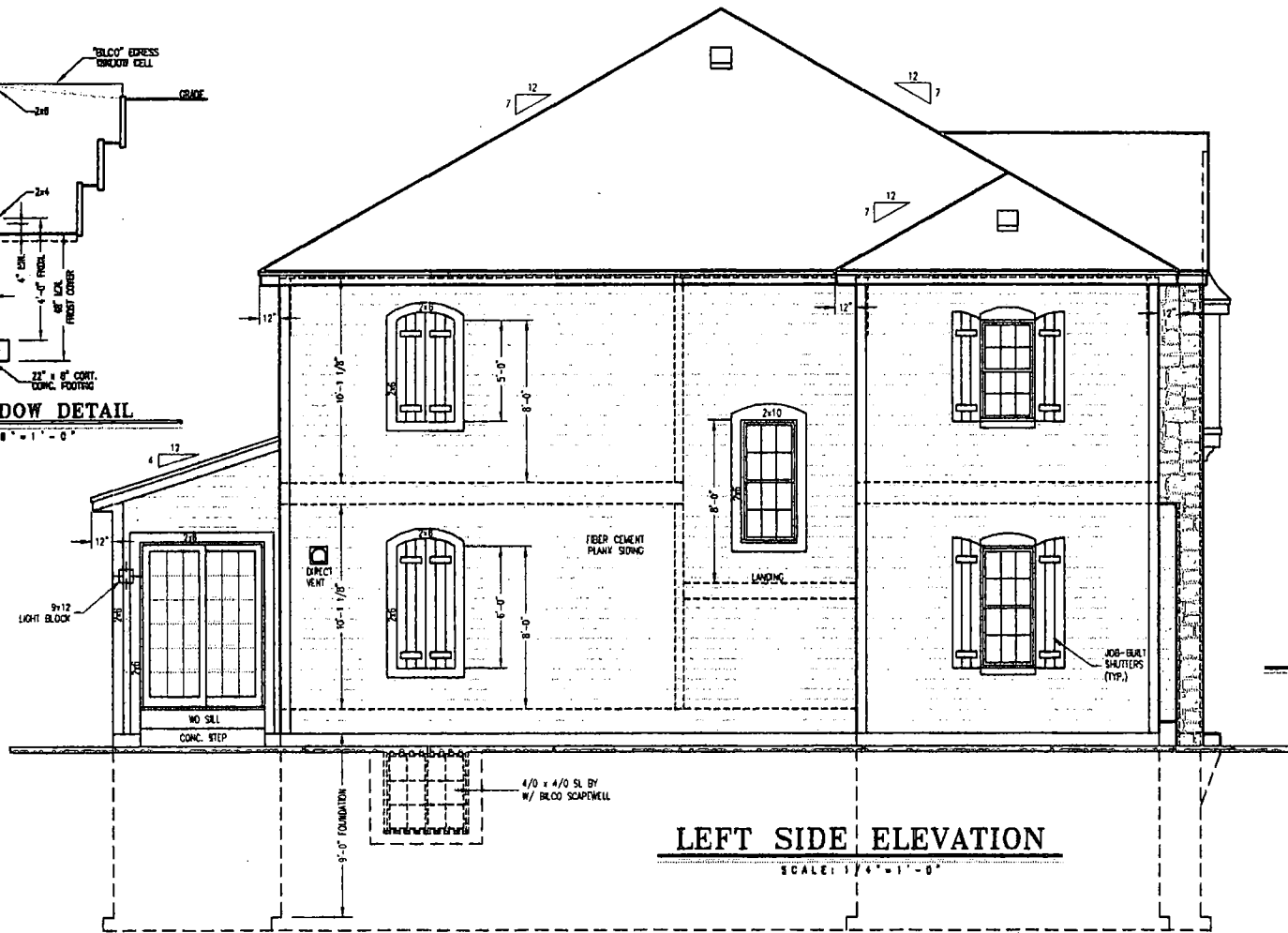
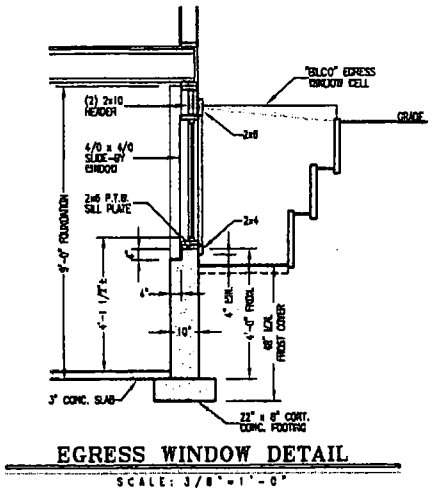
FRONT ELEVATION

SCALE: 1/4" = 1'-0"

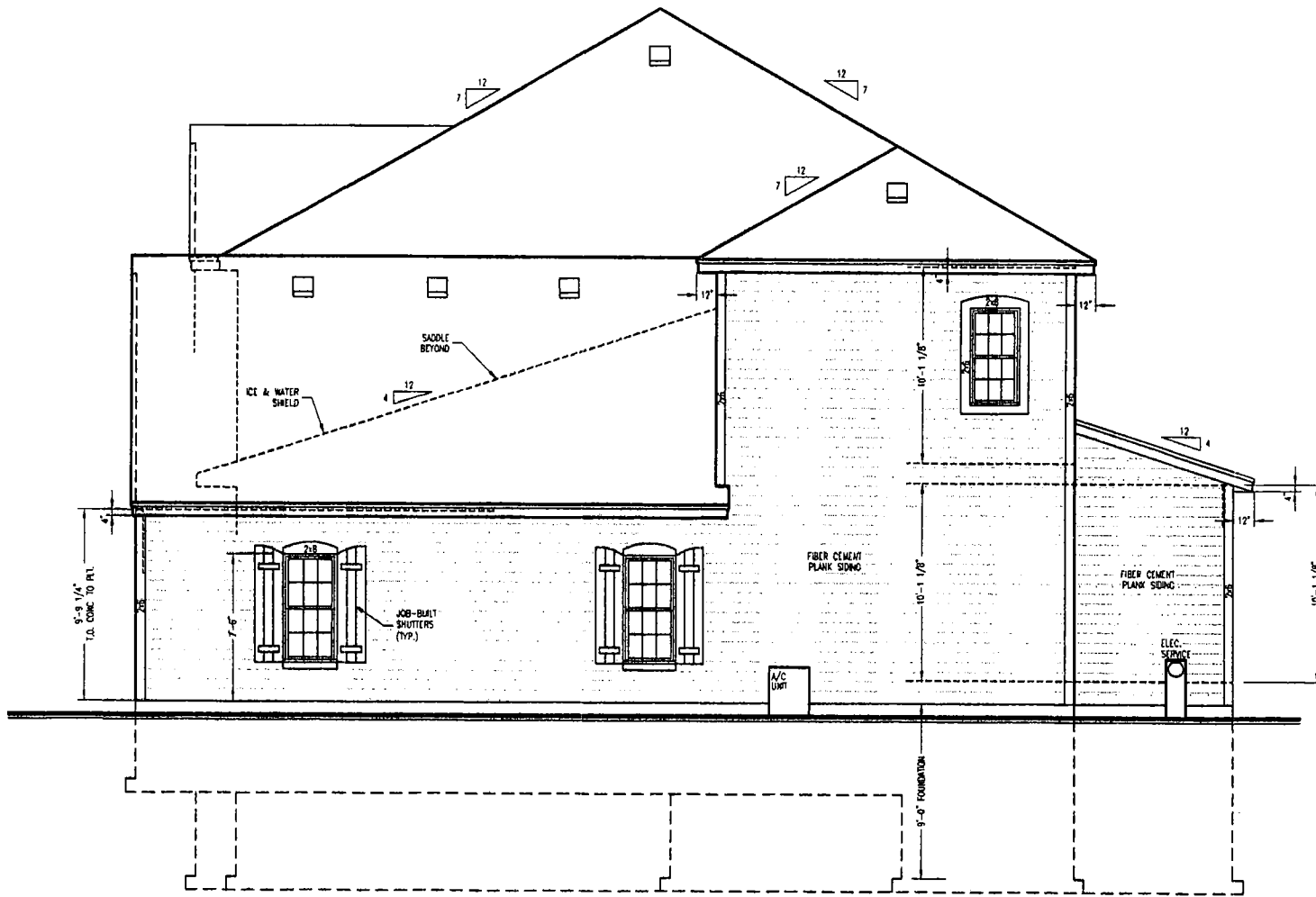
FIRST FLOOR: 1980 SF
 SECOND FLOOR: 1885 SF
 TOTAL: 3865 SF

ENERGY STAR HOME

<p>KINGS WAY HOMES, L.L.C. 700 PILGRIM PARKWAY SUITE 100 ELM GROVE, WI. Phone (262) 797-3600</p>	<p>MODEL THE BELMONT II</p>
	<p>RESIDENCE FOR: PEDRO & LAURA PIERRI</p>
<p>SQ. FT. 3955</p>	<p>DATE REVISED 10/09/09</p>
<p>PHONE (262) 797-3600</p>	<p>SHEET NO. # 1</p>

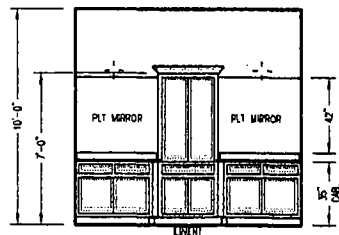


KITCHEN CABINET ELEVATIONS
SCALE: 1/4" = 1'-0"



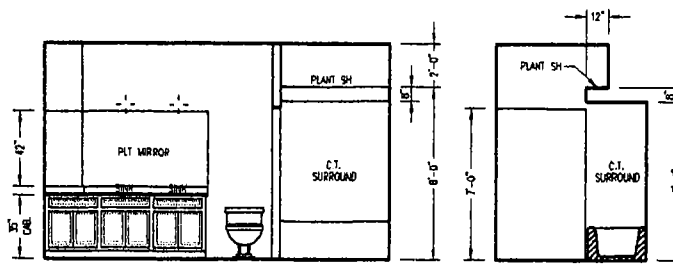
RIGHT SIDE ELEVATION

SCALE: 1/4" = 1'-0"



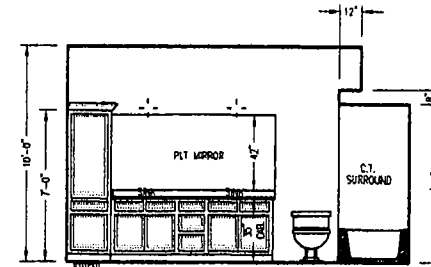
MASTER BATH

SCALE: 1/4" = 1'-0"



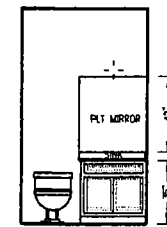
BATH 2

SCALE: 1/4" = 1'-0"



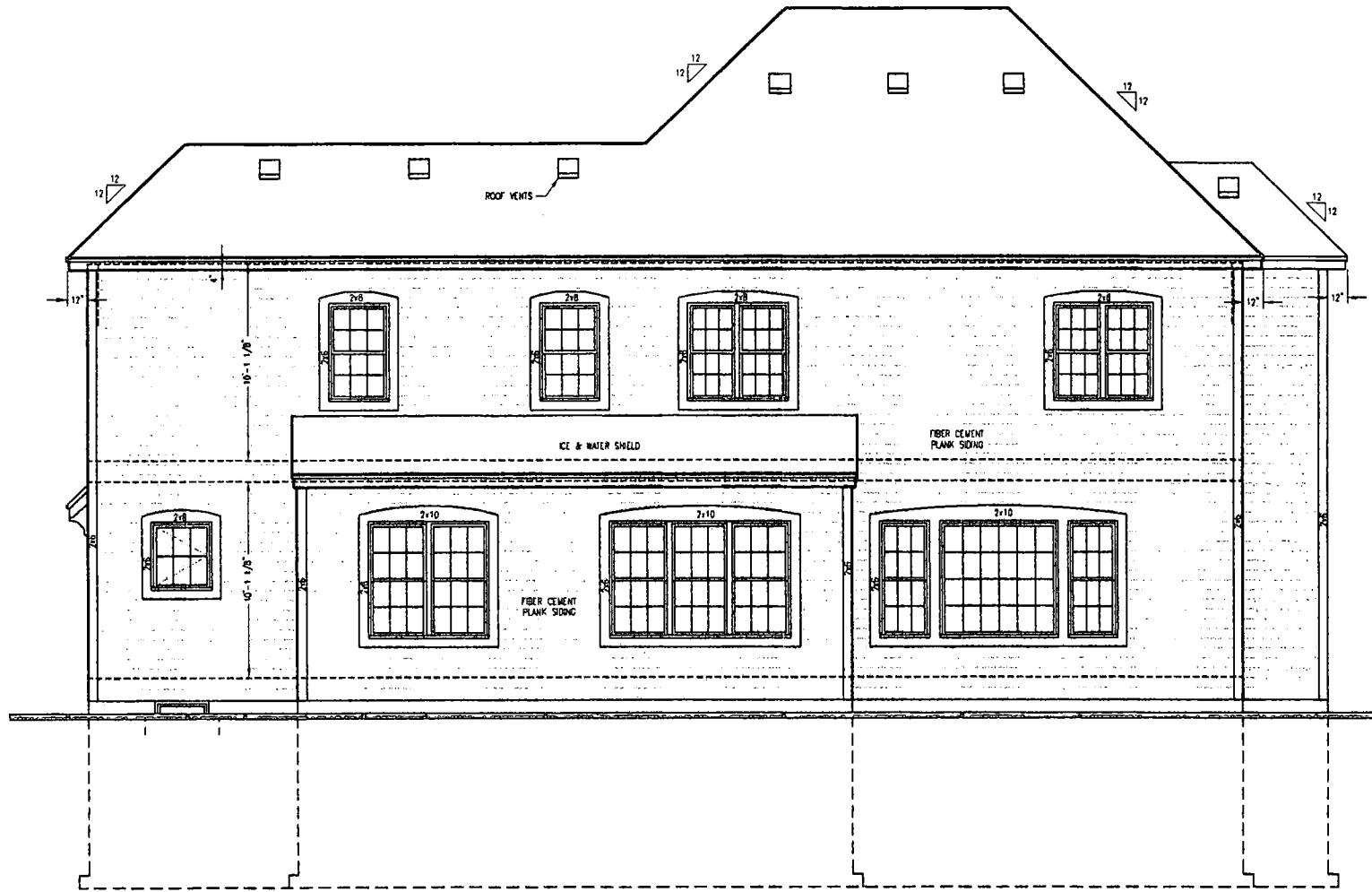
BATH 3

SCALE: 1/4" = 1'-0"



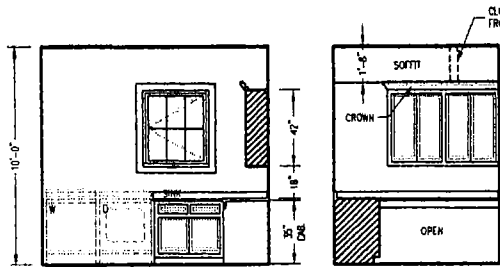
POWDER RM.

SCALE: 1/4" = 1'-0"



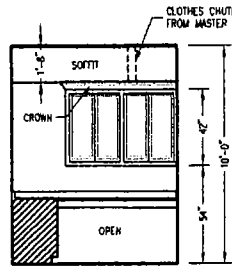
REAR ELEVATION

SCALE: 1/4" = 1'-0"



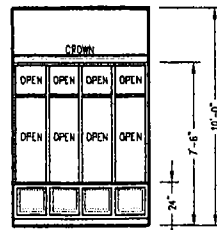
LAUNDRY RM.

SCALE: 1/4" = 1'-0"



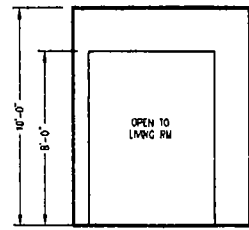
ENTRY

SCALE: 1/4" = 1'-0"



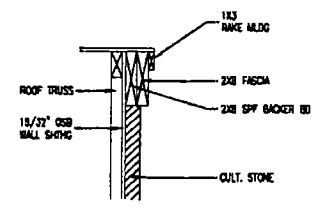
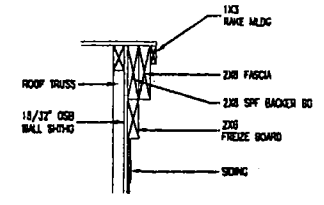
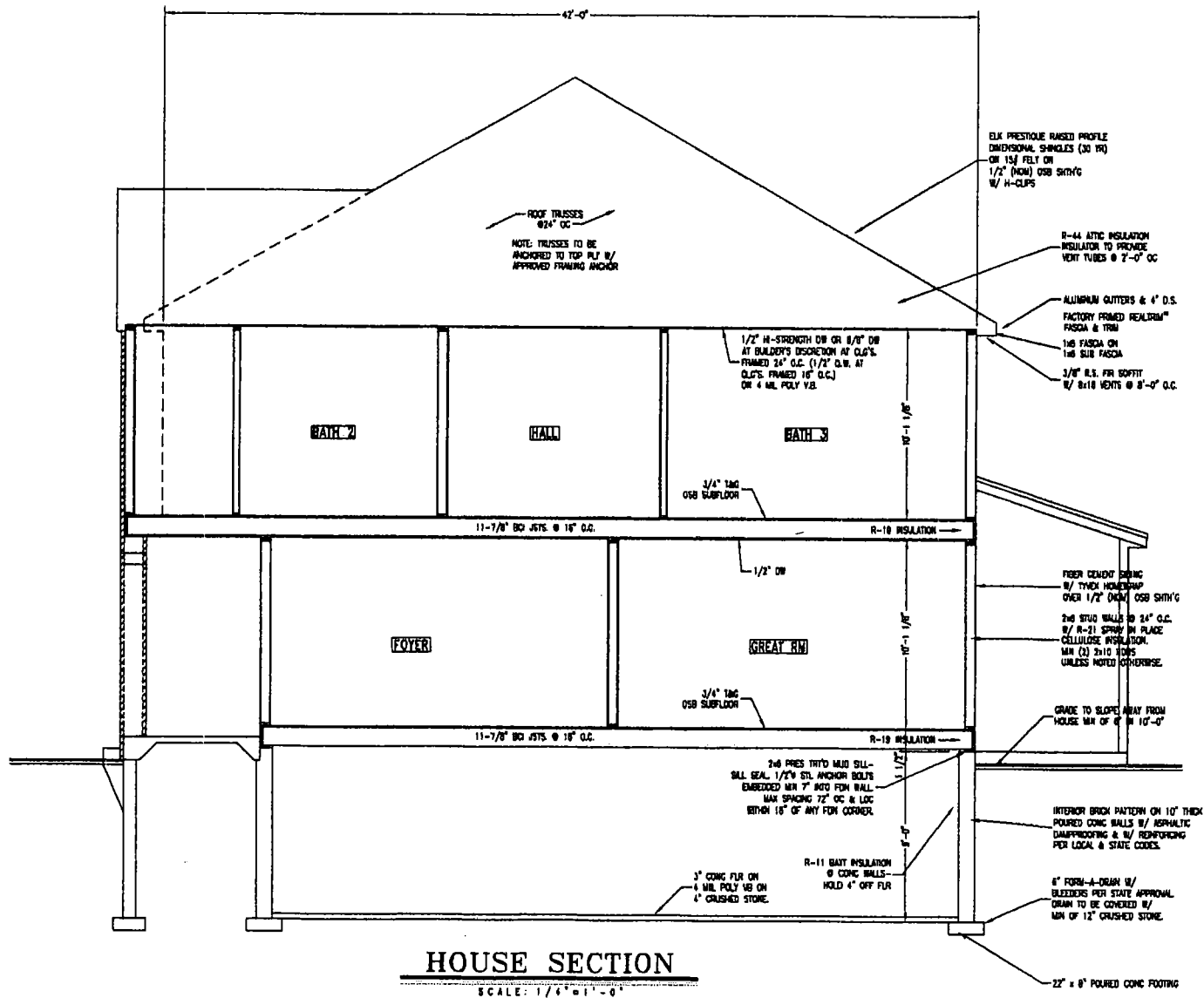
FOYER TO LIVING RM

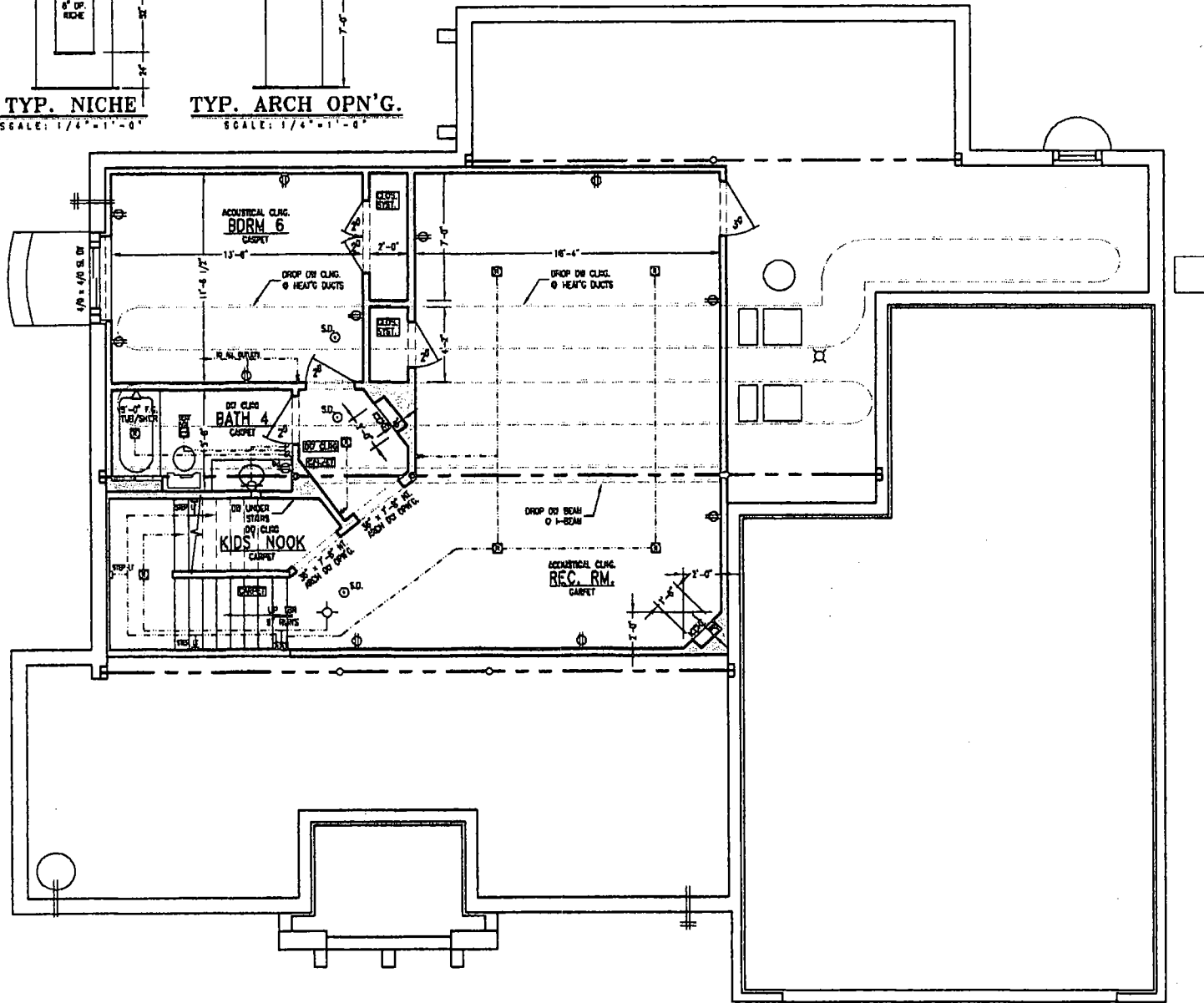
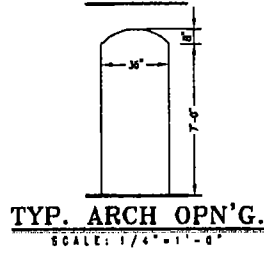
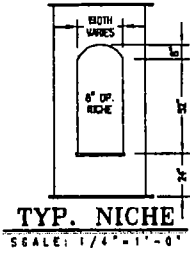
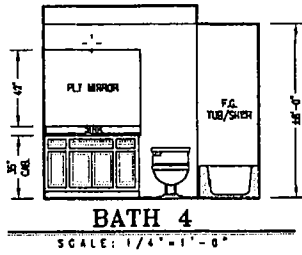
SCALE: 1/4" = 1'-0"



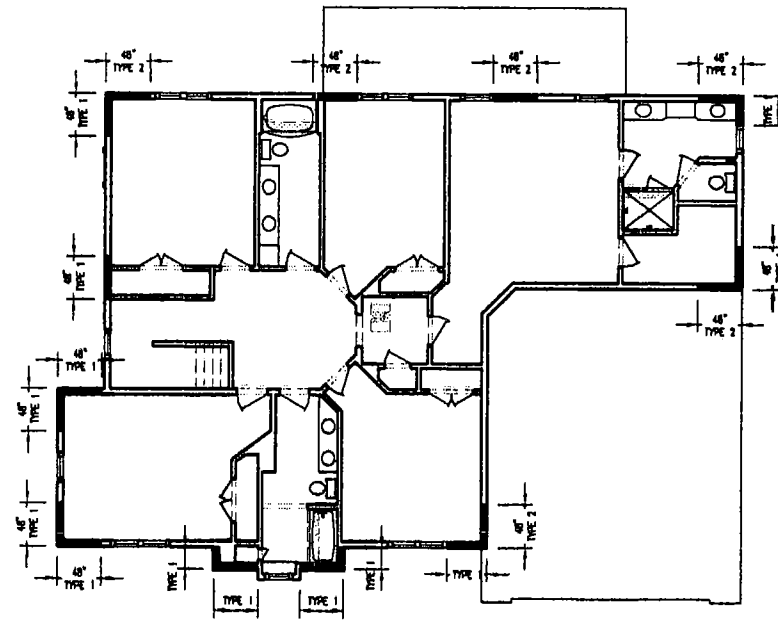
FOYER TO DINING RM

SCALE: 1/4" = 1'-0"



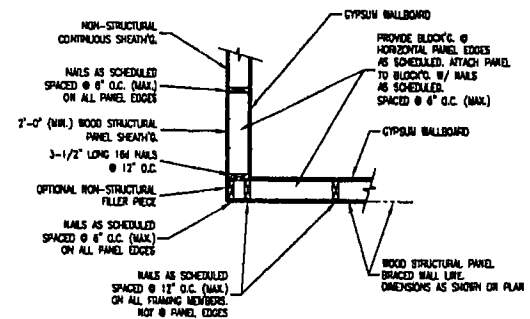


896 S.F.
FINISHED BASEMENT PLAN
SCALE: 1/4" = 1'-0"



SECOND FLOOR SHEAR WALL BRACING PLAN

SCALE: 1/8" = 1'-0"

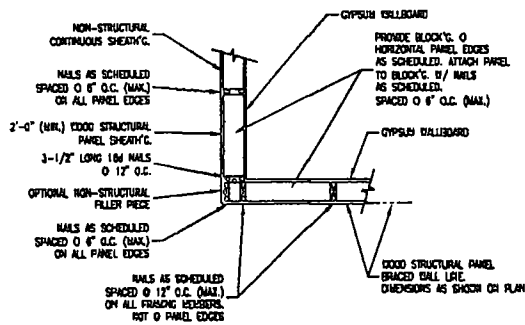


WALL TYPES 1 & 2

TYP. EXTERIOR CORNER FRAMING FOR CONTINUOUS STRUCTURAL PANEL SHEATH'G.

SCALE: 1/2" = 1'-0"

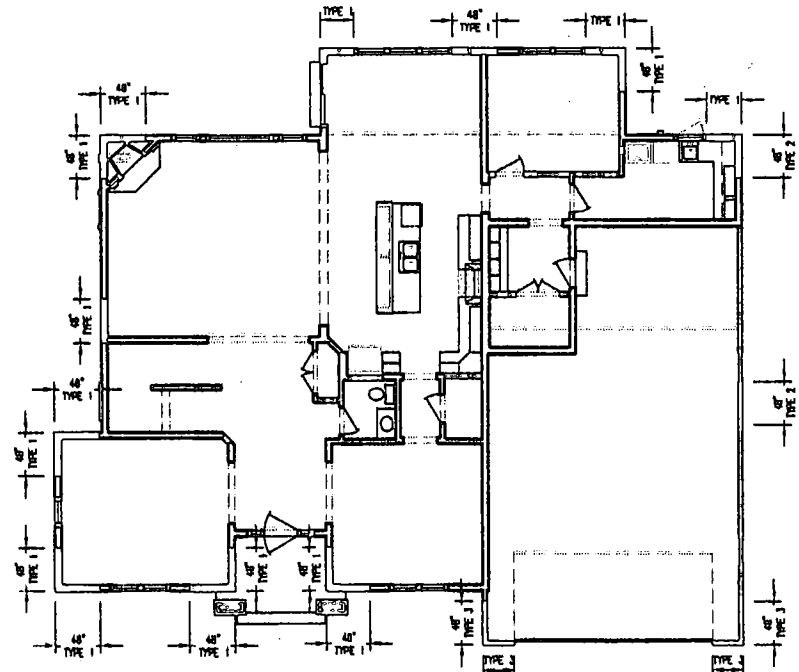
- | |
|--|
| <p>TYPE 1 BRACED WALL PANEL:
 FASTEN TO STUDS USING 8d COMMON OR DEFORMED NAILS (1-1/2") OR STAPLES
 SPACING (MAX.)
 6" @ SHEATHING, PANEL EDGES
 12" @ INTERMEDIATE SUPPORTS
 SEE DETAIL</p> |
| <p>TYPE 2 BRACED WALL PANEL:
 FASTEN TO STUDS USING 8d COMMON OR DEFORMED NAILS (1-1/2") OR STAPLES
 SPACING (MAX.)
 6" @ SHEATHING, PANEL EDGES
 12" @ INTERMEDIATE SUPPORTS
 PROVIDE 2x HORIZONTAL BLOCK'G. @ ALL HORIZONTAL EDGES OF SHEATH'G.
 SEE DETAIL</p> |



WALL TYPES 1 & 2

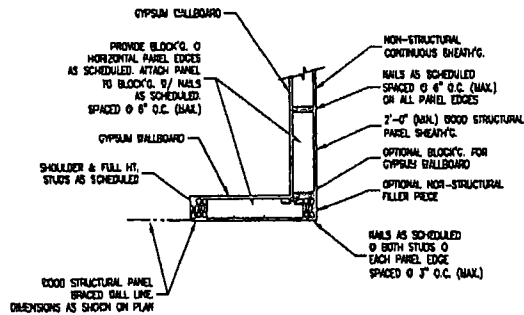
TYP. EXTERIOR CORNER FRAMING FOR CONTINUOUS STRUCTURAL PANEL SHEATH'G.

SCALE: 1/2" = 1'-0"



FIRST FLOOR SHEAR WALL BRACING PLAN

SCALE: 1/8" = 1'-0"

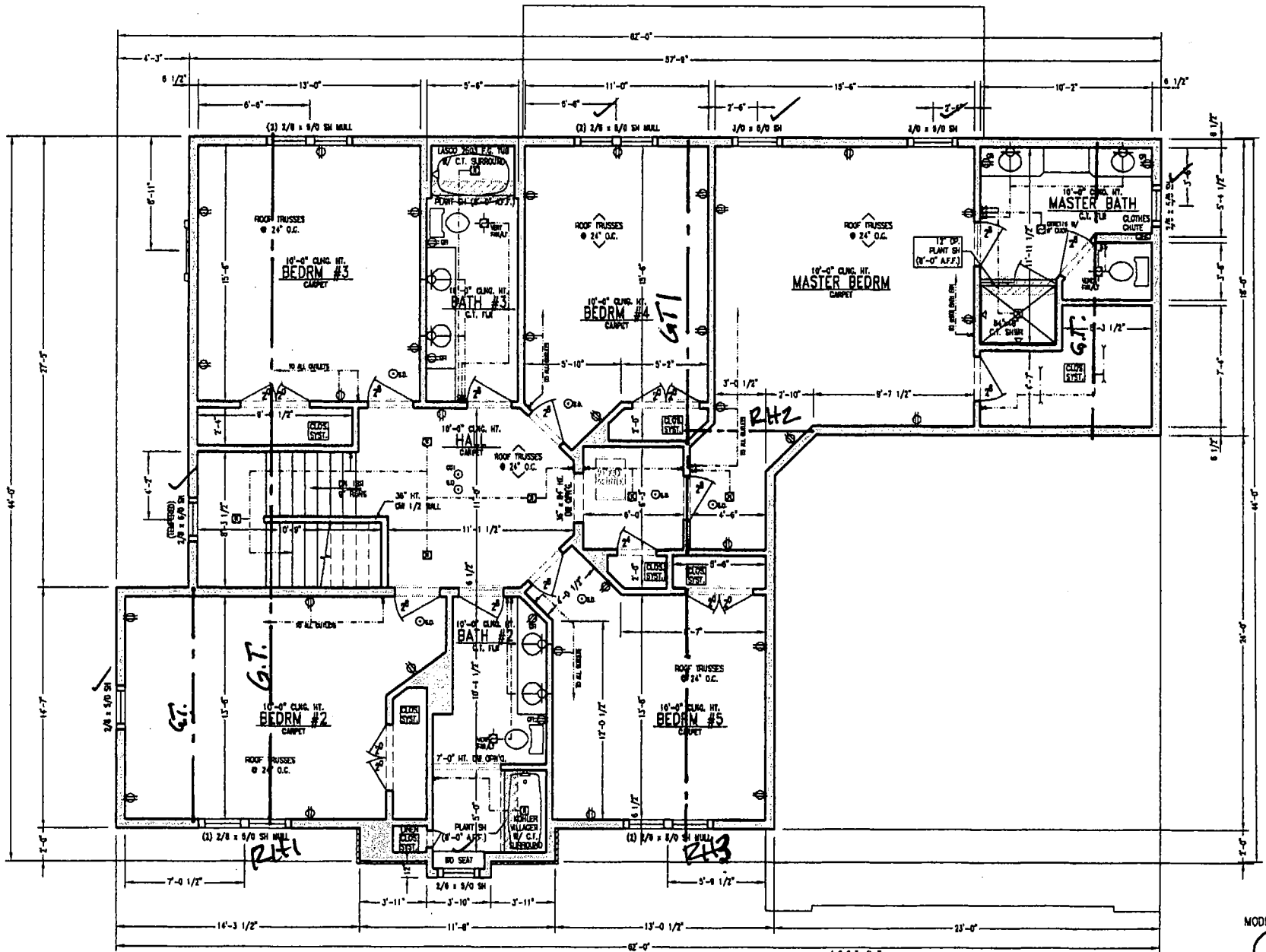


WALL TYPE 3

GARAGE DOOR CORNER FRAMING FOR CONTINUOUS STRUCTURAL PANEL SHEATH'G.

SCALE: 1/2" = 1'-0"

- TYPE 1 BRACED WALL PANEL:**
 FASTEN TO STUDS USING 8d COMMON OR DEFORMED NAILS (1-1/2") OR STAPLES
 SPACING (MAX.)
 6" @ SHEATHING PANEL EDGES
 12" @ INTERMEDIATE SUPPORTS
 SEE DETAIL
- TYPE 2 BRACED WALL PANEL:**
 FASTEN TO STUDS USING 8d COMMON OR DEFORMED NAILS (1-1/2") OR STAPLES
 SPACING (MAX.)
 6" @ SHEATHING PANEL EDGES
 12" @ INTERMEDIATE SUPPORTS
 PROVIDE 2x HORIZONTAL BLOCK'G. @ ALL HORIZONTAL EDGES OF SHEATH'G.
 SEE DETAIL
- TYPE 3 BRACED WALL PANEL:**
 FASTEN TO STUDS USING 8d COMMON OR DEFORMED NAILS (1-1/2") OR STAPLES
 SPACING (MAX.)
 6" @ SHEATHING PANEL EDGES
 12" @ INTERMEDIATE SUPPORTS
 SEE DETAIL

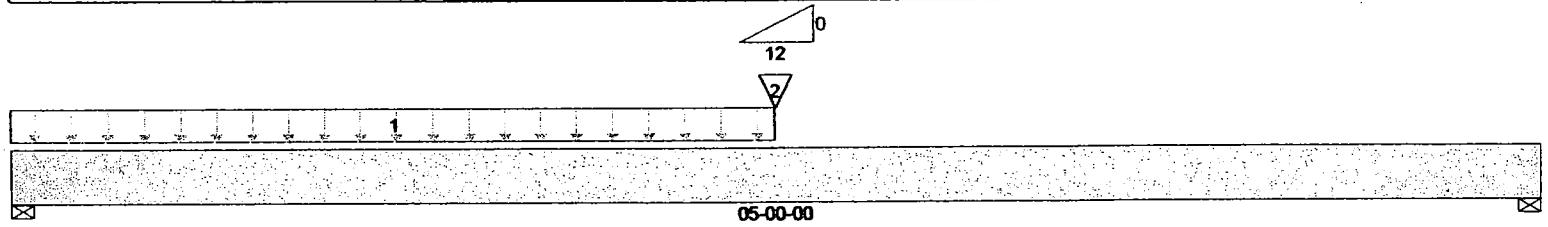


NOTE:
 • 8'-0" HT. SOLID CORE DOORS THROUGHOUT
 • ROUND CORNER BEAD THROUGHOUT
 • SAND TEXTURE W/ (1) COAT PRIMER THROUGHOUT

1988 S.F.
SECOND FLOOR PLAN
 SCALE: 1/4" = 1'-0"

Job Name:
Address:
City, State, Zip: ,
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1040

File Name: Pierri
Description: RH1
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



B0, 3-1/2" DL 2,258 lbs SL 2,162 lbs
B1, 3-1/2" DL 1,369 lbs SL 1,303 lbs

Total Horizontal Product Length = 05-00-00

Load Summary					Live	Dead	Snow	Wind	Roof Live		
Tag	Description	Load Type	Ref.	Start	End	100%	90%	115%	133%	125%	Trib.
1	Standard Load	Unf. Area (psf)	Left	00-00-00	02-06-00		31	30			21-00-00
2	GT	Conc. Pt. (lbs)	Left	02-06-00	02-06-00	1,953	1,890				n/a

Controls Summary	Value	% Allowable	Duration	Case	Span
Pos. Moment	6,031 ft-lbs	37.6%	115%	3	1 - Internal
End Shear	3,022 lbs	41.6%	115%	3	1 - Left
Total Load Defl.	L/1,420 (0.038")	12.7%		3	1
Live Load Defl.	L/2,901 (0.019")	12.4%		3	1
Max Defl.	0.038"	3.8%		3	1
Span / Depth	5.7	n/a			1

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

Bearing Supports	Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0	Wall/Plate 3-1/2" x 3-1/2"	4,420 lbs	84.9%	48.1%	Spruce-Pine-Fir
B1	Wall/Plate 3-1/2" x 3-1/2"	2,672 lbs	51.3%	29.1%	Spruce-Pine-Fir

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Cautions

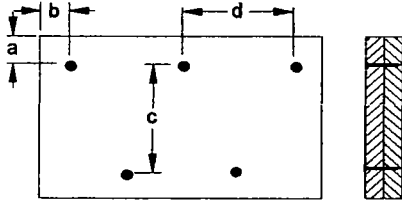
For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur.
For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load.

Notes

Design meets Code minimum (L/180) Total load deflection criteria.
Design meets User specified (L/360) Live load deflection criteria.
Design meets arbitrary (1") Maximum load deflection criteria.

Job Name:
 Address:
 City, State, Zip: ,
 Customer: Kings Way Homes, L.L.C.
 Code reports: ESR-1040

File Name: Pierr
 Description: RH1
 Specifier: stl
 Designer: stl
 Company: STL Consulting
 Misc:

Connection Diagram

a minimum = 2" c = 5-1/2"
 b minimum = 3" d = 12"

Connection design assumes point load is 'top-loaded'. For connection design of 'side-loaded' point loads, please consult a technical representative or professional of Record.

Member has no side loads.

Concentrated loads are not considered in side load analysis.

Connectors are: 16d Common Nails

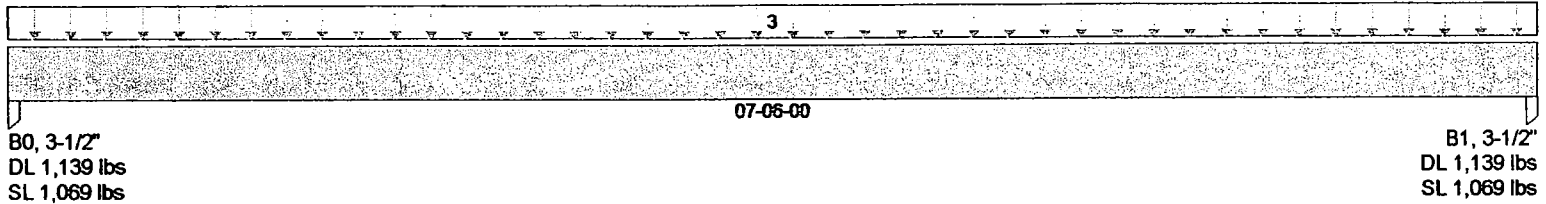
Disclosure

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Job Name:
Address:
City, State, Zip:
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1040

File Name: Pierr
Description: RH2
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



Total Horizontal Product Length = 07-06-00

Load Summary	Tag Description	Load Type	Ref.	Start	End	Live 100%	Dead 90%	Snow 115%	Wind 133%	Roof Live 125%	Trib.
	3 LEFT	Unf. Area (psf)	Left	00-00-00	07-06-00		31	30			09-06-00

Controls Summary	Value	% Allowable	Duration	Case	Span
Pos. Moment	3,650 ft-lbs	22.7%	115%	3	1 - Internal
End Shear	1,570 lbs	21.6%	115%	3	1 - Left
Total Load Defl.	L/1,297 (0.065")	13.9%		3	1
Live Load Defl.	L/2,681 (0.032")	13.4%		3	1
Max Defl.	0.065"	6.5%		3	1
Span / Depth	8.9	n/a			1

Disclosure
Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

Bearing Supports	Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0 Post	3-1/2" x 3-1/2"	2,208 lbs	24.9%	24.0%	Spruce-Pine-Fir
B1 Post	3-1/2" x 3-1/2"	2,208 lbs	24.9%	24.0%	Spruce-Pine-Fir

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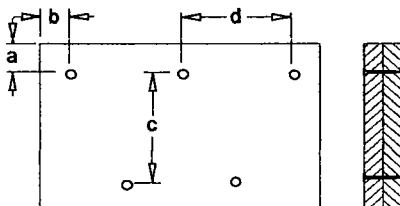
Cautions

For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur.
For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load.

Notes

Design meets Code minimum (L/180) Total load deflection criteria.
Design meets User specified (L/360) Live load deflection criteria.
Design meets arbitrary (1") Maximum load deflection criteria.

Connection Diagram

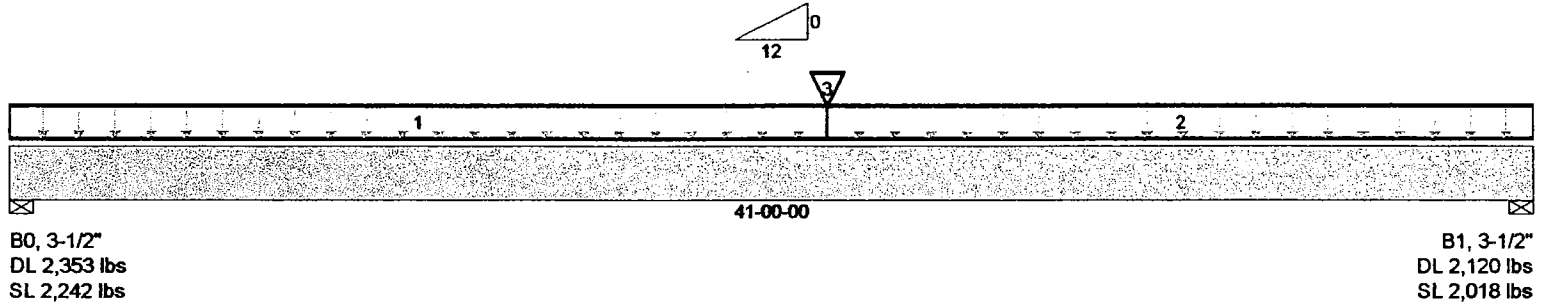


a minimum = 2" c = 5-1/2"
b minimum = 3" d = 12"

Member has no side loads.
Connectors are: 16d Common Nails

Job Name:
Address:
City, State, Zip:
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1040

File Name: Pierr
Description: GT1
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



Total Horizontal Product Length = 41-00-00

Load Summary					Live	Dead	Snow	Wind	Roof Live		
Tag	Description	Load Type	Ref.	Start	End	100%	90%	115%	133%	125%	Trib.
1	FRONT	Unf. Area (psf)	Left	00-00-00	22-00-00		31	30			03-00-00
2	BACK	Unf. Area (psf)	Left	22-00-00	41-00-00		31	30			02-00-00
3	GT	Conc. Pt. (lbs)	Left	22-00-00	22-00-00	1,178		1,140			n/a

Controls Summary	Value	% Allowable	Duration	Case	Span
Pos. Moment	55,329 ft-lbs	4546.2%	115%	3	1 - Internal
End Shear	4,487 lbs	335.3%	115%	3	1 - Left
Total Load Defl.	L/0 (1,208.243")	44703.8%		3	1
Live Load Defl.	L/4 (590.095")	43665.8%		3	1
Max Defl.	1,208.243"	120824.3%		3	1
Span / Depth	139.0	n/a			1

Disclosure
Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

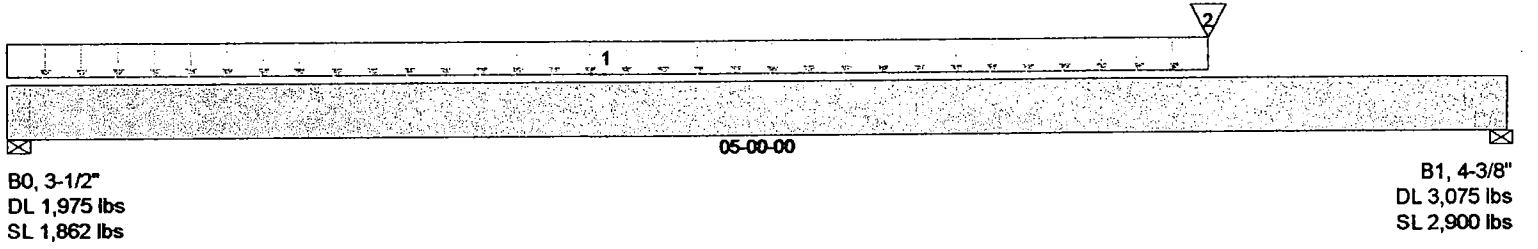
Bearing Supports	Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0	Wall/Plate 3-1/2" x 1-3/4"	4,595 lbs	176.5%	100.0%	Spruce-Pine-Fir
B1	Wall/Plate 3-1/2" x 1-3/4"	4,138 lbs	159.0%	90.1%	Spruce-Pine-Fir

Cautions
 Member has insufficient Pos. Moment resistance to carry loads.
 Member has insufficient End Reaction resistance to carry loads.
 Member has insufficient End Shear resistance to carry the loads.
 Member is insufficient to carry loads for Code minimum load deflection at limit of L/180.
 Member is insufficient to carry loads for Code minimum Live load deflection at limit of L/240.
 Member is insufficient to carry loads for Maximum load deflection at limit of 1".
 Bearing length at bearing B0 should be at least 6-3/16".
 Bearing B0 cannot support a load of 4,595 lbs.
 Bearing length at bearing B1 should be at least 5-5/8".
 Bearing B1 cannot support a load of 4,138 lbs.
 For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur.
 For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load.

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Job Name:
Address:
City, State, Zip: ,
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1040

File Name: Pierri
Description: RH3
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



Total Horizontal Product Length = 05-00-00

Load Summary					Live	Dead	Snow	Wind	Roof Live	Trib.	
Tag	Description	Load Type	Ref.	Start	End	100%	90%	115%	133%	125%	
1	Standard Load	Unf. Area (psf)	Left	00-00-00	04-00-00		31	30			21-00-00
2	GT1	Conc. Pt. (lbs)	Left	04-00-00	04-00-00		2,353	2,242			n/a

Controls Summary	Value	% Allowable	Duration	Case	Span
Pos. Moment	4,819 ft-lbs	30.0%	115%	3	1 - Internal
End Shear	-4,847 lbs	66.7%	115%	3	1 - Right
Total Load Defl.	L/1,533 (0.035")	11.7%		3	1
Live Load Defl.	L/3,156 (0.017")	11.4%		3	1
Max Defl.	0.035"	3.5%		3	1
Span / Depth	5.6	n/a			1

Disclosure
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Bearing Supports	Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0	Wall/Plate 3-1/2" x 3-1/2"	3,837 lbs	73.7%	41.8%	Spruce-Pine-Fir
B1	Wall/Plate 4-3/8" x 3-1/2"	5,976 lbs	91.8%	52.0%	Spruce-Pine-Fir

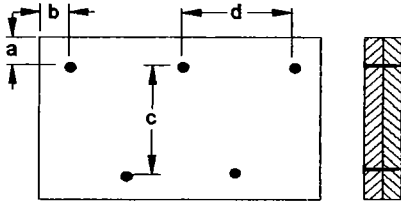
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Cautions
For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur.
For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load.

Notes
Design meets Code minimum (L/180) Total load deflection criteria.
Design meets User specified (L/360) Live load deflection criteria.
Design meets arbitrary (1") Maximum load deflection criteria.

Job Name:
 Address:
 City, State, Zip: ,
 Customer: Kings Way Homes, L.L.C.
 Code reports: ESR-1040

File Name: Pierr
 Description: RH3
 Specifier: stl
 Designer: stl
 Company: STL Consulting
 Misc:

Connection Diagram

a minimum = 2" c = 5-1/2"
 b minimum = 3" d = 12"

Connection design assumes point load is 'top-loaded'. For connection design of 'side-loaded' point loads, please consult a technical representative or professional of Record.

Member has no side loads.

Concentrated loads are not considered in side load analysis.

Connectors are: 16d Common Nails

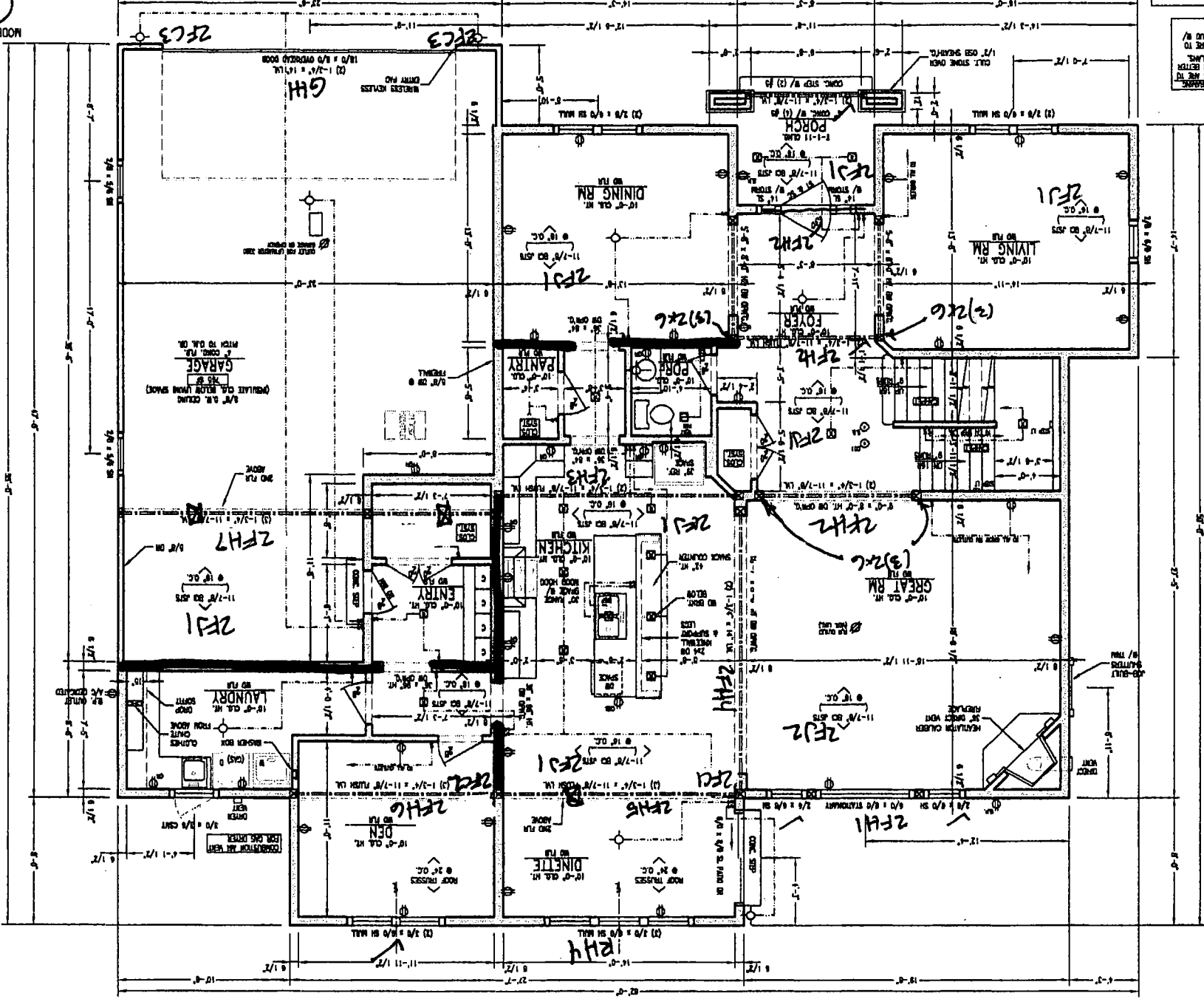
Disclosure

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FIRST FLOOR PLAN

SCALE: 1/4" = 1'-0"



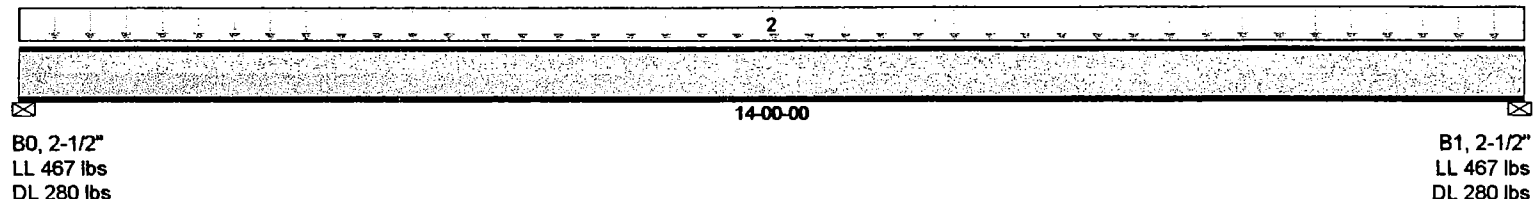
NOTES

- 1 - 1/2\"/>

ELECTRICAL LEGEND	
	110-VOLT OUTLET
	CABINET OUTLET
	4000-CENT. OUTLET FOR RANGE UTS.
	220-VOLT OUTLET
	GROUND FAULT INTERRUPTER OUTLET
	WATER PROOF OUTLET
	DIRECT CONNECT OUTLET
	SWITCH
	SMOKE DETECTOR
	DOWN'S FAN
	RECESSED LIGHT
	RECESSED LIGHT
	RECESSED LIGHT
	RECESSED LIGHT - SLOPED GL.
	CEILING MOUNT LIGHT
	BALL MOUNT LIGHT
	LIGHT WITH PULL DOWN
	CEILING FAN / LIGHT
	FLUORESCENT LIGHT
	FLOOD LIGHT
	POWER JACK
	CABLE JACK
	HOSE BIBB
	SEWER WASTE STACK
	SHOWER IN TUB PROJECT

Job Name:
Address:
City, State, Zip:
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1336

File Name: Pierri
Description: 2FJ1
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



Total Horizontal Product Length = 14-00-00

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 100%	Dead 90%	Snow 115%	Wind 133%	Roof Live 125%	OCS
2	BATHROOM - CT FLR	Unf. Area (psf)	Left	00-00-00	14-00-00	50	30				16"

Controls Summary

	Value	% Allowable	Duration	Case	Span
Pos. Moment	2,506 ft-lbs	82.8%	100%	1	1 - Internal
End Reaction	724 lbs	61.0%	100%	1	1 - Left
Total Load Defl.	L/552 (0.298")	65.2%		1	1
Live Load Defl.	L/883 (0.186")	67.9%		1	1
Max Defl.	0.298"	59.6%		1	1
Span / Depth	13.9	n/a			1

Bearing Supports

	Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0	Wall/Plate 2-1/2" x 1-3/4"	747 lbs	40.2%	n/a	Spruce-Pine-Fir
B1	Wall/Plate 2-1/2" x 1-3/4"	747 lbs	n/a	n/a	Unspecified

Notes

Design meets User specified (L/360) Total load deflection criteria.
Design meets User specified (L/600) Live load deflection criteria.
Design meets arbitrary (0.5") Maximum load deflection criteria.
Composite EI value based on 23/32" thick sheathing glued and nailed to joist.

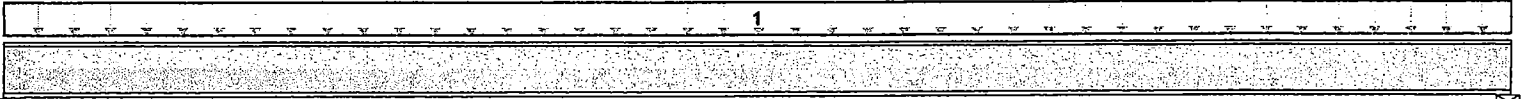
Disclosure

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Job Name:
Address:
City, State, Zip: ,
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1336

File Name: Pierr
Description: 2FJ2
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



B0, 2-1/2"
LL 614 lbs
DL 368 lbs

B1, 3-1/2"
LL 619 lbs
DL 372 lbs

Total Horizontal Product Length = 18-06-00

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 100%	Dead 90%	Snow 115%	Wind 133%	Roof Live 125%	OCS 16"
1	BATHROOM - CT FLR	Unf. Area (psf)	Left	00-00-00	18-06-00	50	30				

Controls Summary

Value	% Allowable	Duration	Case	Span	
Pos. Moment	4,390 ft-lbs	70.4%	100%	1	1 - Internal
End Reaction	960 lbs	71.5%	100%	1	1 - Left
Total Load Defl.	L/395 (0.551")	91.0%		1	1
Live Load Defl.	L/633 (0.344")	94.8%		1	1
Max Defl.	0.551"	55.1%		1	1
Span / Depth	18.3	n/a			1

Disclosure

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Bearing Supports

Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0 Wall/Plate 2-1/2" x 2-5/16"	982 lbs	40.0%	n/a	Spruce-Pine-Fir
B1 Wall/Plate 3-1/2" x 2-5/16"	991 lbs	28.8%	n/a	Spruce-Pine-Fir

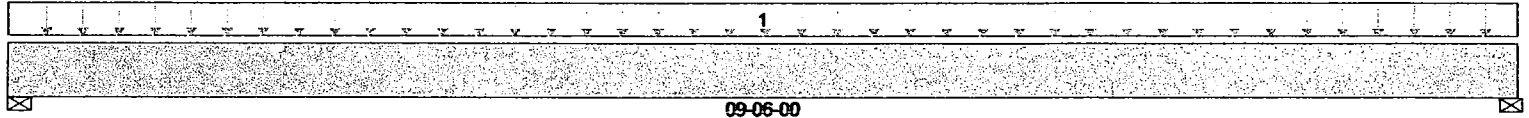
Notes

Design meets User specified (L/360) Total load deflection criteria.
Design meets User specified (L/600) Live load deflection criteria.
Design meets arbitrary (1") Maximum load deflection criteria.
Composite EI value based on 23/32" thick sheathing glued and nailed to joist.

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Job Name:
Address:
City, State, Zip: ,
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1040

File Name: Pierri
Description: RH4
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



B0, 3-1/2"
DL 676 lbs
SL 1,697 lbs

B1, 4-3/8"
DL 640 lbs
SL 1,723 lbs

Total Horizontal Product Length = 09-06-00

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 100%	Dead 90%	Snow 115%	Wind 133%	Roof Live 125%	Trib.
1	DRIFT SNOW	Unf. Area (psf)	Left	00-00-00	09-06-00		31	90			04-00-00

Controls Summary

	Value	% Allowable	Duration	Case	Span
Pos. Moment	5,020 ft-lbs	31.3%	115%	3	1 - Internal
End Shear	1,824 lbs	25.1%	115%	3	1 - Left
Total Load Defl.	L/741 (0.145")	24.3%		3	1
Live Load Defl.	L/1,027 (0.105")	35.1%		3	1
Max Defl.	0.145"	14.5%		3	1
Span / Depth	11.3	n/a			1

Disclosure

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Bearing Supports

	Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0	Wall/Plate 3-1/2" x 3-1/2"	2,373 lbs	45.6%	25.8%	Spruce-Pine-Fir
B1	Wall/Plate 4-3/8" x 3-1/2"	2,363 lbs	36.3%	20.6%	Spruce-Pine-Fir

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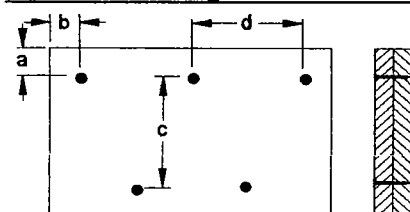
Cautions

For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur.
For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load.

Notes

Design meets Code minimum (L/180) Total load deflection criteria.
Design meets User specified (L/360) Live load deflection criteria.
Design meets arbitrary (1") Maximum load deflection criteria.

Connection Diagram

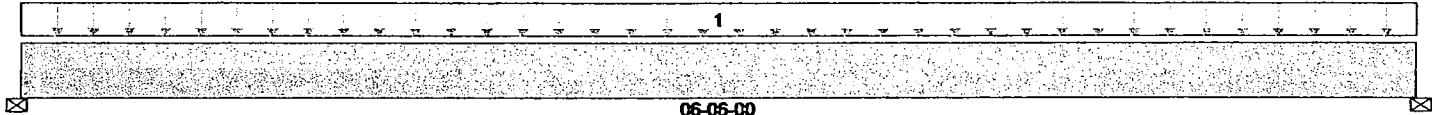


a minimum = 2" c = 5-1/2"
b minimum = 3" d = 12"

Member has no side loads.
Connectors are: 16d Common Nails

Job Name:
Address:
City, State, Zip: ,
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1040

File Name: Pierri
Description: 2FH1
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



B0, 1-3/4"
LL 1,625 lbs
DL 1,005 lbs

B1, 1-3/4"
LL 1,625 lbs
DL 1,005 lbs

Total Horizontal Product Length = 06-06-00

Tag	Description	Load Type	Ref.	Start	End	Live 100%	Dead 90%	Snow 115%	Wind 133%	Roof Live 125%	Trib.
1	2FJ2	Unf. Area (psf)	Left	00-00-00	06-06-00	50	30				10-00-00

Controls Summary	Value	% Allowable	Duration	Case	Span
Pos. Moment	4,058 ft-lbs	29.1%	100%	1	1 - Internal
End Shear	1,872 lbs	29.6%	100%	1	1 - Left
Total Load Defl.	L/1,297 (0.059")	18.5%		1	1
Live Load Defl.	L/2,100 (0.036")	22.9%		1	1
Max Defl.	0.059"	5.9%		1	1
Span / Depth	8.0	n/a			1

Disclosure
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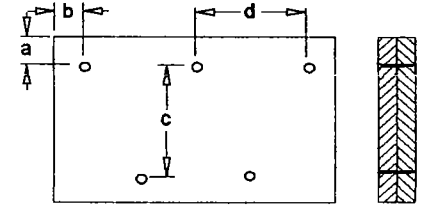
Bearing Supports	Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0	Wall/Plate 1-3/4" x 3-1/2"	2,630 lbs	101.0%	57.3%	Spruce-Pine-Fir
B1	Wall/Plate 1-3/4" x 3-1/2"	2,630 lbs	101.0%	57.3%	Spruce-Pine-Fir

SAY OK

Cautions
Bearing length at bearing B0 should be at least 1-13/16".
Bearing B0 cannot support a load of 2,630 lbs.
Bearing length at bearing B1 should be at least 1-13/16".
Bearing B1 cannot support a load of 2,630 lbs.

Notes
Design meets Code minimum (L/240) Total load deflection criteria.
Design meets User specified (L/480) Live load deflection criteria.
Design meets arbitrary (1") Maximum load deflection criteria.

Connection Diagram

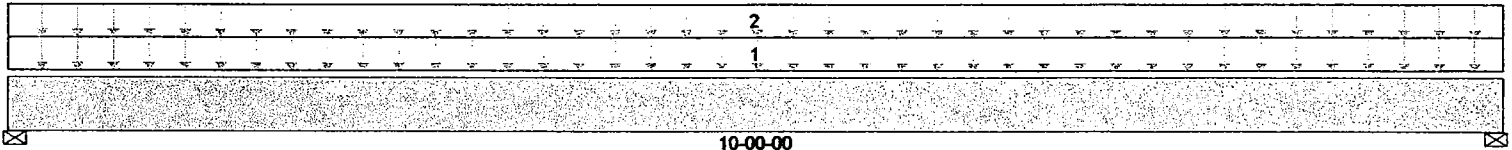


a minimum = 2" c = 5-1/2"
b minimum = 3" d = 12"

Member has no side loads.
Connectors are: 16d Common Nails

Job Name:
Address:
City, State, Zip: ,
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1040

File Name: Pierri
Description: 2FH2
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



B0, 4-3/8"
LL 3,500 lbs
DL 2,086 lbs

B1, 4-3/8"
LL 3,500 lbs
DL 2,064 lbs

Total Horizontal Product Length = 10-00-00

Load Summary		Live	Dead	Snow	Wind	Roof Live
Tag	Description	100%	90%	115%	133%	125% Trib.
1	2FJ1	40	20			05-00-00
2	2FJ2	50	30			10-00-00

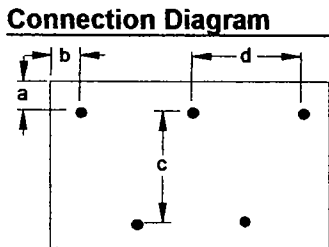
Controls Summary	Value	% Allowable	Duration	Case	Span
Pos. Moment	12,292 ft-lbs	57.8%	100%	1	1 - Internal
End Shear	4,068 lbs	51.5%	100%	1	1 - Left
Total Load Defl.	L/564 (0.2")	63.9%		1	1
Live Load Defl.	L/897 (0.126")	53.5%		1	1
Max Defl.	0.2"	20.0%		1	1
Span / Depth	9.5	n/a			1

Disclosure
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Bearing Supports	Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0	Wall/Plate 4-3/8" x 3-1/2"	5,586 lbs	85.8%	48.6%	Spruce-Pine-Fir
B1	Wall/Plate 4-3/8" x 3-1/2"	5,564 lbs	85.5%	48.4%	Spruce-Pine-Fir

Notes
Design meets User specified (L/360) Total load deflection criteria.
Design meets User specified (L/480) Live load deflection criteria.
Design meets arbitrary (1") Maximum load deflection criteria.

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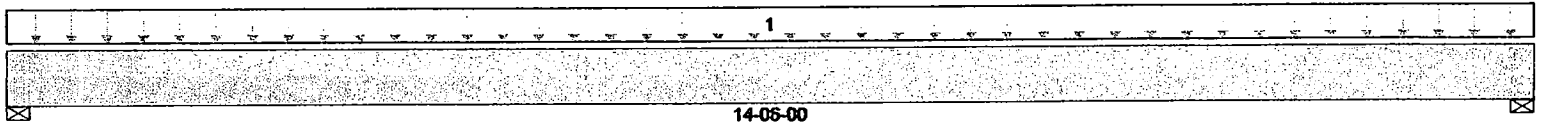


a minimum = 2" c = 7-7/8"
b minimum = 3" d = 12"

Member has no side loads.
Connectors are: 16d Common Nails

Job Name:
Address:
City, State, Zip: ,
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1040

File Name: Pierri
Description: 2FH3
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



B0, 3-1/2" LL 1,450 lbs DL 941 lbs
B1, 3-1/2" LL 1,450 lbs DL 862 lbs

Total Horizontal Product Length = 14-06-00

Load Summary					Live	Dead	Snow	Wind	Roof Live	Trib.	
Tag	Description	Load Type	Ref.	Start	End	100%	90%	115%	133%	125%	
1	2FJ1	Unf. Area (psf)	Left	00-00-00	14-06-00	40	20				05-00-00

Controls Summary	Value	% Allowable	Duration	Case	Span
Pos. Moment	8,007 ft-lbs	37.6%	100%	1	1 - Internal
End Shear	1,959 lbs	24.8%	100%	1	1 - Left
Total Load Defl.	L/580 (0.291")	62.1%		1	1
Live Load Defl.	L/941 (0.179")	51.0%		1	1
Max Defl.	0.291"	29.1%		1	1
Span / Depth	14.2	n/a			1

Disclosure
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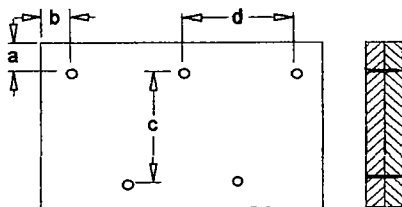
Bearing Supports	Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0	Wall/Plate 3-1/2" x 3-1/2"	2,391 lbs	45.9%	26.0%	Spruce-Pine-Fir
B1	Wall/Plate 3-1/2" x 3-1/2"	2,312 lbs	44.4%	25.2%	Spruce-Pine-Fir

Notes

Design meets User specified (L/360) Total load deflection criteria.
Design meets User specified (L/480) Live load deflection criteria.
Design meets arbitrary (1") Maximum load deflection criteria.

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Connection Diagram

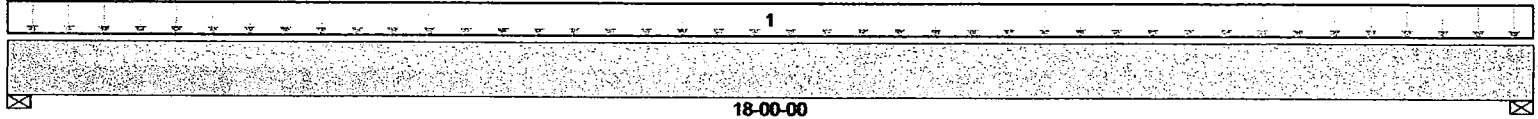


a minimum = 2" c = 7-7/8"
b minimum = 3" d = 12"

Calculated Side Load = 150.0 plf
Connectors are: 16d Common Nails

Job Name:
Address:
City, State, Zip: ,
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1040

File Name: Pierri
Description: 2FH4
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



B0, 3-1/2"
LL 2,880 lbs
DL 1,626 lbs

B1, 3-1/2"
LL 2,880 lbs
DL 1,626 lbs

Total Horizontal Product Length = 18-00-00

Load Summary		Live	Dead	Snow	Wind	Roof Live
Tag	Description	100%	90%	115%	133%	125%
1	2FH1	40	20			08-00-00

Controls Summary	Value	% Allowable	Duration	Case	Span
Pos. Moment	19,259 ft-lbs	44.2%	100%	1	1 - Internal
End Shear	3,776 lbs	27.0%	100%	1	1 - Left
Total Load Defl.	L/474 (0.444")	50.7%		1	1
Live Load Defl.	L/741 (0.284")	64.7%		1	1
Max Defl.	0.444"	44.4%		1	1
Span / Depth	15.0	n/a			1

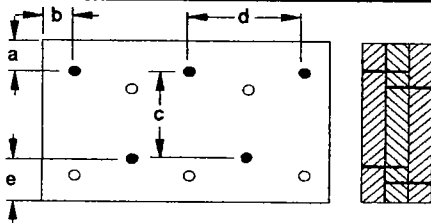
Disclosure
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Bearing Supports	Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0	Wall/Plate 3-1/2" x 5-1/4"	4,506 lbs	57.7%	32.7%	Spruce-Pine-Fir
B1	Wall/Plate 3-1/2" x 5-1/4"	4,506 lbs	57.7%	32.7%	Spruce-Pine-Fir

Notes
Design meets Code minimum (L/240) Total load deflection criteria.
Design meets User specified (L/480) Live load deflection criteria.
Design meets arbitrary (1") Maximum load deflection criteria.

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Connection Diagram

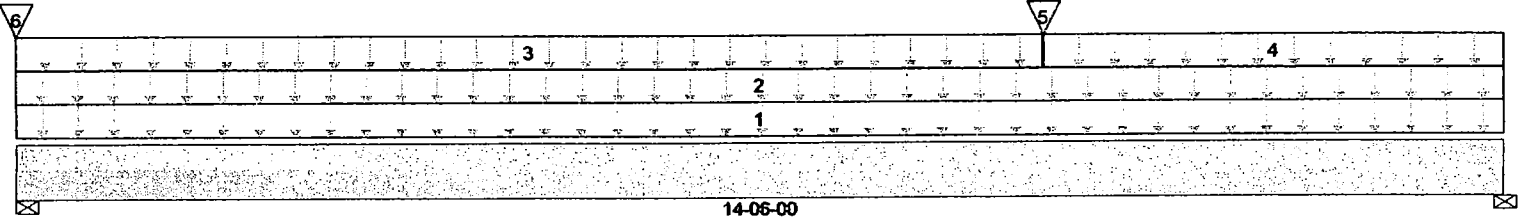


a minimum = 2" c = 9"
b minimum = 3" d = 12"
e minimum = 3"

Member has no side loads.
Connectors are: 16d Common Nails

Job Name:
Address:
City, State, Zip: ,
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1040

File Name: Pierr
Description: 2FH5
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



B0, 5-1/2"
LL 3,450 lbs
DL 8,594 lbs
SL 8,551 lbs

B1, 2-1/2"
LL 570 lbs
DL 6,336 lbs
SL 7,807 lbs

Total Horizontal Product Length = 14-06-00

Load Summary					Live	Dead	Snow	Wind	Roof Live		
Tag	Description	Load Type	Ref.	Start	End	100%	90%	115%	133%	125%	OCS
1	LOW ROOF	Unf. Area (psf)	Left	00-00-00	14-06-00		31	90			05-00-00
2	2FJ1	Unf. Area (psf)	Left	00-00-00	14-06-00	40	20				02-00-00
3	FULL ROOF	Unf. Area (psf)	Left	00-00-00	10-00-00		31	30			22-00-00
4	PARTIAL ROOF	Unf. Area (psf)	Left	10-00-00	14-06-00		31	30			09-00-00
5	GT1	Conc. Pt. (lbs)	Left	10-00-00	10-00-00		2,120	2,018			n/a
6	2FH4	Conc. Pt. (lbs)	Left	00-00-00	00-00-00	2,860	1,564				n/a

Controls Summary	Value	% Allowable	Duration	Case	Span
Pos. Moment	56,302 ft-lbs	87.4%	115%	2	1 - Internal
End Shear	-12,713 lbs	69.3%	115%	2	1 - Right
Total Load Defl.	L/305 (0.549")	78.7%		2	1
Live Load Defl.	L/543 (0.308")	88.3%		2	1
Max Defl.	0.549"	54.9%		2	1
Span / Depth	10.5	n/a			1

Disclosure
Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

Bearing Supports	Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0	Wall/Plate 5-1/2" x 5-1/4"	20,595 lbs	0.7%	95.1%	Steel
B1	Wall/Plate 2-1/2" x 5-1/4"	14,713 lbs	1.1%	149.5%	Steel

see ZFLZ

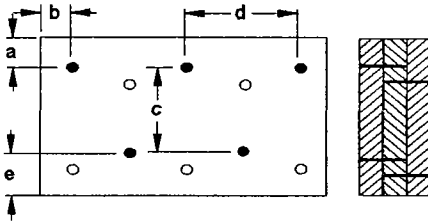
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Cautions
Bearing length at bearing B1 should be at least 3-3/4".

Notes
Design meets Code minimum (L/240) Total load deflection criteria.
Design meets User specified (L/480) Live load deflection criteria.
Design meets arbitrary (1") Maximum load deflection criteria.

Job Name:
Address:
City, State, Zip: ,
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1040

File Name: Pierr
Description: 2FH5
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:

Connection Diagram

a minimum = 2" c = 11"
b minimum = 3" d = 12"
e minimum = 3"

Calculated Side Load = 60.0 plf

Connection design assumes point load is 'top-loaded'. For connection design of 'side-loaded' point loads, please consult a technical representative or professional of Record.

Nailing schedule applies to both sides of the member.

Concentrated loads are not considered in side load analysis.

Connectors are: 16d Common Nails

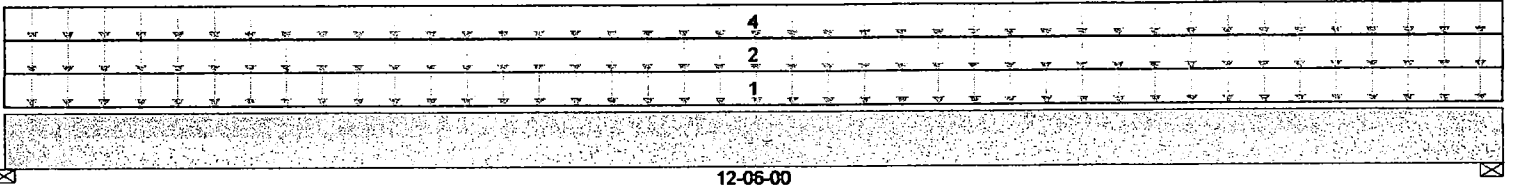
Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

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Job Name:
Address:
City, State, Zip: ,
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1040

File Name: Pierri
Description: 2FH6
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



⊗ B0, 2-1/2"
LL 980 lbs
DL 3,403 lbs
SL 4,410 lbs

⊗ B1, 5-1/2"
LL 1,020 lbs
DL 3,542 lbs
SL 4,590 lbs

Total Horizontal Product Length = 12-06-00

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 100%	Dead 90%	Snow 115%	Wind 133%	Roof Live 125%	OCS
1	LOW ROOF	Unf. Area (psf)	Left	00-00-00	12-06-00		31	90			05-00-00
2	2FJ1	Unf. Area (psf)	Left	00-00-00	12-06-00	40	20				04-00-00
4	ROOF	Unf. Area (psf)	Left	00-00-00	12-06-00		31	30			09-00-00

Controls Summary

Value	% Allowable	Duration	Case	Span
Pos. Moment 25,661 ft-lbs	69.9%	115%	13	1 - Internal
End Shear 7,073 lbs	51.9%	115%	2	1 - Left
Total Load Defl. L/318 (0.451")	75.4%		13	1
Live Load Defl. L/519 (0.276")	92.4%		13	1
Max Defl. 0.451"	45.1%		13	1
Span / Depth 12.1	n/a			1

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

Bearing Supports

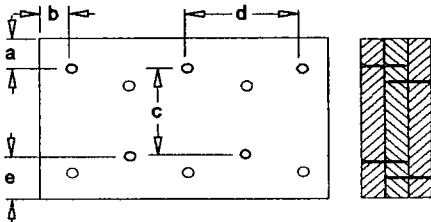
Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0 Wall/Plate 2-1/2" x 5-1/4"	8,793 lbs	0.7%	89.3%	Steel
B1 Wall/Plate 5-1/2" x 5-1/4"	9,152 lbs	74.6%	42.3%	Spruce-Pine-Fir

Notes

Design meets Code minimum (L/240) Total load deflection criteria.
Design meets User specified (L/480) Live load deflection criteria.
Design meets arbitrary (1") Maximum load deflection criteria.

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Connection Diagram



a minimum = 2" c = 6-7/8"
b minimum = 3" d = 12"
e minimum = 3"

Calculated Side Load = 120.0 plf

Nailing schedule applies to both sides of the member.
Connectors are: 16d Common Nails



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COMPANY
STL Consulting
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PROJECT

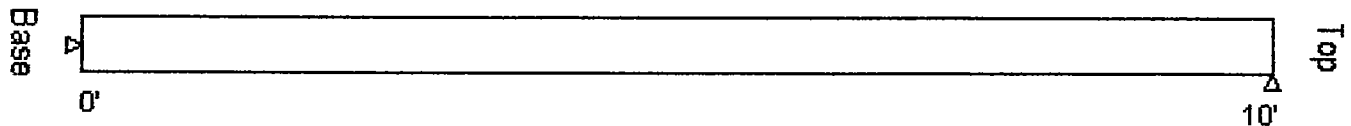
Column1 - 2FC1

Design Check Calculation Sheet
Sizer 8.1

LOADS:

Load	Type	Distribution	Pat-tern	Location [ft]		Magnitude		Unit
				Start	End	Start	End	
Load1	Dead	Axial		(Ecc. = 0.00")		8594		lbs
Load2	Live	Axial		(Ecc. = 0.00")		3450		lbs
Load3	Snow	Axial		(Ecc. = 0.00")		8551		lbs

MAXIMUM REACTIONS (lbs):



Lumber n-ply, S-P-F, Stud, 2x6", ~~3-Plys~~ *USE 5 FOR BEARING PRESSURE*

Self-weight of 5.0 plf included in loads;

Pinned base; Loadface = width(b); Built-up fastener: nails; $K_e \times L_b: 1.00 \times 0.00 = 0.00$ [ft]; $K_e \times L_t: 1.00 \times 0.00 = 0.00$ [ft];
Repetitive factor: applied where permitted (refer to online help);

Analysis vs. Allowable Stress (psi) and Deflection (in) using NDS 2005:

Criterion	Analysis Value	Design Value	Analysis/Design
Axial	$f_c = 713$	$F_c' = 834$	$f_c/F_c' = 0.86$
Axial Bearing	$f_c = 713$	$F_c^* = 834$	$f_c/F_c^* = 0.86^*$

*Column requires a bearing plate at top as per NDS 3.10.1.3

ADDITIONAL DATA:

FACTORS:	F/R	CD	CM	Ct	CL/CP	CF	Cfu	Cr	Cfrc	Ci	LC#
F_c'	725	1.15	1.00	1.00	1.000	1.000	-	-	1.00	1.00	3
F_c^*	725	1.15	1.00	1.00	-	1.000	-	-	1.00	1.00	3

Axial : LC #3 = $D + .75(L+S)$, $P = 17645$ lbs $K_f = 1.00$

D=dead L=live S=snow W=wind I=impact Lr=roof live Lc=concentrated

All LC's are listed in the Analysis output

Load combinations: ICC-IBC

DESIGN NOTES:

1. Please verify that the default deflection limits are appropriate for your application.
2. BUILT-UP COLUMNS: nailed or bolted built-up columns shall conform to the provisions of NDS Clause 15.3.



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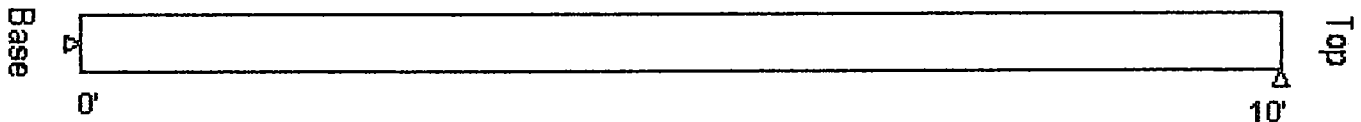
Column1 - 2FC2

Design Check Calculation Sheet
 Sizer 8.1

LOADS:

Load	Type	Distribution	Pat-tern	Location [ft]		Magnitude		Unit
				Start	End	Start	End	
Load1	Dead	Axial		(Ecc. = 0.00")		9878		lbs
Load2	Live	Axial		(Ecc. = 0.00")		1590		lbs
Load3	Snow	Axial		(Ecc. = 0.00")		12397		lbs

MAXIMUM REACTIONS (lbs):



Lumber n-ply, S-P-F, Stud, 2x6", 5-Plys

Self-weight of 8.34 plf included in loads;

Pinned base; Loadface = width(b); Built-up fastener: nails; $K_e \times L_b: 1.00 \times 0.00 = 0.00$ [ft]; $K_e \times L_t: 1.00 \times 10.00 = 10.00$ [ft]; Repetitive factor: applied where permitted (refer to online help);

WARNING: 5-ply built-up columns are not recommended. Refer to Design Note below.

Analysis vs. Allowable Stress (psi) and Deflection (in) using NDS 2005:

Criterion	Analysis Value	Design Value	Analysis/Design
Axial	$f_c = 542$	$F_c' = 549$	$f_c/F_c' = 0.99$
Axial Bearing	$f_c = 542$	$F_c^* = 834$	$f_c/F_c^* = 0.65$

ADDITIONAL DATA:

FACTORS:	F/E	CD	CM	Ct	CL/CP	CF	Cfu	Cr	Cft	Ci	LC#
F_c'	725	1.15	1.00	1.00	0.658	1.000	-	-	1.00	1.00	4
F_c^*	725	1.15	1.00	1.00	-	1.000	-	-	1.00	1.00	4

Axial : LC #4 = D+S, P = 22358 lbs Kf = 1.00

D=dead L=live S=snow W=wind I=impact Lr=roof live Lc=concentrated

All LC's are listed in the Analysis output

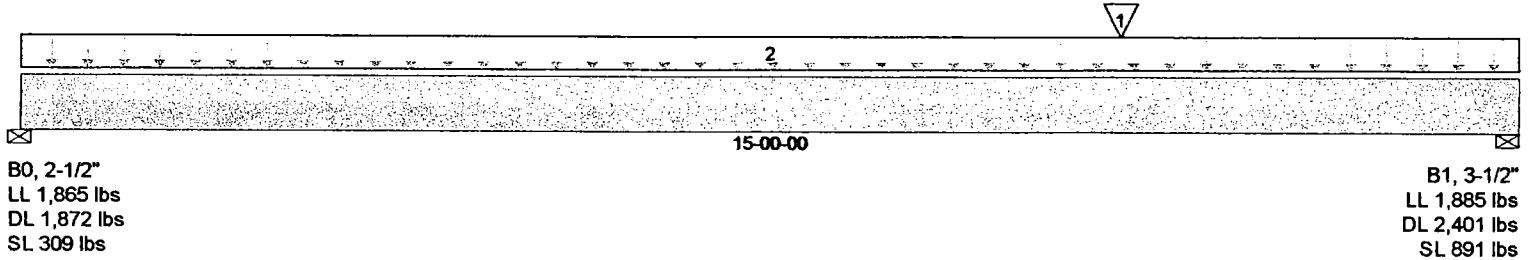
Load combinations: ICC-IBC

DESIGN NOTES:

1. Please verify that the default deflection limits are appropriate for your application.
2. BUILT-UP COLUMNS: nailed or bolted built-up columns shall conform to the provisions of NDS Clause 15.3.

Job Name:
Address:
City, State, Zip: ,
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1040

File Name: Pierri
Description: 2FH7
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



Total Horizontal Product Length = 15-00-00

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 100%	Dead 90%	Snow 115%	Wind 133%	Roof Live 125%	OCS
1	ROOF	Conc. Pt. (lbs)	Left	11-00-00	11-00-00		1,240	1,200			n/a
2	2FJ1	Unf. Area (psf)	Left	00-00-00	15-00-00	50	30				05-00-00

Controls Summary

Value	% Allowable	Duration	Case	Span
Pos. Moment 17,243 ft-lbs	47.0%	115%	2	1 - Internal
End Shear -4,642 lbs	34.1%	115%	2	1 - Right
Total Load Defl. L/387 (0.454")	93.1%		2	1
Live Load Defl. L/728 (0.241")	65.9%		2	1
Max Defl. 0.454"	45.4%		2	1
Span / Depth 14.8	n/a			1

Disclosure

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Bearing Supports

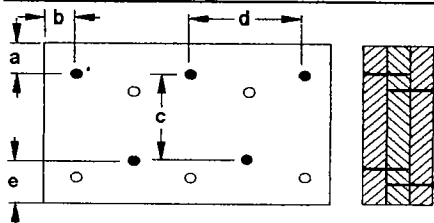
Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0 Wall/Plate 2-1/2" x 5-1/4"	4,046 lbs	72.5%	41.1%	Spruce-Pine-Fir
B1 Wall/Plate 3-1/2" x 5-1/4"	5,177 lbs	66.3%	37.6%	Spruce-Pine-Fir

Notes

Design meets User specified (L/360) Total load deflection criteria.
Design meets User specified (L/480) Live load deflection criteria.
Design meets arbitrary (1") Maximum load deflection criteria.

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Connection Diagram



a minimum = 2" c = 6-7/8"
b minimum = 3" d = 12"
e minimum = 3"

Calculated Side Load = 200.0 plf

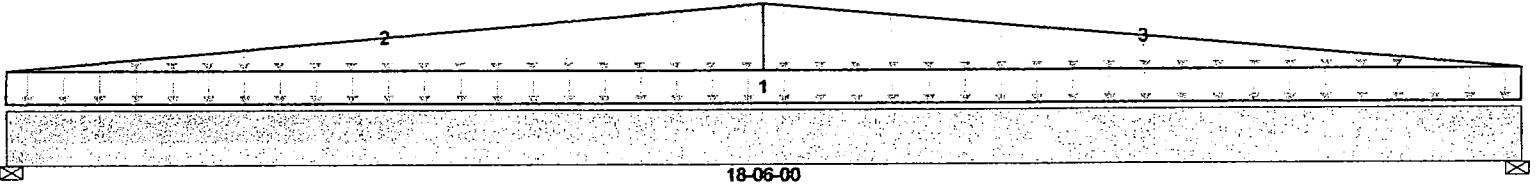
Connection design assumes point load is 'top-loaded'. For connection design of 'side-loaded' point loads, please consult a technical representative or professional of Record.

Concentrated loads are not considered in side load analysis.

Connectors are: 16d Common Nails

Job Name:
Address:
City, State, Zip: ,
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1040

File Name: Pierr
Description: GH1
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



B0, 2-1/2"
DL 1,876 lbs
SL 1,110 lbs

B1, 2-1/2"
DL 1,876 lbs
SL 1,110 lbs

Total Horizontal Product Length = 18-06-00

Load Summary					Live	Dead	Snow	Wind	Roof Live		
Tag	Description	Load Type	Ref.	Start	End	100%	90%	115%	133%	125%	Trib.
1	Standard Load	Unf. Area (psf)	Left	00-00-00	18-06-00		31	30			04-00-00
2	GABLE END	Trapezoidal (plf)	Left	00-00-00			0				n/a
					09-03-00		130				n/a
3	GABLE END	Trapezoidal (plf)	Left	09-03-00			130				n/a
					18-06-00		0				n/a

Controls Summary	Value	% Allowable	Duration	Case	Span
Pos. Moment	14,304 ft-lbs	42.8%	115%	3	1 - Internal
End Shear	2,618 lbs	24.5%	115%	3	1 - Left
Total Load Defl.	L/414 (0.528")	87.0%		3	1
Live Load Defl.	L/1,178 (0.185")	30.5%		3	1
Max Defl.	0.528"	52.8%		3	1
Span / Depth	15.6	n/a			1

Disclosure
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Bearing Supports	Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0	Wall/Plate 2-1/2" x 3-1/2"	2,986 lbs	80.3%	45.5%	Spruce-Pine-Fir
B1	Wall/Plate 2-1/2" x 3-1/2"	2,986 lbs	80.3%	45.5%	Spruce-Pine-Fir

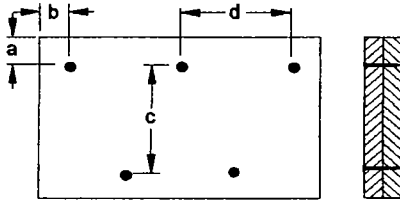
Cautions
For roof members with slope (1/4)/12 or less final design must ensure that ponding instability will not occur.
For roof members with slope (1/2)/12 or less final design must account for Rain-on-Snow surcharge load.

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Notes
Design meets User specified (L/360) Total load deflection criteria.
Design meets User specified (L/360) Live load deflection criteria.
Design meets arbitrary (1") Maximum load deflection criteria.

Job Name:
 Address:
 City, State, Zip: ,
 Customer: Kings Way Homes, L.L.C.
 Code reports: ESR-1040

File Name: Pierri
 Description: GH1
 Specifier: stl
 Designer: stl
 Company: STL Consulting
 Misc:

Connection Diagram

a minimum = 2" c = 10"
 b minimum = 3" d = 12"

Member has no side loads.
 Connectors are: 16d Common Nails

Disclosure

Completeness and accuracy of input must be verified by anyone who would rely on output as evidence of suitability for particular application. Output here based on building code-accepted design properties and analysis methods. Installation of BOISE engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

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CONSTRUCTION SOFTWARE

COMPANY
 STL Consulting
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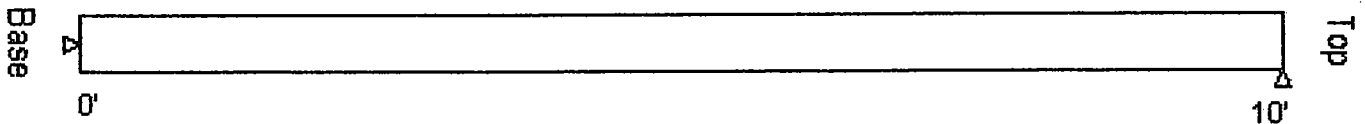
Column1 - 2FC3A

Design Check Calculation Sheet
 Size: 8.1

LOADS:

Load	Type	Distribution	Pat-tern	Location [ft]		Magnitude		Unit
				Start	End	Start	End	
Load1	Dead	Axial		(Ecc. = 0.00")		1876		lbs
Load2	Snow	Axial		(Ecc. = 0.00")		1110		lbs

MAXIMUM REACTIONS (lbs):



Lumber n-ply. S-P-F, Stud, 2x6", 2-Plys

Self-weight of 3.33 plf included in loads;

Pinned base; Loadface = width(b); Built-up fastener: nails; $K_e \times L_b: 1.00 \times 0.00 = 0.00$ [ft]; $K_e \times L_d: 1.00 \times 10.00 = 10.00$ [ft];

Analysis vs. Allowable Stress (psi) and Deflection (in) using NDS 2005:

Criterion	Analysis Value	Design Value	Analysis/Design
Axial	$f_c = 183$	$F_c' = 549$	$f_c/F_c' = 0.33$
Axial Bearing	$f_c = 183$	$F_c^* = 834$	$f_c/F_c^* = 0.22$

ADDITIONAL DATA:

FACTORS:	F/E	CD	CM	Ct	CL/CP	CF	Cfu	Cr	Cfrc	Ci	LC#
F_c'	725	1.15	1.00	1.00	0.658	1.000	-	-	1.00	1.00	2
F_c^*	725	1.15	1.00	1.00	-	1.000	-	-	1.00	1.00	2

Axial : LC #2 = D+S, P = 3019 lbs Kf = 1.00

D=dead L=live S=snow W=wind I=impact Lr=roof live Lc=concentrated

All LC's are listed in the Analysis output

Load combinations: ICC-IBC

DESIGN NOTES:

- Please verify that the default deflection limits are appropriate for your application.
- BUILT-UP COLUMNS: nailed or bolted built-up columns shall conform to the provisions of NDS Clause 15.3.



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PROJECT

 Column1 - 2FC3B

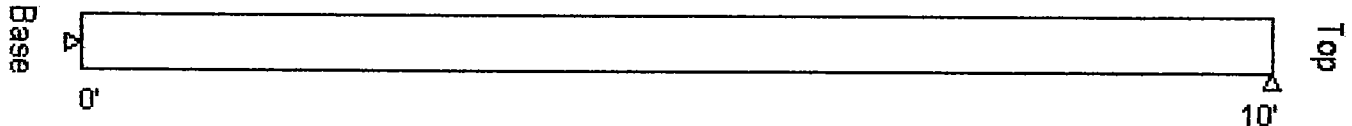
Design Check Calculation Sheet
 Size 8.1

LOADS:

Load	Type	Distribution	Pat-tern	Location [ft]		Magnitude		Unit
				Start	End	Start	End	
Load1	Wind	Full Area				20.00	(9.00)*	psf

*Tributary Width (ft)

MAXIMUM REACTIONS (lbs):



Unfactored:			
Dead			
Other	900		900
Factored:			
Total	900		900

Lumber n-ply, S-P-F, Stud, 2x6", 3-Plys

Self-weight of 5.0 plf included in loads;

Pinned base; Loadface = width(b); Built-up fastener: nails; $K_e \times L_b: 1.00 \times 0.00 = 0.00$ [ft]; $K_e \times L_d: 1.00 \times 10.00 = 10.00$ [ft]; Lateral support: top = Lb, bottom = Lb; Repetitive factor: applied where permitted (refer to online help);

Analysis vs. Allowable Stress (psi) and Deflection (in) using NDS 2005:

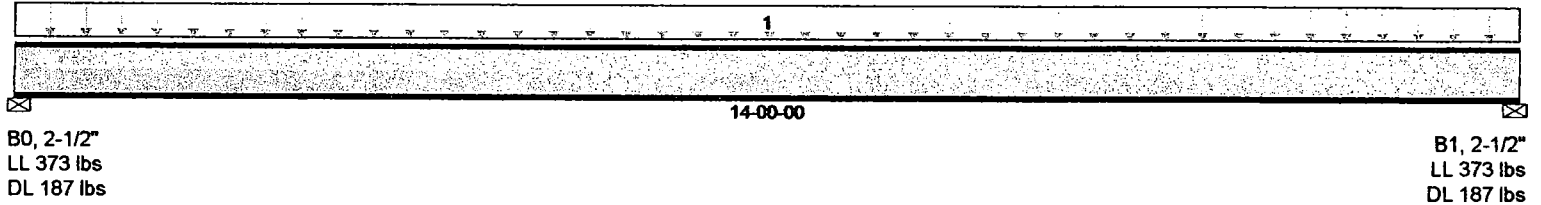
Criterion	Analysis Value	Design Value	Analysis/Design
Shear	$f_v = 55$	$F_v' = 216$	$f_v/F_v' = 0.25$
Bending(+)	$f_b = 1190$	$F_b' = 1242$	$f_b/F_b' = 0.96$
Axial	$f_c = 2$	$F_c' = 483$	$f_c/F_c' = 0.00$
Axial Bearing	$f_c = 2$	$F_c^* = 652$	$f_c/F_c^* = 0.00$
Combined (axial compression + side load bending)			Eq. 3.9-3 = 0.96
Live Defl'n	$0.54 = L/221$	$0.67 = L/180$	0.81
Total Defl'n	$0.54 = L/221$	$0.67 = L/180$	0.81

ADDITIONAL DATA:

FACTORS:	F/E	CD	CM	Ct	CL/CP	CF	Cfu	Cr	Cfrc	Ci	LC#
F_v'	135	1.60	1.00	1.00	-	-	-	-	1.00	1.00	2
F_b'	675	1.60	1.00	1.00	1.000	1.000	1.00	1.15	1.00	1.00	2
F_c'	725	0.90	1.00	1.00	0.741	1.000	-	-	1.00	1.00	1
F_c' comb	725	1.60	-	-	0.533	-	-	-	-	-	2

Job Name:
Address:
City, State, Zip: ,
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1336

File Name: Pierri
Description: FJ1
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



Total Horizontal Product Length = 14-00-00

Load Summary		Live	Dead	Snow	Wind	Roof Live	
Tag	Description	100%	90%	115%	133%	125%	OCS
1	Standard Load	40	20				16"

Controls Summary	Value	% Allowable	Duration	Case	Span
Pos. Moment	1,879 ft-lbs	62.1%	100%	1	1 - Internal
End Reaction	543 lbs	45.8%	100%	1	1 - Right
Total Load Defl.	L/736 (0.224")	48.9%		1	1
Live Load Defl.	L/1,104 (0.149")	54.4%		1	1
Max Defl.	0.224"	22.4%		1	1
Span / Depth	13.9	n/a			1

Disclosure
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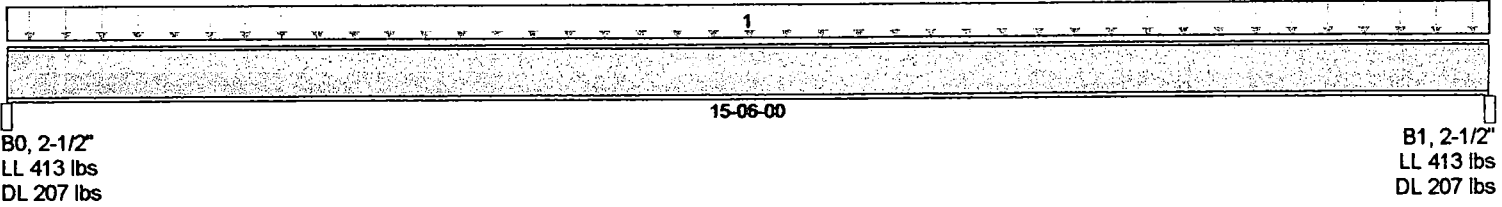
Bearing Supports		Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0	Wall/Plate	2-1/2" x 1-3/4"	560 lbs	n/a	n/a	Unspecified
B1	Wall/Plate	2-1/2" x 1-3/4"	560 lbs	n/a	n/a	Unspecified

Notes
Design meets User specified (L/360) Total load deflection criteria.
Design meets User specified (L/600) Live load deflection criteria.
Design meets arbitrary (1") Maximum load deflection criteria.
Composite EI value based on 23/32" thick sheathing glued and nailed to joist.

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCI®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Wood Products, L.L.C.

Job Name:
Address:
City, State, Zip: ,
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1336

File Name: Pierri
Description: FJ2
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



Total Horizontal Product Length = 15-06-00

Load Summary					Live	Dead	Snow	Wind	Roof Live		
Tag	Description	Load Type	Ref.	Start	End	100%	90%	115%	133%	125%	OCS
1	Standard Load	Unf. Area (psf)	Left	00-00-00	15-06-00	40	20				16"

Controls Summary	Value	% Allowable	Duration	Case	Span
Pos. Moment	2,313 ft-lbs	76.5%	100%	1	1 - Internal
End Reaction	603 lbs	50.8%	100%	1	1 - Left
Total Load Defl.	L/553 (0.33")	65.1%		1	1
Live Load Defl.	L/829 (0.22")	72.3%		1	1
Max Defl.	0.33"	33.0%		1	1
Span / Depth	15.4	n/a			1

Disclosure

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Bearing Supports		Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0	Beam	2-1/2" x 1-3/4"	620 lbs	0.1%	n/a	Steel
B1	Beam	2-1/2" x 1-3/4"	620 lbs	0.1%	n/a	Steel

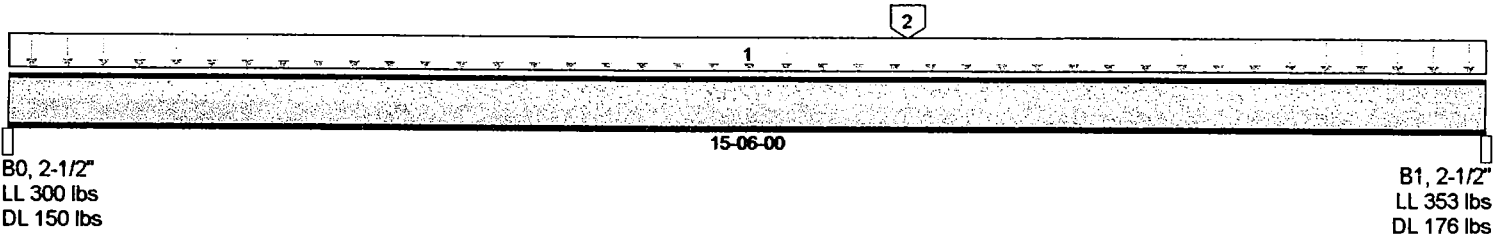
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Notes

Design meets User specified (L/360) Total load deflection criteria.
Design meets User specified (L/600) Live load deflection criteria.
Design meets arbitrary (1") Maximum load deflection criteria.
Composite EI value based on 23/32" thick sheathing glued and nailed to joist.

Job Name:
Address:
City, State, Zip:
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1336

File Name: Pierri
Description: FJ3
Specifier: stl
Designer: sti
Company: STL Consulting
Misc:



Total Horizontal Product Length = 15-06-00

Load Summary					Live	Dead	Snow	Wind	Roof Live		
Tag	Description	Load Type	Ref.	Start	End	100%	90%	115%	133%	125%	OCS
1	Standard Load	Unf. Area (psf)	Left	00-00-00	15-06-00	40	20				8"
2	LD BRG WALL	Conc. Lin. (plf)	Left	09-05-00	09-05-00	360	180				8"

Controls Summary	Value	% Allowable	Duration	Case	Span
Pos. Moment	2,403 ft-lbs	79.4%	100%	1	1 - internal
End Reaction	521 lbs	43.9%	100%	1	1 - Right
Total Load Defl.	L/471 (0.388")	76.5%		1	1
Live Load Defl.	L/706 (0.258")	85.0%		1	1
Max Defl.	0.388"	38.8%		1	1
Span / Depth	15.4	n/a			1

Disclosure
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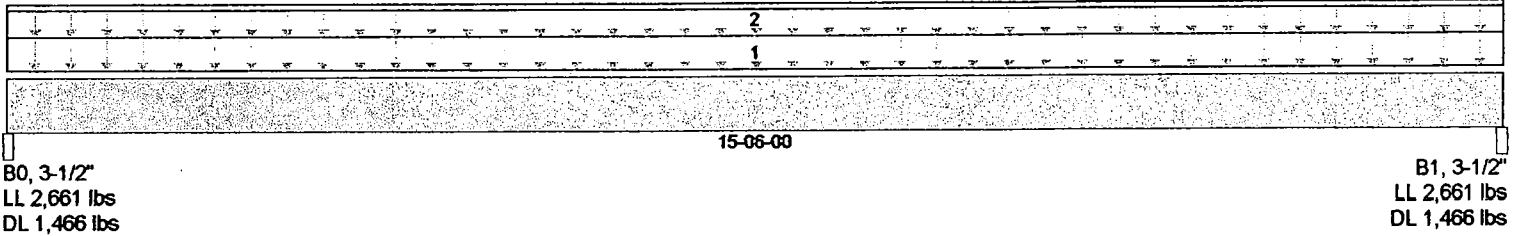
Bearing Supports	Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0 Beam	2-1/2" x 1-3/4"	451 lbs	0.1%	n/a	Steel
B1 Beam	2-1/2" x 1-3/4"	529 lbs	0.1%	n/a	Steel

Notes
Design meets User specified (L/360) Total load deflection criteria.
Design meets User specified (L/600) Live load deflection criteria.
Design meets arbitrary (1") Maximum load deflection criteria.
Composite EI value based on 23/32" thick sheathing glued and nailed to joist.

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Job Name:
Address:
City, State, Zip: ,
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1040

File Name: Pierri
Description: FH1
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



Total Horizontal Product Length = 15-06-00

Load Summary					Live	Dead	Snow	Wind	Roof Live		
Tag	Description	Load Type	Ref.	Start	End	100%	90%	115%	133%	125%	Trib.
1	STD	Unf. Area (psf)	Left	00-00-00	15-06-00	40	20				01-04-00
2	LD BRG WALL	Unf. Lin. (plf)	Left	00-00-00	15-06-00	290	145				n/a

Controls Summary	Value	% Allowable	Duration	Case	Span
Pos. Moment	15,061 ft-lbs	47.2%	100%	1	1 - Internal
End Shear	3,445 lbs	29.1%	100%	1	1 - Left
Total Load Defl.	L/431 (0.419")	83.5%		1	1
Live Load Defl.	L/669 (0.27")	89.7%		1	1
Max Defl.	0.419"	41.9%		1	1
Span / Depth	15.2	n/a			1

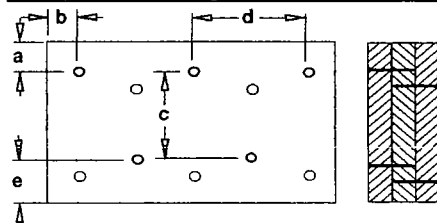
Disclosure
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Bearing Supports	Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0 Beam	3-1/2" x 5-1/4"	4,127 lbs	0.2%	29.9%	Steel
B1 Beam	3-1/2" x 5-1/4"	4,127 lbs	0.2%	29.9%	Steel

BC CALC®, BC FRAMER®, AJS™, ALLJOIST®, BC RIM BOARD™, BCi®, BOISE GLULAM™, SIMPLE FRAMING SYSTEM®, VERSA-LAM®, VERSA-RIM PLUS®, VERSA-RIM®, VERSA-STRAND®, VERSA-STUD® are trademarks of Boise Wood Products, L.L.C.

Notes
Design meets User specified (L/360) Total load deflection criteria.
Design meets User specified (L/600) Live load deflection criteria.
Design meets arbitrary (1") Maximum load deflection criteria.

Connection Diagram

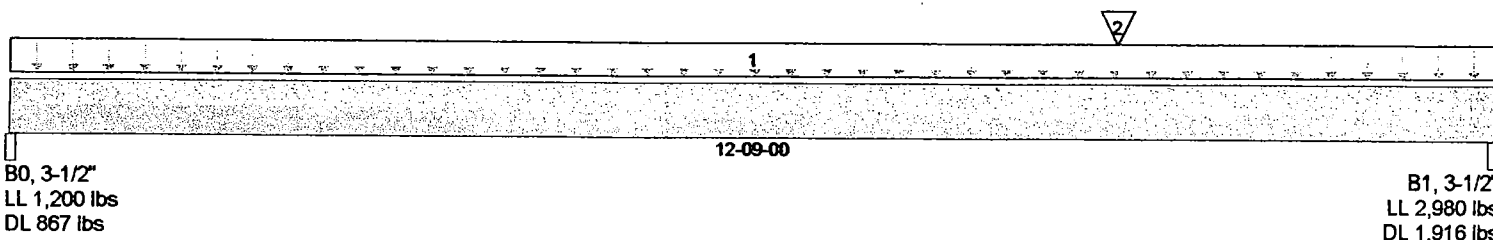


a minimum = 2" c = 6-7/8"
b minimum = 3" d = 12"
e minimum = 3"

Member has no side loads.
Connectors are: 16d Common Nails

Job Name:
Address:
City, State, Zip :
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1040

File Name: Pierri
Description: FH2
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



Total Horizontal Product Length = 12-09-00

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 100%	Dead 90%	Snow 115%	Wind 133%	Roof Live 125%	Trib.
1	STD	Unf. Area (psf)	Left	00-00-00	12-09-00	40	20				01-04-00
2	2FH2	Conc. Pt. (lbs)	Left	09-06-00	09-06-00	3,500	2,064				n/a

Controls Summary

Value	% Allowable	Duration	Case	Span
Pos. Moment 14,194 ft-lbs	66.7%	100%	1	1 - internal
End Shear -4,755 lbs	60.2%	100%	1	1 - Right
Total Load Defl. L/462 (0.319")	77.9%		1	1
Live Load Defl. L/765 (0.193")	78.4%		1	1
Max Defl. 0.319"	31.9%		1	1
Span / Depth 12.4	n/a			1

Disclosure

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Bearing Supports

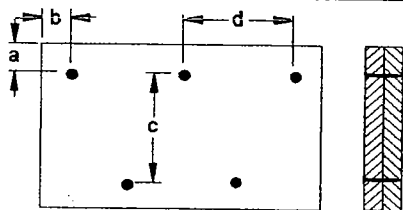
Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0 Beam 3-1/2" x 3-1/2"	2,067 lbs	0.2%	22.5%	Steel
B1 Beam 3-1/2" x 3-1/2"	4,896 lbs	0.4%	53.3%	Steel

Notes

Design meets User specified (L/360) Total load deflection criteria.
Design meets User specified (L/600) Live load deflection criteria.
Design meets arbitrary (1") Maximum load deflection criteria.

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Connection Diagram



a minimum = 2" c = 7-7/8"
b minimum = 3" d = 12"

Connection design assumes point load is 'top-loaded'. For connection design of 'side-loaded' point loads, please consult a technical representative or professional of Record.

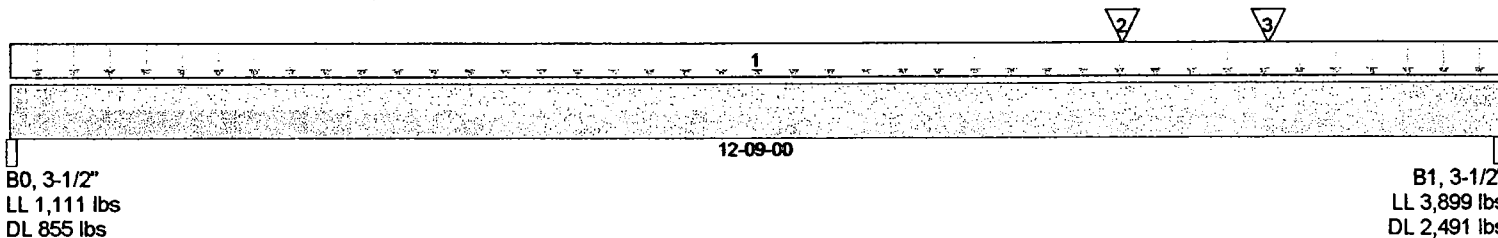
Member has no side loads.

Concentrated loads are not considered in side load analysis.

Connectors are: 16d Common Nails

Job Name:
Address:
City, State, Zip: ,
Customer: Kings Way Homes, L.L.C.
Code reports: ESR-1040

File Name: Pierri
Description: FH3
Specifier: stl
Designer: stl
Company: STL Consulting
Misc:



Total Horizontal Product Length = 12-09-00

Load Summary

Tag	Description	Load Type	Ref.	Start	End	Live 100%	Dead 90%	Snow 115%	Wind 133%	Roof Live 125%	Trib.
1	STD	Unf. Area (psf)	Left	00-00-00	12-09-00	40	20				01-04-00
2	2FH3	Conc. Pt. (lbs)	Left	09-06-00	09-06-00	1,450	941				n/a
3	2FH4	Conc. Pt. (lbs)	Left	10-09-00	10-09-00	2,880	1,626				n/a

Controls Summary

Value	% Allowable	Duration	Case	Span
Pos. Moment 13,051 ft-lbs	61.3%	100%	1	1 - Internal
End Shear -6,243 lbs	79.1%	100%	1	1 - Right
Total Load Defl. L/484 (0.304")	74.3%		1	1
Live Load Defl. L/817 (0.181")	73.5%		1	1
Max Defl. 0.304"	30.4%		1	1
Span / Depth 12.4	n/a			1

Disclosure

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Bearing Supports

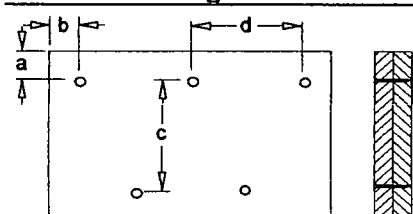
Dim. (L x W)	Value	% Allow Support	% Allow Member	Material
B0 Beam 3-1/2" x 3-1/2"	1,966 lbs	0.2%	21.4%	Steel
B1 Beam 3-1/2" x 3-1/2"	6,389 lbs	0.5%	69.5%	Steel

Notes

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Design meets User specified (L/600) Live load deflection criteria.
Design meets arbitrary (1") Maximum load deflection criteria.

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Connection Diagram



a minimum = 2" c = 7-7/8"
b minimum = 3" d = 12"

Connection design assumes point load is 'top-loaded'. For connection design of 'side-loaded' point loads, please consult a technical representative or professional of Record.
Member has no side loads.
Concentrated loads are not considered in side load analysis.
Connectors are: 16d Common Nails

Beam: **WF1**

Shape: **W8X21**

Material: **A36 Gr.36**

Length: **13 ft**

I Joint: **N1**

J Joint: **N2**

LC 2: 0+L

Code Check: **0.957 (bending)**

Report Based On 97 Sections

A ----- B

f_a ----- **ksi**

9691.247 at 0 ft

V ----- **lb**

-9691.247 at 13 ft

20.78 at 6.5 ft

f_c ----- **ksi**

M ----- **k-ft**

-31.497 at 6.5 ft

f_t ----- **ksi**

-20.78 at 6.5 ft

D ----- **in**

- 439 at 6.5 ft

AISC 13th ASD Code Check

Max Bending Check **0.957**

Max Shear Check **0.325**

Location **6.5 ft**

Location **0 ft**

Equation **H1-fb**

Max Defl Ratio **L/356**

Bending Flange **Compact**

Compression Flange **Non-Slender**

Bending Web **Compact**

Compression Web **Non-Slender**

F_y **36 ksi**

Out Plane In Plane

P_n/φ_t **59204.759 lb**

L_b **13 ft** **13 ft**

P_n/φ_v **132790.419 lb**

KL_r **123.87** **44.619**

M_n/φ_t **32.902 k-ft**

Sway **No** **No**

V_n/φ_t **29.808 lb**

L Comp Flange **13 ft**

C_b **1.136**

Beam: WF2

Shape: W8X21

Material: A36 (Fy=36)

Length: 12 ft

Point: M3

Point: M1

Code Check: 0.487 (bending)

Report Based On: 97 Sections

fa ksi

11.109 at 5.825 ft

fb ksi

-11.109 at 5.825 ft

fc ksi

-11.109 at 5.825 ft

fa ksi

5900.433 at 0 ft

fb ksi

-15.238 at 5.825 ft

fc ksi

-1.98 at 5.875 ft

AISC 13th ASD Code Check

Max Bending Check: 0.487

Location: 5.825 ft

Equation: H1-B

Bending Flange: Compact

Bending Web: Compact

Max Shear Check: 0.200

Location: 0 ft

Max C/ft Ratio: L734

Compression Flange: Non-Slender

Compression Web: Non-Slender

Fy: 36 ksi

Flange: 66719.319 lb

Web: 132790.419 lb

Area: 31.596 k-ft

Weight: 29.808 lb

CP: 1.155

Lb

12 ft

KLb

11.312

Sway

NO

Out Plane

12 ft

KLb

11.312

Sway

NO

In Plane

12 ft

KLb

11.187

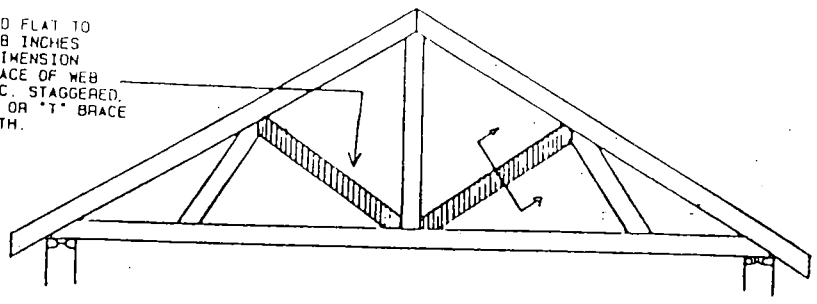
Sway

NO

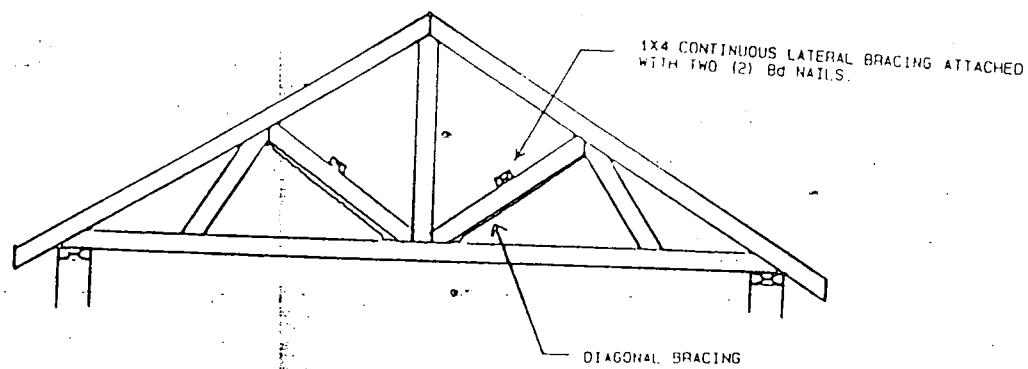
**RICHCO
STRUCTURES**

**IMPORTANT
DOCUMENTS**

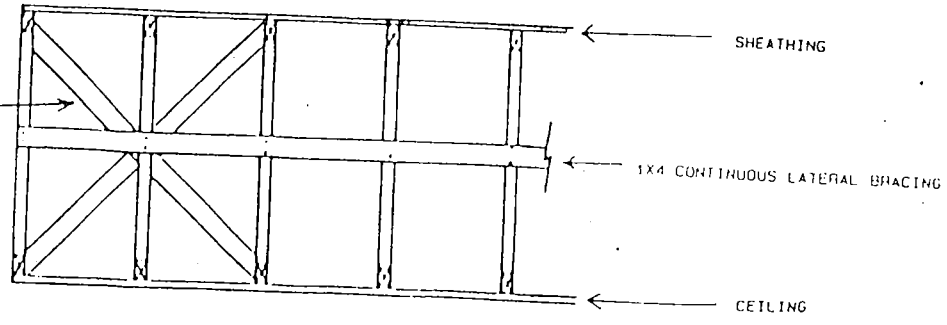
1/2" BRACE STIFFENER NAILED FLAT TO
 VE OF WEB WITH 80 NAILS AT 8 INCHES
 C. OR A SCAB (OF THE SAME DIMENSION
 O GRADE AS WEB) NAILED TO FACE OF WEB
 WITH 100 NAILS AT 8 INCHES O.C. STAGGERED.
 SEE INDICATED BY (*) SCAB OR "T" BRACE
 EXTEND FOR 90% OF WEB LENGTH.



LATERAL BRACING



DIAGONAL BRACING REPEATED AT EACH END
 OF BUILDING AND AT 20' INTERVALS



VILLAGE OF FOX POINT

SITE PLAN REVIEW



Sparco™

File Folders

SP111-1/3