INSPECTION DEPARTMENT

Certificate of Occupancy

Village of Fox Point, WI 53217

No.: 1294
Date: July 31, 2015

ISSUED TO **Rick Stratton**

OWNER **Rick Stratton**

PERMISSION IS HEREBY GRANTED TO OCCUPY **1015 E. Quarles Place**

TO BE USED FOR **Residence – Single Family**

"Section 756-13. OCCUPANCY PERMIT ---

A. INSPECTIONS.

(1) The Building Inspector shall make a final inspection of all new buildings, additions and alterations. If no violations of this or any other ordinance be found, the Building Inspector shall issue an occupancy permit, stating the purpose for which the building is to be used.

(2) No building, nor part thereof, shall be occupied until such certificate has been issued, nor shall any building be occupied in any manner which conflicts with the conditions set forth in the occupancy permit.

B. USE DISCONTINUED.

(1) Whenever any building or portion thereof is being used or occupied contrary to the provisions of this chapter, the Building Inspector shall order such use or occupancy discontinued and the building, or portion thereof, vacated by notice served on any person using or causing such use or occupancy to be continued and such person shall vacate such building or portion thereof within 10 days after receipt of the notice or make the building, or portion thereof, comply with the requirements of this chapter.

(2) Any building, structure or premises, or any part thereof, hereafter vacated or damaged by any cause whatsoever so as to jeopardize public safety or health shall not hereafter be occupied or used under an existing occupancy permit or without the same, until an application has been filed and a new occupancy permit issued.

C. CHANGE.

It shall be unlawful to change the use of any building, structure, premises, or part thereof without first obtaining, from the Building Inspector, an approval of such change in the occupancy or use and an occupancy permit therefor."

Scott Miller, Building Inspector, Village of Fox Point

VILLAGE OF FOX POINT

MILWAUKEE COUNTY
WISCONSIN

VILLAGE HALL
7200 N. SANTA MONICA BLVD.
FOX POINT 53217-3505
414-351-8900
FAX 414-351-8909

July 30, 2015

Rick & Keri Stratton
3009 N. Hackett Ave.
Milwaukee, WI 53211

RE: Occupancy Permit
1015 E. Quarles Place
Fox Point, WI 53217

Dear Mr. & Mrs. Stratton:

I made an Occupancy Inspection at the above referenced property on July 29, 2015. The inspection revealed the following code violations, issues and concerns. They are:

1) Please provide the Village with a topographical survey showing that you have complied with the grading and drainage plan that was approved by the Building Board and the Director of Public Works/Village Engineer. (ref SPS 320.02(2)(b) & FPC 756-7)
2) Please provide a code compliant cover or guard to protect your area well opening. (ref. SPS 321.04(3)(c))
3) Please obtain a building permit for the construction of your new fence and arbor. (FPC 745-7(3)(a))
4) Please obtain a permit for the installation of your satellite dish. (ref. FPC 455-2)
5) Please properly complete your furnace's venting system. (ref. SPS 323.04(b))
6) Please properly complete your water heater's venting system. (ref. SPS 323.04(b))
7) I observed that the water heater is being supplied by an extension cord. Please eliminate the extension cord and provide a code compliant receptacle outlet to supply electricity to your water heater as required by code. (ref. NEC 400-8)
8) Please provide a code compliant Ground-Fault Circuit Interrupter (GFCI) protected, 120-volt, single phase, 15 or 20 ampere receptacle outlet within 25'00" of your air-conditioning unit.
9) Please properly close all open electrical boxes. (ref. NEC 210.63)
10) I observed that the low-voltage lighting transformer is laying on top of the HVAC ductwork in your basement. Please properly support this transformer as required by code. (ref. NEC 110.3(b))

11) All open wires must terminate in a code compliant electrical box. (ref. NEC 300.15)
12) The nonmetallic-sheathed cable branch circuits that come out of the top of the basement service panelboard must be supported within twelve (12) inches of this equipment. (ref. NEC 334.30)
13) The nonmetallic-sheathed cables that are located below the first floor joist(s) shall be properly protected from damage as required by code. (ref. NEC 334.15)
14) Please properly seal all openings in the basement rim joist. (SPS 321.24)
15) Please properly firestop all openings in the first floor system. (SPS 321.085)
16) Please properly identify all circuit breakers. (ref. NEC 110.22 & 408.4(A))
17) The inspection revealed that your fireplace hearth is only 19 inches deep. This is contrary to section 321.29(6) of the Wisconsin Administrative Code which requires a fireplace hearth depth of 20 inches. As such, please provide the Village with documentation showing that you are complying with the fireplace manufacturer's installation requirements with respect to depth of your fireplace hearth. (ref SPS 321.32 & 321.29(6))
18) All 120-volt, single phase, 15 and 20 ampere branch circuits supplying outlets installed in dwelling unit family rooms, dining rooms, living rooms, parlors, libraries, dens, bedrooms, sunrooms, recreational rooms, closets, hallways, or similar rooms shall be protected by an Arc-Fault Circuit Interrupter (AFCI). (ref. 210.12(A))

For the foregoing reasons, I am hereby withholding the issuance of your Occupancy Permit at this time. Please be aware that all of the above stated code violations must be corrected within 30-days of the date of this letter unless an extension of time is granted pursuant to section 320.21 of the Wisconsin Administrative Code.

Please feel free to contact me should you have any questions with respect to this letter.

Sincerely,

Scott Miller
Building/Plumbing/Electrical Inspector
Village of Fox Point

Cc Village Manager
Village Attorney
Applebrook Construction
File

Sue Genrich

From: Scott Miller
Sent: Friday, July 31, 2015 12:39 PM
To: Sue Genrich
Subject: FW: Fireplace

Thanks,
Scott Miller

From: Rick Stratton [mailto:rstratton@feed.us]
Sent: Friday, July 31, 2015 12:24 PM
To: Scott Miller <smiller@vil.fox-point.wi.us>
Subject: Fireplace

Hello Scott,

Thank you for your patience on the fireplace hearth issue.

We will not have a fire in the fireplace until we are granted a variance or have the hearth replaced and reinspected.

Thanks again for your help through the entire building process and have a good vacation.

Regards,
Frederick Stratton (Rick)
1015 E. Quarles Place

VILLAGE OF FOX POINT

MILWAUKEE COUNTY
WISCONSIN

VILLAGE HALL
7200 N. SANTA MONICA BLVD.
FOX POINT 53217-3505
414-351-8900
FAX 414-351-8909

July 30, 2015

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3009 N. Hackett Ave.
Milwaukee, WI 53211

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1015 E. Quarles Place
Fox Point, WI 53217

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8) Please provide a code compliant Ground-Fault Circuit Interrupter (GFCI) protected, 120-volt, single phase, 15 or 20 ampere receptacle outlet within 25'00" of your air-conditioning unit.
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Fox Light

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Please feel free to contact me should you have any questions with respect to this letter.

Sincerely,

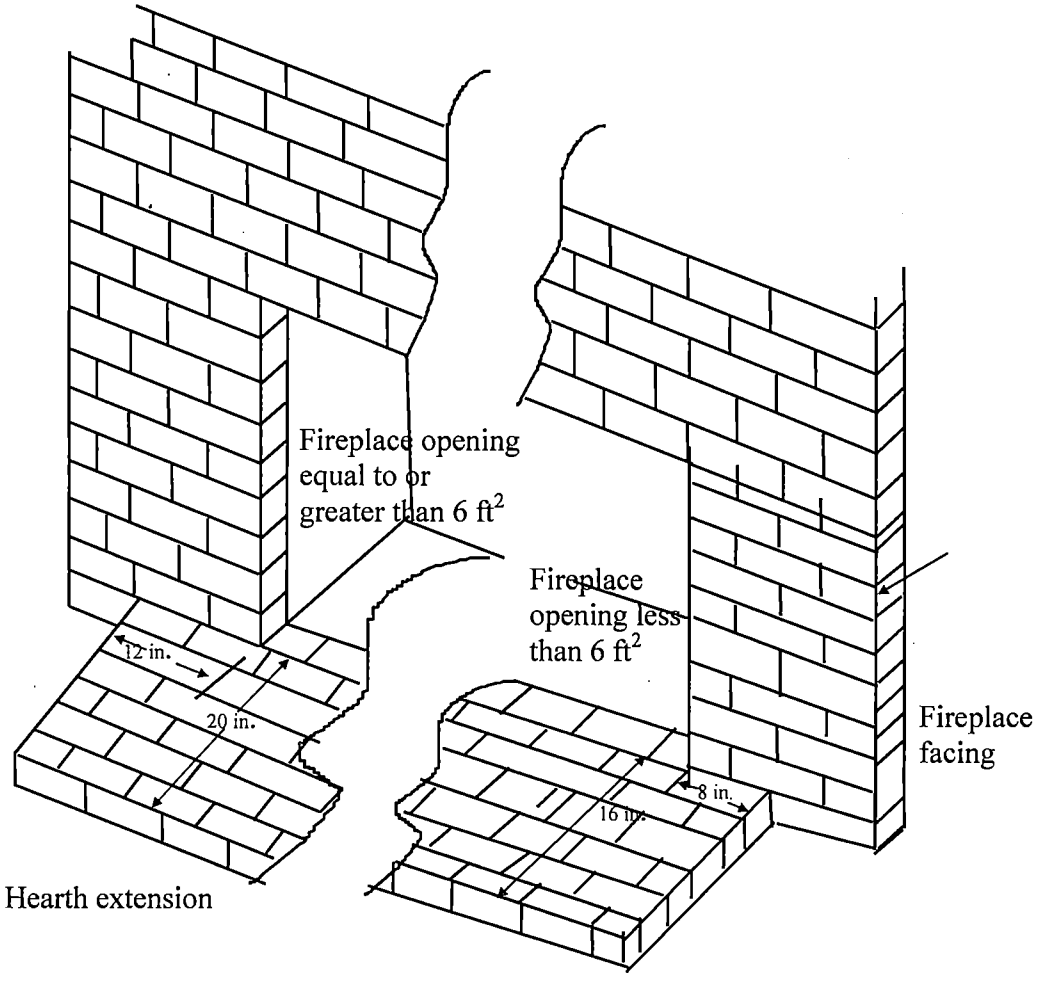
Scott Miller
Building/Plumbing/Electrical Inspector
Village of Fox Point

Cc Village Manager
Village Attorney
Applebrook Construction
File

321.29(6) Hearth Extension

Question: How is the hearth extension measured?

Answer: The hearth or hearth extension is measured from the face of the fireplace opening and not from the front of the firebox, spark screen, or glass doors. The face of the fireplace includes any trim materials provided on the front of the fireplace. Earlier editions of the UDC permitted measurement from the firebox, but as of the 1989 Edition, the measurement is to be taken from the face of the fireplace opening.



Fireplace hearth extension requirements

Scott Miller

From: Hubeler, Duane D - DSPS <Duane.Hubeler@Wisconsin.gov>
Sent: Friday, July 31, 2015 10:25 AM
To: Scott Miller
Cc: Gregory Norman
Subject: Hearth Extension

Scott, I just talked with Greg Norman on the hearth extension issue. He sent me a photo which shows the hearth extension is approximately ½ to 1" short of the required 20 inches from the front of the unit. Based on that photo and 2 or 3 past variances, I would say this petition has a high probability of being granted.

Duane Hubeler
Engineering Consultant - Uniform Dwelling Code
(608) 266-1390
duane.hubeler@wisconsin.gov

Scott Miller

From: Rick Stratton <rstratton@feed.us>
Sent: Friday, July 31, 2015 12:24 PM
To: Scott Miller
Subject: Fireplace

Hello Scott,

Thank you for your patience on the fireplace hearth issue.

We will not have a fire in the fireplace until we are granted a variance or have the hearth replaced and reinspected.

Thanks again for your help through the entire building process and have a good vacation.

Regards,
Frederick Stratton (Rick)
1015 E. Quarles Place

DIVISION OF INDUSTRY SERVICES
PO BOX 7162
MADISON WI 53707-7162
Contact Through Relay
http://dsps.wi.gov/programs/industry-services
www.wisconsin.gov

Scott Walker, Governor
Dave Ross, Secretary

November 20, 2015

CUST ID No. 1334621

RICK STRATTON
1015 E QUARLES PLACE
FOX POINT WI 53217

ATTN: One and Two Family Inspector

SCOTT MILLER
VILLAGE OF FOX POINT
7200 N SANTA MONICA BLVD
FOX POINT WI 53217-3505

APPROVAL OF PETITION FOR VARIANCE

Identification Numbers
Transaction ID No. 2632718
Site ID No. 819157
Please refer to both identification numbers, above, in all correspondence with the agency.

SITE:
Rick Stratton and Keri Saragian
1015 E Quarles Place
Village of Fox Point, 53217
Milwaukee County

FOR: Petition for Variance
SPS 321.29 (6) (d)

The submittal described above has been reviewed for equivalency to applicable Wisconsin Administrative Codes and compliance with Wisconsin Statutes. The submittal has been CONDITIONALLY APPROVED. The owner, as defined in section 101.01(10), Wisconsin Statutes, is responsible for compliance with all conditions of this petition approval and other applicable code requirements.

The code section petitioned requires a 20-inch hearth extension in front of the fireplace opening.

The variance requested is to allow approximately 1 inch of non-combustible decorative facing to infringe on the required 20-inch measurement.

The intent of the code section petitioned is to prevent a log or ember that may roll out of the firebox from contacting a combustible material.

The petitioner submitted the SB-9890 application form including 8 additional pages of supporting documents.

Reviewer's Comments:

1. The municipal building inspection department offered no comment.
2. The wording under the UDC code section mentioned above is not completely clear as to where the 20 inch measurement is measured from.
3. With this fireplace, it is clear that the firebox ends more than 20 inches from the end of the hearth extension. It is unlikely that any flaming material would be placed outside of the boundary of the firebox.
4. The installation instructions for the fireplace imply that measurements may be taken from the firebox rather than from the front of the decorative facing.

Departmental Action: **CONDITIONAL APPROVAL**

Reviewer's Conditions of Approval: No additional requirements

- All of the petitioner's statements of fact or intent included on the variance application form, any other documents submitted to the Department shall be carried out.. This variance is specific to the subject code section petitioned and the building as it will exist following completion of the current construction project and shall not be used for any additional or future modifications, additions, or alterations to the subject building.
-

This decision will become final unless the department within 30 days from the date of this letter receives a written request for a hearing. A request for hearing should be sent to the address shown on this letterhead. A copy of this letter must be included with the request for a hearing. The request for hearing should state the reasons for objecting to the department's decision, because a request for hearing may be denied if it does not present a significant question in fact, law or policy.

Inquiries concerning this correspondence may be made to me at the telephone number listed below, or at the address on this letterhead.

Sincerely,

Duane D Hubeler

Duane D Hubeler
Eng Conslt Uniform Dwelling , Division of Industry Services
(608)266-1390 , Monday-Friday 7:45 AM-4:30 pm
duane.hubeler@wisconsin.gov

Fee Required $	175.00
Fee Received $	175.00
Balance Due $	0.00

WiSMART code: 7648

cc:

Village of Fox Point
7200 N. Santa Monica Blvd.
Fox Point, WI 53217
(414) 351-8900

Date Submitted 8/29/15 No. 16270

APPLICATION FOR BUILDING Compliance

The undersigned hereby applies for a permit to build, in accordance with the information tabulated hereafter,

Type of Project ARBOR GARDEN WITH FENCE Address 1015 E QUARLES PLACE
Residence, Garage, Store, Office, School, Fence, Shed, Sign, Swimming Pool, Underground Storage Tank, Etc.

Lot _____ Block _____ Subdivision _____ District _____
Does contemplated structure violate the Village zoning ordinance? _____
Height of Structure _____ (stories or feet)
Width (parallel to highway) _____ (feet) Depth (perpendicular to highway) _____ (feet)
Distance: Street Line to Front Line of Structure _____ (feet)
Distance: Side Lot Line to Structure _____
Type of Construction: _____ Exterior finish _____
Frame, Brick-tile, etc. Stucco, Siding, Brick Veneer, Etc.

Height of front yard above street grade _____
Number of rooms _____ Baths _____
Estimated cost Garage _____
Building _____
Structure 500
Is there a private garage? _____
Does the contemplated garage violate the Village zoning ordinance? _____
Size _____ Number of Stalls _____ Where Situated _____
Have plans been submitted to the Wisconsin Department of Industry, Labor and Human Relations for examination and approval? _____
Have plans been approved as being in compliance with all applicable sections of the Wisconsin Administrative code? _____
Herewith are filed the following duplicate plans _____ in number, which I certify I will conform to in the work hereby applied for:
Remarks: CEDAR FENCE POSTS W/ DEER MESH FENCE. 16'x22' and 8' High entrance ARBOR

Herewith are filed the specifications that describe the work in question and as shown on plans above submitted.

In making the application the undersigned agrees to obey the Fox Point Building and Zoning Codes pertaining to the erection of all structures and also agrees to obey all other ordinances of the Village of Fox Point.

The undersigned, owner or being duly authorized so to do, hereby gives express authorization to the Village of Fox Point, its officers, agents and employees, to enter upon the premises herein described and fill up any excavation, or tear down, remove or enclose the unfinished structure for which a permit is herein requested in the event of cessation of the building, whenever the Building Inspector shall determine that such premises in the unfinished condition of the structure are dangerous to members of the public, including children, even though trespassers. The undersigned further hereby waives all statutory notices and consents to the determination by the Village Board and the levy and placing upon the tax roll of a special assessment in the amount of the cost to the Village, including customary Village overhead charges incurred in filling up any such excavation or tearing down, removing or enclosing any such unfinished structure.

We hereby agree to provide a house number plate or sign readily observable from the public highway which will be installed not less than 15 days after the structure is occupied.

Owner of Structure FREDERICK STRATTON Arch. or Contr. GREG NORMAN BUTLER CHASE
Address 1015 E QUARLES PLACE Address _____
City FOX POINT State WI Zip 53217 City MEQUON State WI Zip 53092
Phone 414-534-6695 Phone 414-915-7493
Size of Structure 16x22 (sq. ft.) Permit Fee $140.00 Receipt #8.000534
Dwelling Contractor Certification No. _____ Expires _____
Dwelling Contractor Qualifier Certification No. _____ Expires _____
Building Contractor Certification No. _____ Applicant Signature _____
Architect, Owner, Builder
Date of Approved 7/31/15
Builder Inspector

5

Village of Fox Point
7200 N. Santa Monica Blvd.
Fox Point, WI 53217
(414) 351-8900

Date Submitted 7/29/15 No. 16271

APPLICATION FOR BUILDING compliance

The undersigned hereby applies for a permit to build, in accordance with the information tabulated hereafter,

Type of Project? Satellite dish Address: 1015 E QUARLES PLACE
Residence, Garage, Store, Office, School, Fence, Shed, Sign, Swimming Pool, Underground Storage Tank, Etc.

Lot____ Block____ Subdivision____ District____
Does contemplated structure violate the Village zoning ordinance?____
Height of Structure____ (stories or feet)
Width (parallel to highway)____ (feet) Depth (perpendicular to highway)____ (feet)
Distance: Street Line to Front Line of Structure____ (feet)
Distance: Side Lot Line to Structure____
Type of Construction:____ Exterior finish____
Frame, Brick-tile, etc. Stucco, Siding, Brick Veneer, Etc.

Height of front yard above street grade____
Number of rooms____ Baths____
Estimated cost Garage 0
Building 0
Structure 0
Is there a private garage?____
Does the contemplated garage violate the Village zoning ordinance?____
Size____ Number of Stalls____ Where Situated____
Have plans been submitted to the Wisconsin Department of Industry, Labor and Human Relations for examination and approval?____
Have plans been approved as being in compliance with all applicable sections of the Wisconsin Administrative code?____
Herewith are filed the following duplicate plans____ in number, which I certify I will conform to in the work hereby applied for:
Remarks: DIRECTV DISH ON POLE, 3' FROM WEST SIDE OF HOUSE

Herewith are filed the specifications that describe the work in question and as shown on plans above submitted.

In making the application the undersigned agrees to obey the Fox Point Building and Zoning Codes pertaining to the erection of all structures and also agrees to obey all other ordinances of the Village of Fox Point.

The undersigned, owner or being duly authorized so to do, hereby gives express authorization to the Village of Fox Point, its officers, agents and employees, to enter upon the premises herein described and fill up any excavation, or tear down, remove or enclose the unfinished structure for which a permit is herein requested in the event of cessation of the building, whenever the Building Inspector shall determine that such premises in the unfinished condition of the structure are dangerous to members of the public, including children, even though trespassers. The undersigned further hereby waives all statutory notices and consents to the determination by the Village Board and the levy and placing upon the tax roll of a special assessment in the amount of the cost to the Village, including customary Village overhead charges incurred in filling up any such excavation or tearing down, removing or enclosing any such unfinished structure.

We hereby agree to provide a house number plate or sign readily observable from the public highway which will be installed not less than 15 days after the structure is occupied.

Owner of Structure FREDERICK STRATTON Arch. or Contr.____
Address 1015 E QUARLES PLACE Address____
City FOX POINT State WI Zip 53217 City____ State____ Zip____
Phone 414 534 6695 Phone____
Size of Structure____ (sq. ft.) Permit Fee $140.00 Receipt #8.000535
Dwelling Contractor Certification No.____ Expires____
Dwelling Contractor Qualifier Certification No.____ Expires____
Building Contractor Certification No.____ Applicant Signature____
Architect, Owner, Builder
Date of Approved 7/31/15
Builder Inspector

5

VILLAGE OF FOX POINT
MILWAUKEE COUNTY
WISCONSIN

VILLAGE HALL
7200 N. SANTA MONICA BLVD.
FOX POINT WI 53217-3505
414-351-8900
FAX 414-351-8909

CAUTIONARY STATEMENT TO OWNERS OBTAINING BUILDING PERMITS

101.65(lr) of the Wisconsin Statutes requires municipalities that enforce the Uniform Dwelling Code to provide an owner who applies for a building permit with a statement advising the owner that:

If the owner hires a contractor to perform work under the building permit and the contractor is not bonded or insured as required under s. 101.654 (2) (a), the following consequences might occur:

(a) The owner may be held liable for any bodily injury to or death of others or for any damage to the property of others that arises out of the work performed under the building permit or that is caused by any negligence by the contractor that occurs in connection with the work performed under the building permit.

(b) The owner may not be able to collect from the contractor damages for any loss sustained by the owner because of a violation by the contractor of the one- and two- family dwelling code or an ordinance enacted under sub. (1) (a), because of any bodily injury to or death of others or damage to the property of others that arises out of the work performed under the building permit or because of any bodily injury to or death of others or damage to the property of others that is caused by any negligence by the contractor that occurs in connection with the work performed under the building permit.

FREDERICK SPRATTON
Homeowner's Name – PRINTED

1015 E QUARLES PL
Fox Point Property Address

Homeowner's Signature

8/29/15
Date

Village of Fox Point
7200 N. Santa Monica Blvd.
Fox Point, WI 53217
(414) 351-8900

Date Submitted 6-8-15 No. 16225

APPLICATION FOR BUILDING

The undersigned hereby applies for a permit to build, in accordance with the information tabulated hereafter,

Type of Project PATIO WITH SEATING WALL Address 7015 E QUARLES PLACE
Residence, Garage, Store, Office, School, Fence, Shed, Sign, Swimming Pool, Underground Storage Tank, Etc.

Lot _____ Block _____ Subdivision _____ District _____
Does contemplated structure violate the Village zoning ordinance? _____
Height of Structure _____ (stories or feet)
Width (parallel to highway) _____ (feet) Depth (perpendicular to highway) _____ (feet)
Distance: Street Line to Front Line of Structure _____ (feet)
Distance: Side Lot Line to Structure _____
Type of Construction: _____ (Frame, Brick-tile, etc.) Exterior finish _____ (Stucco, Siding, Brick Veneer, Etc.)

Height of front yard above street grade _____
Number of rooms _____ Baths _____
Estimated cost. Garage _____ Building _____ Structure $14,000.00
Is there a private garage? _____
Does the contemplated garage violate the Village zoning ordinance? _____
Size _____ Number of Stalls _____ Where Situated _____
Have plans been submitted to the Wisconsin Department of Industry, Labor and Human Relations for examination and approval? _____
Have plans been approved as being in compliance with all applicable sections of the Wisconsin Administrative code? _____
Herewith are filed the following duplicate plans _____ in number, which I certify I will conform to in the work hereby applied for:
Remarks: 24' x 29' PATIO WITH 18" SEATING WALLS ON TWO (2) SIDES. INTEGRATED GRILL AT 34" HIGH.

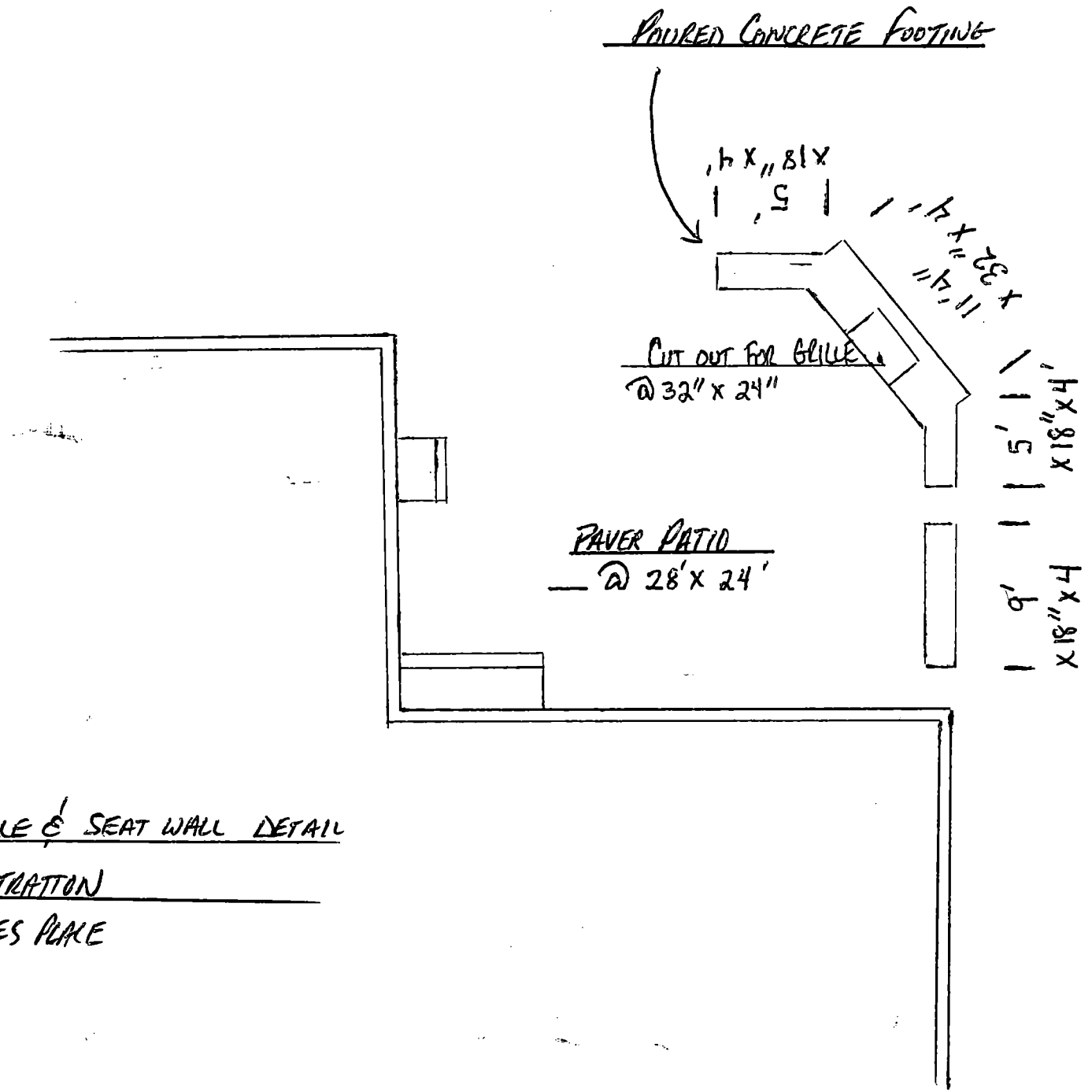
Herewith are filed the specifications that describe the work in question and as shown on plans above submitted.

In making the application the undersigned agrees to obey the Fox Point Building and Zoning Codes pertaining to the erection of all structures and also agrees to obey all other ordinances of the Village of Fox Point.

The undersigned, owner or being duly authorized so to do, hereby gives express authorization to the Village of Fox Point, its officers, agents and employees, to enter upon the premises herein described and fill up any excavation, or tear down, remove or enclose the unfinished structure for which a permit is herein requested in the event of cessation of the building, whenever the Building Inspector shall determine that such premises in the unfinished condition of the structure are dangerous to members of the public, including children, even though trespassers. The undersigned further hereby waives all statutory notices and consents to the determination by the Village Board and the levy and placing upon the tax roll of a special assessment in the amount of the cost to the Village, including customary Village overhead charges incurred in filling up any such excavation or tearing down, removing or enclosing any such unfinished structure.

We hereby agree to provide a house number plate or sign readily observable from the public highway which will be installed not less than 15 days after the structure is occupied.

Owner of Structure RICK STRATTON & KERI SARAJIAN Arch. or Contr. WIRED CONSTRUCTION MANAGEMENT DBA BUTLER-CHASE CUSTOM BUILDERS
Address 3009 N. HACKETT AVE Address _____
City MILWAUKEE State WI Zip 53211 City _____ State _____ Zip _____
Phone 414-534-6695 Phone _____
Size of Structure _____ (sq. ft.) Permit Fee 133.00 Receipt #50253
Dwelling Contractor Certification No. 1269359 Expires 5/4/16
Dwelling Contractor Qualifier Certification No. 1003500 Expires 3/14/17
Building Contractor Certification No. _____ Applicant Signature _____ (Architect, Owner, Builder)
Date of Approved 6/16/15 (Builder Inspector)

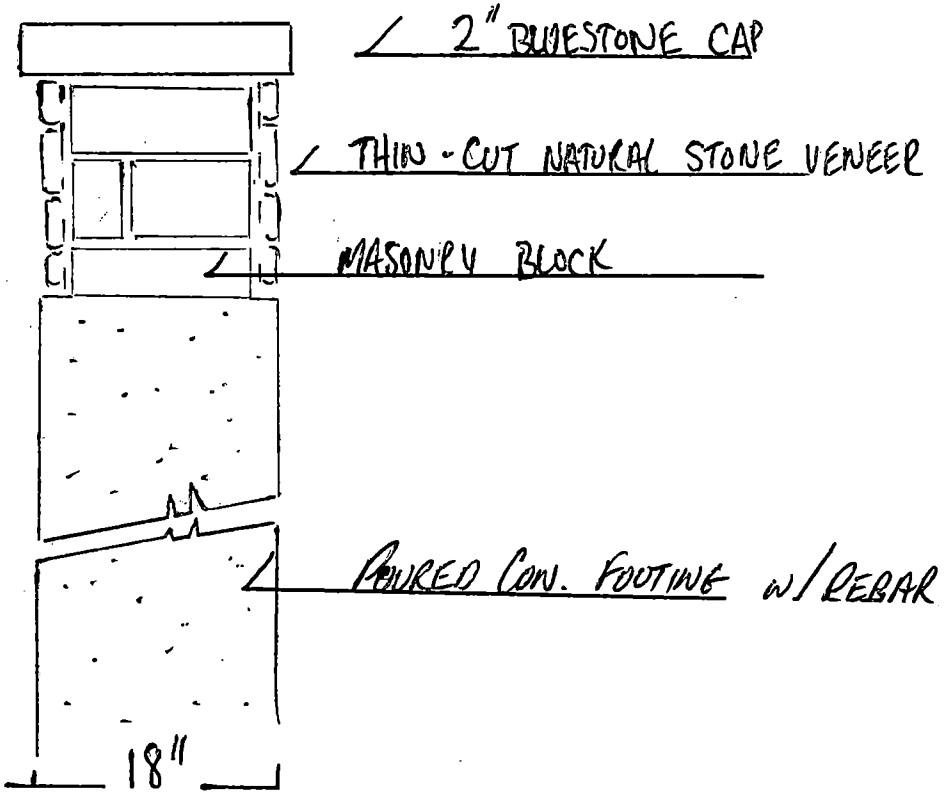


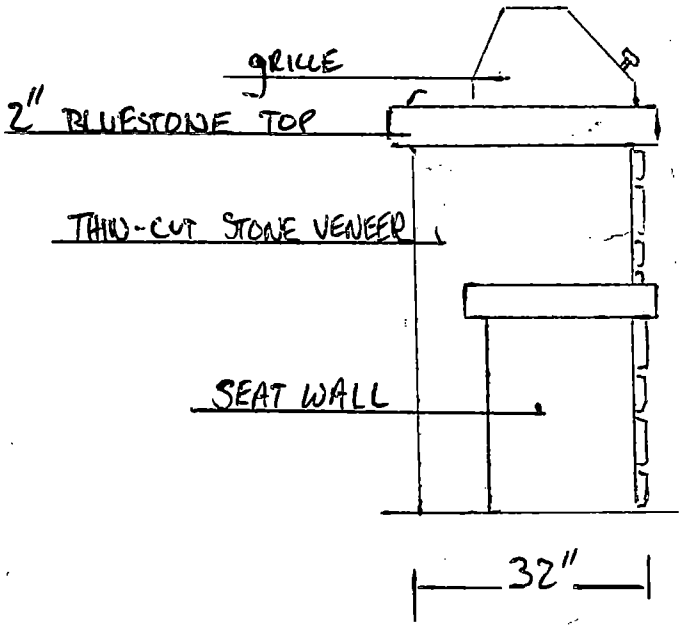
OUTDOOR GRILLE & SEAT WALL DETAIL

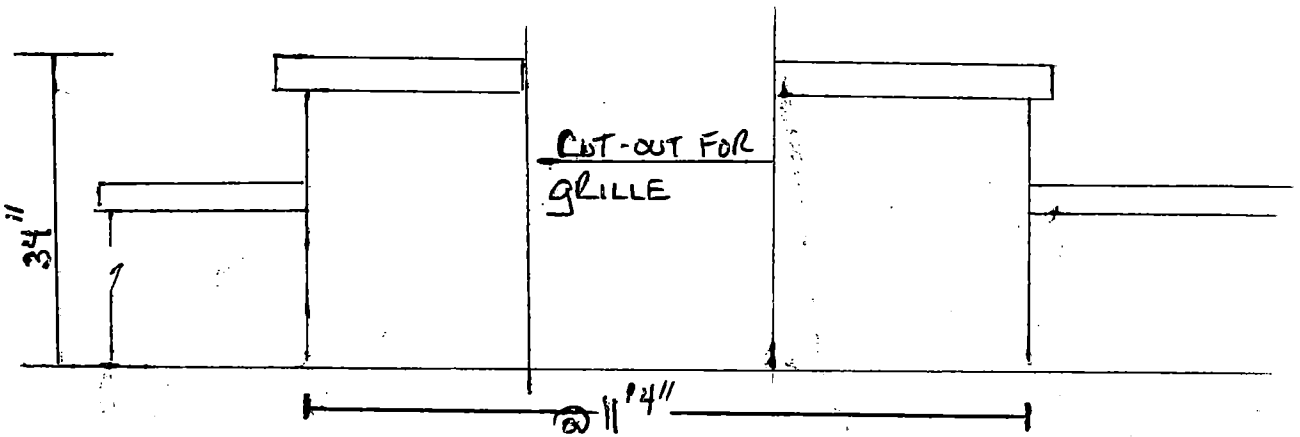
RICK & KERI STRATTON

1015 E. QUARLES PLACE

SEATING WALL




GRILLE
2" BLUESTONE TOP
THIN-CUT STONE VENEER
SEAT WALL
32"


34"
CUT-OUT FOR
GRILLE
11'4"

Scott Miller

From: Hubeler, Duane D - DSPS [Duane.Hubeler@Wisconsin.gov]
Sent: Monday, March 10, 2014 12:42 PM
To: Scott Miller
Subject: RE:

Having 2 stairways to the first floor will certainly meet the code requirement under s. SPS 321.03 (2) (a).

From: Scott Miller [mailto:smiller@vil.fox-point.wi.us]
Sent: Monday, March 10, 2014 7:57 AM
To: Hubeler, Duane D - DSPS
Subject: FW:

Duane-

In follow-up to this email, please respond in writing to the question in the first paragraph regarding the two stairways. This project is moving along quickly and they will be requesting an inspection sometime soon.

Thanks,
Scott Miller

From: Scott Miller
Sent: Friday, January 17, 2014 8:50 AM
To: Laura Johnson
Subject: FW:

From: Hubeler, Duane D - DSPS [mailto:Duane.Hubeler@Wisconsin.gov]
Sent: Thursday, January 16, 2014 2:51 PM
To: Scott Miller
Subject: RE:

From: Scott Miller [mailto:smiller@vil.fox-point.wi.us]
Sent: Thursday, January 16, 2014 11:18 AM
To: Hubeler, Duane D - DSPS
Subject:

Duane-

Thanks for getting back to me with regard to the egress question. Per your voice mail message, it appears that it is okay not to have egress windows on the second story of a dwelling, provided however, that there are two stairways that connect to the first floor. In other words, one of the stairways does not have to discharge to grade. Can you please provide me with something in writing for my file? Please let me know. Also, I have two additional questions. They are:

1) I have a contractor that is applying for a building permit. The application shows that he is planning on installing several load bearing beams. One of the beams is a 4-ply wood and 3-ply A36 steel (7-plys total) built-up flitch

plated beam with 3/8 inch bolts staggered at 24 inches o.c spacing that will be supporting another load bearing beam that attaches to its side. My concern is that the engineering is not stamped by a licensed engineer or architect. The contractor said that his designer is not an engineer. The contractor said that it would cost him an additional $1200 to provide the Village with "stamped calculations." The contractor further said that we do not have the authority to require stamped calculations. My concern is that if we grant the permit without an engineer's stamp could we be held liable should there be a structural problem in the future? It seems that the code is silent on this issue. If the house in question is post-1980, it comes under the UDC prohibition on requiring an architect or engineer's seal. SPS 320.09 (6) (c) is where the prohibition is. You can certainly require the designer to do the calculations in coordination with the framing plan. Your description doesn't mention any point loads or the depth of the beam - that's where the framing plan comes in. If the beams are going to be connected by a saddle or similar, Simpson can supply the contractor with the calcs for sizing and fastening the saddle - assuming it's a Simpson saddle. If you have very specific structural questions, I recommend you contact Steve Dobratz, who is in charge of the UDC program. His phone is (920) 492-5611.

2) I have another contractor that would like to pour a grade beam for a detached garage this time of year. The ground is frozen. Is this permissible? Can he try to thaw-out the ground prior to pouring the footings? Please let me know. A detached garage is outside the scope of the UDC, so we don't really have a horse in this race. The only way that I have seen that could work in this scenario is tenting over a torpedo heater. It would take quite awhile and cost a lot as most of the heat will go up. Good luck in thawing 2 feet of frost in your clay.

The above contractors to like me to issue their permits asap. So, your prompt reply will be most appreciated.

Please let me know if you have any questions.

Thanks,
Scott Miller

REScheck Software Version 4.4.4

Compliance Certificate

Project Title:

Energy Code:	2009 IECC
Location:	Fox Point, Wisconsin
Construction Type:	Single-family
Project Type:	New Construction
Conditioned Floor Area:	4,874 ft2
Glazing Area Percentage:	18%
Heating Degree Days:	6804
Climate Zone:	6
Permit Date:	

Construction Site: Owner/Agent: Designer/Contractor:

Compliance: Passes using UA trade-off

Compliance: 2.2% Better Than Code Maximum UA: 757 Your UA: 740

This % Better or Worse Than Code indicates how close to compliance the house is based on code trade-off rules. It DOES NOT provide an estimate of energy use or cost relative to a minimum code home.

Envelope Assemblies

Assembly	Gross Area or Perimeter	Cavity R-Value	Cont. R-Value	Glazing or Door U-Factor	UA
Wall 1: Wood Frame, 16" o.c.	4,541	21.0	0.0		209
Window 1: Wood Frame:Double Pane with Low-E	768			0.310	237
Door 1: Solid	23			0.340	8
Door 2: Glass	67			0.330	35
Basement Wall 1: Solid Concrete or Masonry Wall height: 8.0' Depth below grade: 7.3' Insulation depth: 8.0'	2,909	0.0	10.0		116
Floor 1: All-Wood Joist/Truss:Over Unconditioned Space	636	38.0	0.0		17
Floor 3: All-Wood Joist/Truss:Over Outside Air	21	38.0	0.0		1
Floor 2: Slab-On-Grade:Unheated Insulation depth: 8.0'	37		10.0		23
Ceiling 1: Flat Ceiling or Scissor Truss	2,522	38.0	0.0		78
Skylight 1: Wood Frame:Double Pane with Low-E	36			0.550	20
Ceiling 2: Cathedral Ceiling	200	25.0	0.0		6

Compliance Statement: The proposed building design described here is consistent with the building plans, specifications, and other calculations submitted with the permit application. The proposed building has been designed to meet the 2009 IECC requirements in REScheck Version 4.4.4 and to comply with the mandatory requirements listed in the REScheck Inspection Checklist.

Name - Title	Signature	Date

REScheck Software Version 4.4.4
Inspection Checklist

Requirements: 58.0% were addressed directly in the REScheck software

Text in the "Comments/Assumptions" column is provided by the user in the REScheck Requirements screen. For each requirement, the user certifies that a code requirement will be met and how that is documented, or that an exception is being claimed. Where compliance is itemized in a separate table, a reference to that table is provided.

2009 IECC	Pre-Inspection/Plan Review	Plans Verified Value	Field Verified Value	Complies?	Comments/Assumptions
103.2 [PRI]*	Construction drawings and documentation demonstrate energy code compliance for the building envelope.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	Requirement will be met. Location on plans/spec: Building Section - U factors will be added
103.3, 401.2 [PRI]*	Construction drawings and documentation demonstrate energy code compliance for lighting and mechanical systems. Systems serving multiple dwelling units must demonstrate compliance with the commercial code.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	
403.6 [PRI]*	Heating and cooling equipment is sized per ACCA Manual S based on loads per ACCA Manual J or other approved methods.	Heating: Btu/hr_____ Cooling: Btu/hr_____	Heating: Btu/hr_____ Cooling: Btu/hr_____	☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	

Additional Comments/Assumptions:

1	High Impact (Tier 1)	2	Medium Impact (Tier 2)	3	Low Impact (Tier 3)

2009 IECC	Foundation Inspection	Plans Verified Value	Field Verified Value	Complies?	Comments/Assumptions
402.1.1 [FO9]	Slab edge insulation R-value.	R-____ ☐ Unheated ☐ Heated	R-____ ☐ Unheated ☐ Heated	☐ Complies ☐ Does Not Comply ☐ Not Observable ☐ Not Applicable	See the Envelope Assemblies table for values.
303.2, 402.2.8 [FO4]	Slab edge insulation installed per manufacturer's instructions.			☐ Complies ☐ Does Not Comply ☐ Not Observable ☐ Not Applicable	Requirement will be met. Location on plans/spec: Building section
402.1.1 [FO2]	Slab edge insulation depth/length.	____ ft	____ ft	☐ Complies ☐ Does Not Comply ☐ Not Observable ☐ Not Applicable	See the Envelope Assemblies table for values.
402.1.1 [FO4]	Conditioned basement wall insulation R-value. Where interior insulation is used, verification may need to occur during insulation inspection. Not required in warm-humid locations in Climate Zone 3.	R-____	R-____	☐ Complies ☐ Does Not Comply ☐ Not Observable ☐ Not Applicable	See the Envelope Assemblies table for values.
303.2 [FO5]	Conditioned basement wall insulation installed per manufacturer's instructions.			☐ Complies ☐ Does Not Comply ☐ Not Observable ☐ Not Applicable	Requirement will be met. Location on plans/spec: Building section
402.2.7 [FO6]	Conditioned basement wall insulation depth of burial or distance from top of wall.	____ ft	____ ft	☐ Complies ☐ Does Not Comply ☐ Not Observable ☐ Not Applicable	See the Envelope Assemblies table for values.
303.2.1 [FO11]	A protective covering is installed to protect exposed exterior insulation and extends a minimum of 6 in. below grade.			☐ Complies ☐ Does Not Comply ☐ Not Observable ☐ Not Applicable	Requirement will be met. Location on plans/spec: Building section
403.8 [FO12]	Snow- and ice-melting system controls installed.			☐ Complies ☐ Does Not Comply ☐ Not Observable ☐ Not Applicable	

Additional Comments/Assumptions:

1	High Impact (Tier 1)	2	Medium Impact (Tier 2)	3	Low Impact (Tier 3)

2009 IECC	Framing/ Rough-in Inspection	Plans Verified Value	Field Verified Value	Complies?	Comments/Assumptions
402.1.1, 402.3.4 [FR1]	Door U-Factor.	U-_____	U-_____	☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	See the Envelope Assemblies table for values.
402.1.1, 402.3.1, 402.3.3, 402.5 [FR2]	Glazing U-factor (area-weighted average).	U-_____	U-_____	☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	See the Envelope Assemblies table for values.
303.1.3 [FR4]	U-factors of fenestration products are determined in accordance with the NFRC test procedure or taken from the default table.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	Requirement will be met. Location on plans/spec: will be added
402.1.1, 402.3.3, 402.5 [FR5]	Skylight U-Factor.	U-_____	U-_____	☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	See the Envelope Assemblies table for values.
402.3.6 [FR6]	Sunrooms enclosing conditioned space have a maximum fenestration U-factor of 0.50 in Climate Zones 4-8. New glazing separating the sunroom from conditioned space must meet code requirements.	U-_____	U-_____	☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	Exception: Requirement is not applicable.
402.3.6 [FR9]	Sunrooms enclosing conditioned space have a maximum skylight U-factor of 0.75 in Climate Zones 4-8.	U-_____	U-_____	☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	Exception: Requirement is not applicable.
402.4.4 [FR20]	Penetration that is not site built is listed and labeled as meeting AAMA/WDMA/CSA 101/I.S.2/A440 or has infiltration rates per NFRC 400 that do not exceed code limits.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	Requirement will be met.
402.4.5 [FR16]	Gasketed recessed lighting fixtures sealed at housing/interior finish and labeled to indicate 2.0 cfm leakage at 75 Pa.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	Requirement will be met.
403.2.1 [FR12]	Supply ducts in attics are insulated to R-8. All other ducts in unconditioned spaces or outside the building envelope are insulated to R-6.	R-_____ R-_____	R-_____ R-_____	☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	
403.2.2 [FR13]	All joints and seams of air ducts, air handlers, filter boxes, and building cavities used as return ducts are sealed.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	
403.2.3 [FR15]	Building cavities are not used for supply ducts.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	
403.3 [FR17]	HVAC piping conveying fluids above 105 °F or chilled fluids below 55 °F are insulated to R-3.	R-_____	R-_____	☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	
403.4 [FR18]	Circulating service hot water pipes are insulated to R-2.	R-_____	R-_____	☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	

1	High Impact (Tier 1)	2	Medium Impact (Tier 2)	3	Low Impact (Tier 3)

2009 IECC	Framing/ Rough-In Inspection	Plans Verified Value	Field Verified Value	Complies?	Comments/Assumptions
403.8 [FR19]	Automatic or gravity dampers are installed on all outdoor air intakes and exhausts.			☐ Complies ☐ Does Not Comply ☐ Not Observable ☐ Not Applicable	Requirement will be met. Location on plans/spec: Will be noted on HVAC drawings

Additional Comments/Assumptions:

1	High Impact (Tier 1)	2	Medium Impact (Tier 2)	3	Low Impact (Tier 3)

2009 IECC	Insulation Inspection	Plans Verified Value	Field Verified Value	Complies?	Comments/Assumptions
303.1 [RN13]*	All installed insulation is labeled or the installed R-values provided.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	Requirement will be met.
402.1.1, 402.2.5, 402.2.6 [RN1]*	Floor insulation R-value.	R-_____ ☐ Wood ☐ Steel	R-_____ ☐ Wood ☐ Steel	☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	See the Envelope Assemblies table for values.
303.2, 402.2.6 [RN2]*	Floor insulation installed per manufacturer's instructions, and in substantial contact with the underside of the subfloor.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	Requirement will be met. Location on plans/spec: Building section
402.1.1, 402.2.4, 402.2.6 [RN3]*	Wall insulation R-value. If this is a mass wall with at least ½ of the wall insulation on the wall exterior, the exterior insulation requirement applies.	R-_____ ☐ Wood ☐ Mass ☐ Steel	R-_____ ☐ Wood ☐ Mass ☐ Steel	☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	See the Envelope Assemblies table for values.
303.2 [RN4]*	Wall insulation is installed per manufacturer's instructions.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	Requirement will be met. Location on plans/spec: Building Section
402.2.11 [RN5]*	Sunroom wall insulation has a minimum R-value of R-12. New walls separating the sunroom from conditioned space must meet code requirements.	R-_____	R-_____	☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	Exception: Requirement is not applicable.
303.2 [RN6]*	Sunroom wall insulation installed per manufacturer's instructions.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	Exception: Requirement is not applicable.
402.2.11 [RN12]*	Sunroom ceiling minimum insulation R-value of R-15 in Climate Zones 1-4, and R-24 in Climate Zones 5-8.	R-_____	R-_____	☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	Exception: Requirement is not applicable.
303.2 [RN13]*	Sunroom ceiling insulation is installed per manufacturer's instructions.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	Exception: Requirement is not applicable.

Additional Comments/Assumptions:

1	High Impact (Tier 1)	2	Medium Impact (Tier 2)	3	Low Impact (Tier 3)

2009 IECC	Final Inspection Provisions	Plans Verified Value	Field Verified Value	Complies?	Comments/Assumptions
402.1.1, 402.2.1, 402.2.2 [FI1][1]	Ceiling Insulation R-value. Where > R-30 is required, R-30 can be used if insulation is not compressed at eaves. R-30 may be used for 500 ft² or 20% (whichever is less) where sufficient space is not available.	R-_____ ☐ Wood ☐ Steel	R-_____ ☐ Wood ☐ Steel	☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	See the Envelope Assembly table for values.
303.1.1.1, 303.2 [FI2][1]	Ceiling insulation installed per manufacturer's instructions. Blown insulation marked every 300 ft².			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	Requirement will be met. Location on plans/spec: Building section
402.2.3 [FI3][1]	Attic access hatch and door insulation R-value of the adjacent assembly.	R-_____	R-_____	☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	Requirement will be met. Location on plans/spec: will be added
402.4.2, 402.4.2.1 [FI17][1]	Building envelope tightness verified by blower door test result of <7 ACH at 50 Pa. This requirement may instead be met via visual inspection, in which case verification may need to occur during insulation inspection.	ACH 50 =_____	ACH 50 =_____	☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	Requirement will be met. Location on plans/spec: Note will be added to drawings
402.4.3 [FI8][1]	Wood-burning fireplaces have gasketed doors and outdoor combustion air.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	Requirement will be met. Location on plans/spec: will be added
403.2.2 [FI4][1]	Post construction duct tightness test result of 8 cfm to outdoors, or 12 cfm across systems. Or, rough-in test result of 5 cfm across systems or 4 cfm without air handler. Rough-in test verification may need to occur during Framing Inspection.	_____ cfm	_____ cfm	☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	
403.1.1 [FI9][1]	Programmable thermostats installed on forced air furnaces.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	
403.1.2 [FI10][1]	Heat pump thermostat installed on heat pumps.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	
403.4 [FI11][1]	Circulating service hot water systems have automatic or accessible manual controls.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	
403.8.1 [FI12][1]	Readily accessible switch on heaters for swimming pools.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	
403.8.2 [FI13][1]	Timer switches on pool heaters and pumps are present.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	
403.8.3 [FI20][1]	Heated swimming pools have a cover. Covers on pools heated over 90 °F are insulated to R-12.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	
404.1 [FI5][1]	50% of lamps in permanent fixtures are high efficacy lamps.			☐Complies ☐Does Not Comply ☐Not Observable ☐Not Applicable	

1	High Impact (Tier 1)	2	Medium Impact (Tier 2)	3	Low Impact (Tier 3)

2009 IECC	Final Inspection Provisions	Plans Verified Value	Field Verified Value	Complies?	Comments/Assumptions
401.3 [FI7]	Compliance certificate posted.			☐ Complies ☐ Does Not Comply ☐ Not Observable ☐ Not Applicable	Requirement will be met. Location on plans/spec: Building Section - U factors will be added
303.3 [FI16]	Manufacturer manuals for mechanical and water heating equipment have been provided.			☐ Complies ☐ Does Not Comply ☐ Not Observable ☐ Not Applicable	

Additional Comments/Assumptions:

1	High Impact (Tier 1)	2	Medium Impact (Tier 2)	3	Low Impact (Tier 3)

2009 IECC Energy Efficiency Certificate

Insulation Rating	R-Value
Wall	21.00
Floor	10.00
Ceiling / Roof	38.00
Ductwork (unconditioned spaces):	_____

Glass & Door Rating	U-Factor	SHGC
Window	0.31	
Door	0.30	
Skylight	0.55	

Heating & Cooling Equipment	Efficiency
Heating System: _____	_____
Cooling System: _____	_____
Water Heater: _____	_____

Name: _____ Date: _____

Comments:

Filing Fee $75.00

Village of Fox Point
7200 N. Santa Monica Blvd.
Fox Point, WI 53217
(414) 351-8900

Date Submitted 2/3/14 Rec#46006

No. 15547 (Part 2)

APPLICATION FOR BUILDING

The undersigned hereby applies for a permit to build, in accordance with the information tabulated hereafter,

Type of Project NEW CONSTRUCTION - MODIFICATIONS Address 1015 E QUARLES PLACE
Residence, Garage, Store, Office, School, Fence, Shed, Sign, Swimming Pool, Underground Storage Tank, Etc.

Lot _____ Block _____ Subdivision _____ District _____
Does contemplated structure violate the Village zoning ordinance? _____
Height of Structure _____ (stories or feet)
Width (parallel to highway) _____ (feet) Depth (perpendicular to highway) _____ (feet)
Distance: Street Line to Front Line of Structure _____ (feet)
Distance: Side Lot Line to Structure _____
Type of Construction: _____ Exterior finish _____
Frame, Brick-tile, etc. Stucco, Siding, Brick Veneer, Etc.

Height of front yard above street grade _____
Number of rooms _____ Baths _____
Garage _____
Estimated cost Building NO CHANGE
Structure _____
Is there a private garage? _____
Does the contemplated garage violate the Village zoning ordinance? _____
Size _____ Number of Stalls _____ Where Situated _____
Have plans been submitted to the Wisconsin Department of Industry, Labor and Human Relations for examination and approval? _____
Have plans been approved as being in compliance with all applicable sections of the Wisconsin Administrative code? _____
Herewith are filed the following duplicate plans _____ in number, which I certify I will conform to in the work hereby applied for:
Remarks: MODIFICATIONS TO SIDING, WINDOWS & MINOR COSMETIC IMPROVEMENTS

Herewith are filed the specifications that describe the work in question and as shown on plans above submitted.

In making the application the undersigned agrees to obey the Fox Point Building and Zoning Codes pertaining to the erection of all structures and also agrees to obey all other ordinances of the Village of Fox Point.

The undersigned, owner or being duly authorized so to do, hereby gives express authorization to the Village of Fox Point, its officers, agents and employees, to enter upon the premises herein described and fill up any excavation, or tear down, remove or enclose the unfinished structure for which a permit is herein requested in the event of cessation of the building, whenever the Building Inspector shall determine that such premises in the unfinished condition of the structure are dangerous to members of the public, including children, even though trespassers. The undersigned further hereby waives all statutory notices and consents to the determination by the Village Board and the levy and placing upon the tax roll of a special assessment in the amount of the cost to the Village, including customary Village overhead charges incurred in filling up any such excavation or tearing down, removing or enclosing any such unfinished structure.

We hereby agree to provide a house number plate or sign readily observable from the public highway which will be installed not less than 15 days after the structure is occupied.

Owner of Structure RICK STRATTON & KERI SARAJIAN
Address 3009 N HACKETT AVENUE
City MILWAUKEE State WI Zip 53
Phone 414-534-6695

Arch. or (Contr) WIRED CONSTRUCTION MGMT LLC (APPLEBROOK CONSTRUCTION (PARENT CO.))
Address 2022 E NORTH AVENUE
City MILWAUKEE State WI Zip 53202
Phone 414-915-7493

Size of Structure _____ (sq. ft.) Permit Fee NC Receipt _____
Dwelling Contractor Certification No. 1269359 Expires _____
Dwelling Contractor Qualifier Certification No. 1003500 Expires _____
Building Contractor Certification No. _____
Applicant Signature _____
Architect, Owner, Builder
Date of Approved 2/7/14
Builder Inspector

Wisconsin Division of Safety and Buildings

Wisconsin Stats. 101.63, 101.73

WISCONSIN UNIFORM BUILDING PERMIT APPLICATION

Instructions on back of second ply. The information you provide may be used by other government agency programs [(Privacy Law, s. 15.04 (1)(m)]

~~Application No.~~ Permit No. 15547

~~Parcel No.~~

PERMIT REQUESTED ☑ Constr. ☐ HVAC ☐ Electric ☐ Plumbing ☐ Erosion Control Other:

Owner's Name: RICK STRATTON & KERI SARAJIAN		Mailing Address: 3009 N HACKETT AVE, MILWAUKEE WI 53211	Tel. 414-534-6695
Contractor's Name: ☑Con ☐Elec ☐HVAC ☐Plbg: APPLEBROOK CONSTRUCTION, INC	Lic/Cert# 1083531	Mailing Address: 3430 W COUNTY LINE RD. MEQUON, WI 53092	Tel. 414-915-7493 FAX# 414-921-9709
Contractor's Name: ☐Con ☑Elec ☐HVAC ☐Plbg: CURRENT ELECTRIC	Lic/Cert#	Mailing Address: 12625 W BURLEIGH RD BROOKFIELD, WI 53005	Tel. 262-786-5585 FAX# 262-786-7856
Contractor's Name: ☐Con ☐Elec ☑HVAC ☐Plbg: LAKE COUNTRY HEATING & COOLING	Lic/Cert# 662679	Mailing Address: N6333 HWY F OCONOMOWOC, WI 53066	Tel. 262-593-8300 FAX# 262-593-8306
Contractor's Name: ☐Con ☐Elec ☐HVAC ☑Plbg: ALPINE PLUMBING	Lic/Cert# 226982	Mailing Address: 14580 W. GREENFIELD AVE. BROOKFIELD, WI 53005	Tel. 262-797-4130 FAX# 262-797-4135

PROJECT LOCATION Lot area 35,267 Sq. ft. ____ 1/4, ____ 1/4, of Section , T N, R E (or) W

Building Address: 1015 E QUARRIES PLACE | Subdivision Name: FOX POINT SUBDIVISION | Lot No. 4 | Block No. 1

Zoning District(s) | Zoning Permit No. | **Setbacks:** Front ft. | Rear ft. | Left ft. | Right ft.

1. PROJECT
☑ New ☐ Repair
☐ Alteration ☐ Raze
☐ Addition ☐ Move
☐ Other:

2. AREA INVOLVED
Unfin. Bsmt 2218 Sq Ft
Living Area 4214 Sq Ft
Garage 872 Sq Ft PROPOSED
Deck 575 Sq. Ft.

3. OCCUPANCY
☑ Single Family
☐ Two Family
☐ Garage
☐ Other:

4. CONST. TYPE
☑ Site-Built
☐ Mfd: ☐ WI UDC ☐ U.S. HUD

5. STORIES
☐ 1-Story
☑ 2-Story
☐ Other:
☐ Plus Basement

6. ELECTRICAL
Entrance Panel Amps: 200
☐ Underground
☐ Overhead

7. FOUNDATION
☑ Concrete
☐ Masonry
☐ Treated Wood
☐ Other:

8. USE
☐ Seasonal
☑ Permanent
☐ Other:

9. HVAC EQUIPMENT
☑ Forced Air Furnace
☐ Radiant Basebd/ Panel
☐ Heat Pump
☐ Boiler
☐ Central Air Cond.
☐ Other:

10. SEWER
☑ Municipal
☐ Sanitary Permit No.:

11. WATER
☑ Municipal Utility
☐ Private On-Site Well

12. ENERGY SOURCE

Fuel	Nat Gas	LP	Oil	Elec	Solid	Solar
Space Htg	✓					
Water Htg	✓					

☐ Dwelling unit has 3 kilowatt or more in electric space heating equipment capacity.

13. HEAT LOSS
____ BTU/HR Total Calculated Envelope and Infiltration Losses ("Maximum Allowable Heating Equipment Output" on Energy Worksheet; "Total Building Heating Load" on WIScheck report)

14. EST. BUILDING COST
$800,000.00

I agree to comply with all applicable codes, statutes and ordinances and with the conditions of this permit; understand that the issuance of the permit creates no legal liability, express or implied, on the state or municipality; and certify that all the above information is accurate. If I am an owner applying for an erosion control or construction permit, I have read the cautionary statement regarding contractor financial responsibility on the reverse side of the last ply. I expressly grant the building inspector, or the inspector's authorized agent, permission to enter the premises for which this permit is sought at all reasonable hours and for any proper purpose to inspect the work which is being done.

APPLICANT'S SIGNATURE (GREG NORMAN) **DATE SIGNED** 9/20/13

APPROVAL CONDITIONS This permit is issued pursuant to the following conditions. Failure to comply may result in suspension or revocation of this permit or other penalty. ☐ **See attached for conditions of approval.**

1) The Applicant installing A code compliant Erosion control system prior to starting the Project.
2) The Applicant providing the Village with an "As Built" survey Immediately After the Foundation System is complete to Verify Setback compliance.
3) The Applicant complying with All Applicable code Requirements

ISSUING JURISDICTION ☐ Town of ☑ Village of ☐ City of ☐ County of ☐ State Inspection Agency #: Fox Point | Municipality Number of Dwelling Location 41126

FEES:

Plan Review	$ 760.00
Inspection	$
Wis. Permit Seal	$ 30.00
Other Dep.	$ 100.00
Sewer Conn.	250.00
Total	$ 1,930.00

PERMIT(S) ISSUED
☑ Construction
☐ HVAC
☐ Electrical
☐ Plumbing
☐ Erosion Control

WIS PERMIT SEAL # 428230

PERMIT ISSUED BY:
Name Scott Miller
Date 10/1/13 Tel. (414) 351-8900
Cert No. 70229

BD-5823 (R.4/02) Distribution: ☐Ply 1 - Issuing Jurisdiction ☐Ply 2 - Municipality Forwards to State If New Dwelling ☐Ply 3 - Inspector ☐Ply 4 - Applicant

Rec# 45403 11/1/13

Wisconsin Division of Safety and Buildings

Wisconsin Stats. 101.63, 101.73

WISCONSIN UNIFORM BUILDING PERMIT APPLICATION

Instructions on back of second ply. The information you provide may be used by other government agency programs [(Privacy Law, s. 15.04 (1)(m)]

Application No.

Parcel No.

PERMIT REQUESTED ☑ Constr. ☐ HVAC ☐ Electric ☐ Plumbing ☐ Erosion Control Other:

Owner's Name	Mailing Address		Tel.
RICK STRATTON & KERI SABATIAN	3003 N HACKETT AVE MILWAUKEE WI 53211		414-534-6125
Contractor's Name: ☑Con ☐Elec ☐HVAC ☐Plbg	Lic/Cert#	Mailing Address	Tel. / FAX#
APPLEBROOK CONSTRUCTION, INC	1013531	3430 W COUNTY LINE RD. MEQUON WI 53092	Tel. 414-515-2482 FAX# 414-971-9769
Contractor's Name: ☐Con ☑Elec ☐HVAC ☐Plbg	Lic/Cert#	Mailing Address	Tel. / FAX#
CURRENT ELECTRIC		12625 W BURLEIGH RD BROOKFIELD, WI 53005	Tel. 262-780-5885 FAX# 262-780-7856
Contractor's Name: ☐Con ☐Elec ☑HVAC ☐Plbg	Lic/Cert#	Mailing Address	Tel. / FAX#
LAKE COUNTRY HEATING & COOLING	602679	N6333 HWY F OCONOMOWOC, WI 53066	Tel. 262-593-8300 FAX# 262-593-8325
Contractor's Name: ☐Con ☐Elec ☐HVAC ☑Plbg	Lic/Cert#	Mailing Address	Tel. / FAX#
ALPINE PLUMBING	226982	14580 W. GREENFIELD AVE. BROOKFIELD, WI 53005	Tel. 262-797-4130 FAX# 262-797-4135

PROJECT LOCATION Lot area 35,267 Sq. ft. ____ 1/4, ____ 1/4, of Section ____, T ____ N, R ____ E (or) W

Building Address	Subdivision Name	Lot No.	Block No.
1015 E QUARLES PLACE	FOX POINT SUBDIVISION	4	3

Zoning District(s)	Zoning Permit No.	Setbacks:	Front ft.	Rear ft.	Left ft.	Right ft.

1. PROJECT: ☑ New ☐ Repair ☐ Alteration ☐ Raze ☐ Addition ☐ Move ☐ Other:

2. AREA INVOLVED: Unfin. Bsmt 2213 Sq Ft; Living Area 4714 Sq Ft; Garage 897 Sq Ft; Deck PROPOSED 525 Sq Ft.

3. OCCUPANCY: ☑ Single Family ☐ Two Family ☐ Garage ☐ Other:

4. CONST. TYPE: ☑ Site-Built ☐ Mfd: ☐ WI UDC ☐ U.S. HUD

5. STORIES: ☐ 1-Story ☑ 2-Story ☐ Other: ☐ Plus Basement

6. ELECTRICAL: Entrance Panel Amps: 200 ☐ Underground ☐ Overhead

7. FOUNDATION: ☑ Concrete ☐ Masonry ☐ Treated Wood ☐ Other:

8. USE: ☐ Seasonal ☑ Permanent ☐ Other:

9. HVAC EQUIPMENT: ☑ Forced Air Furnace ☐ Radiant Basebd/ Panel ☐ Heat Pump ☐ Boiler ☐ Central Air Cond. ☐ Other:

10. SEWER: ☑ Municipal ☐ Sanitary Permit No.:

11. WATER: ☑ Municipal Utility ☐ Private On-Site Well

12. ENERGY SOURCE

Fuel	Nat Gas	LP	Oil	Elec	Solid	Solar
Space Htg	✓					
Water Htg	✓					

☐ Dwelling unit has 3 kilowatt or more in electric space heating equipment capacity.

13. HEAT LOSS

________ BTU/HR Total Calculated Envelope and Infiltration Losses ("Maximum Allowable Heating Equipment Output" on Energy Worksheet; "Total Building Heating Load" on WIScheck report)

14. EST. BUILDING COST $800,000.00

I agree to comply with all applicable codes, statutes and ordinances and with the conditions of this permit; understand that the issuance of the permit creates no legal liability, express or implied, on the state or municipality; and certify that all the above information is accurate. If I am an owner applying for an erosion control or construction permit, I have read the cautionary statement regarding contractor financial responsibility on the reverse side of the last ply. I expressly grant the building inspector, or the inspector's authorized agent, permission to enter the premises for which this permit is sought at all reasonable hours and for any proper purpose to inspect the work which is being done.

APPLICANT'S SIGNATURE [signature] (GREG NORMAN) **DATE SIGNED** 9/20/13

APPROVAL CONDITIONS This permit is issued pursuant to the following conditions. Failure to comply may result in suspension or revocation of this permit or other penalty. ☐ **See attached for conditions of approval.**

1) The applicant install a [illegible] [illegible] [illegible] system prior to [illegible] the project

2) The applicant provide the Village with an "As Built" survey [illegible] the foundation system is complete to verify setback compliance

3) The applicant comply with all applicable [illegible] [illegible]

ISSUING JURISDICTION ☐ Town of ☑ Village of ☐ City of ☐ County of ☐ State Inspection Agency #: Fox Point

Municipality Number of Dwelling Location 41121

FEES:		PERMIT(S) ISSUED	WIS PERMIT SEAL #	PERMIT ISSUED BY:
Plan Review	$ 76.00	☑ Construction	428230	Name [signature]
Inspection	$ [illegible]	☐ HVAC		Date [illegible] Tel. [illegible]
Wis. Permit Seal	$ 30.00	☐ Electrical		
Other Occp.	$ 150.00	☐ Plumbing		
Sewer Conn	200.00	☐ Erosion Control		
Total	$ [illegible]			Cert No. 70229

SBD-5823 (R.4/02) Distribution: ☐Ply 1 - Issuing Jurisdiction ☐Ply 2 - Municipality Forwards to State If New Dwelling ☐Ply 3 - Inspector ☐Ply 4 - Applicant

Receipt No: 1.045403 Nov 01, 2013

1015 E QUARLES PL

LICENSES & PERMITS-BUILDING PERMIT	7,600.00
24-44460 BUILDING PERMIT	
WI PERMIT SEAL	
LICENSES & PERMITS-BUILDING PERMIT	30.00
24-44460 BUILDING PERMIT	
OCCUPANCY PERMIT APPLICATION	
LICENSES & PERMITS-OCCUPANCY PERMIT	100.00
24-44420 OCCUPANCY PERMIT	
SEWER CONNECTION	
LICENSES & PERMITS-OTHER PERMIT	200.00
10-44540 OTHER PERMIT	
Total:	7,930.00

CHECK	Chk No: 2567	7,930.00
Total Applied:		7,930.00
Change Tendered:		.00

11/01/13 10:58am

VILLAGE OF FOX POINT
7200 N. SANTA MONICA BLVD
FOX POINT, WI 53217 414-351-8900

SITE INFO

SUBDIVISION _____

LOT NO. _____ BLOCK NO. _____

ZONING DISTRICT _____

_____ 1/4, _____ 1/4, SEC _____, T _____, N, R _____ E or W

PARCEL NO. _____

SETBACKS:

FRONT _____ ft REAR _____ ft

LEFT _____ ft RIGHT _____ ft

INSPECTIONS

PHASE	ROUGH	FINAL	EROSION
FOOTING			
FOUNDATION			
BSMT DRAIN TILES			
CONSTRUCTION			
PLUMBING			
HEAT/VENT/AC			
ELECTRICAL			
INSULATION			
OCCUPANCY			

CONTRACTORS

_____ # _____
G.C.

_____ # _____
HVAC

_____ # _____
ELECT.

_____ # _____
PLBG.

Work shall not proceed until the inspector has approved the various stages of construction or the 2 business day period since notification has elapsed. This permit will expire 24 months after the date of issuance if the building's exterior has not been completed. **Keep this card posted until final inspection has been made.** (WI Stats. 101.63)

WISCONSIN UNIFORM BUILDING PERMIT

WISCONSIN UNIFORM BUILDING PERMIT SEAL 428230

15547

☒ Constr ☐ HVAC ☐ Elect ☐ Plumb ☐ Erosion

Project:

Issued To

OWNER (AGENT): Rick Stratton

BUILDING SITE ADDRESS: 1015 E Quarles Place

CITY, VILLAGE, TOWN: Fox Point, WI 53217

Issued by

PERSON ISSUING: S A Miller

CERT. NO.: 7022

DATE ISSUED: 10/1/17

TELEPHONE: (414) 351-8900

Comments:

See Conditions Noted on Back of This Permit Card.

NOTICE OF NONCOMPLIANCE: This issuing jurisdiction shall notify the applicant in writing of any violations to be corrected. All cited violations shall be corrected within 30 days of notification, unless extension time is granted.

SBD-5824 (R. 05/00)

THIS BUILDING PERMIT IS SUBJECT TO THE FOLLOWING:

1. **The applicant installing a code compliant erosion control system prior to starting this project.**
2. **The applicant providing the Village with an "As-Built" survey immediately after the foundation system is completed to verify setback compliance.**
3. **The applicant complying with all applicable code requirements.**

Wisconsin Division of Safety and Buildings

Wisconsin Stats. 101.63, 101.73

WISCONSIN UNIFORM BUILDING PERMIT APPLICATION

Instructions on back of second ply. The information you provide may be used by other government agency programs [(Privacy Law, s. 15.04 (1)(m)]

~~Application No.~~ Permit No. 15547

~~Parcel No.~~

PERMIT REQUESTED ☑ Constr. ☐ HVAC ☐ Electric ☐ Plumbing ☐ Erosion Control Other:

Owner's Name: RICK STRATTON & KERI SARAJIAN		Mailing Address: 3009 N HACKETT AVE, MILWAUKEE WI 53211	Tel. 414-534-6695
Contractor's Name: ☑Con ☐Elec ☐HVAC ☐Plbg: APPLEBROOK CONSTRUCTION, INC	Lic/Cert# 1083531	Mailing Address: 3430 W COUNTY LINE RD. MEQUON, WI 53092	Tel. 414-915-7493 FAX# 414-921-9709
Contractor's Name: ☐Con ☑Elec ☐HVAC ☐Plbg: CURRENT ELECTRIC	Lic/Cert#	Mailing Address: 12625 W BURLEIGH RD BROOKFIELD, WI 53005	Tel. 262-786-5885 FAX# 262-786-7856
Contractor's Name: ☐Con ☐Elec ☑HVAC ☐Plbg: LAKE COUNTRY HEATING & COOLING	Lic/Cert# 662679	Mailing Address: N6333 HWY F OCONOMOWOC, WI 53066	Tel. 262-593-8300 FAX# 262-593-8306
Contractor's Name: ☐Con ☐Elec ☐HVAC ☑Plbg: ALPINE PLUMBING	Lic/Cert# 226982	Mailing Address: 14580 W. GREENFIELD AVE. BROOKFIELD, WI 53005	Tel. 262-797-4130 FAX# 262-797-4135

PROJECT LOCATION

Lot area 35,267 Sq. ft. _____ 1/4, _____ 1/4, of Section , T N, R E (or) W

Building Address: 1015 E QUARRIES PLACE | Subdivision Name: FOX POINT SUBDIVISION | Lot No. 4 | Block No. 1

Zoning District(s) | Zoning Permit No. | Setbacks: Front ft. | Rear ft. | Left ft. | Right ft.

1. PROJECT
☑ New ☐ Repair ☐ Alteration ☐ Raze ☐ Addition ☐ Move ☐ Other:

2. AREA INVOLVED
Unfin. Bsmt 2218 Sq Ft
Living Area 4214 Sq Ft
Garage 872 Sq Ft
Proposed Deck 575 Sq Ft.

3. OCCUPANCY
☑ Single Family ☐ Two Family ☐ Garage ☐ Other:

4. CONST. TYPE
☑ Site-Built ☐ Mfd: ☐ WI UDC ☐ U.S. HUD

5. STORIES
☐ 1-Story ☑ 2-Story ☐ Other: ☐ Plus Basement

6. ELECTRICAL
Entrance Panel Amps: 200
☐ Underground ☐ Overhead

7. FOUNDATION
☑ Concrete ☐ Masonry ☐ Treated Wood ☐ Other:

8. USE
☐ Seasonal ☑ Permanent ☐ Other:

9. HVAC EQUIPMENT
☑ Forced Air Furnace ☐ Radiant Basebd/ Panel ☐ Heat Pump ☐ Boiler ☐ Central Air Cond. ☐ Other:

10. SEWER
☑ Municipal ☐ Sanitary Permit No.:

11. WATER
☑ Municipal Utility ☐ Private On-Site Well

12. ENERGY SOURCE

Fuel	Nat Gas	LP	Oil	Elec	Solid	Solar
Space Htg	✓					
Water Htg	✓					

☐ Dwelling unit has 3 kilowatt or more in electric space heating equipment capacity.

13. HEAT LOSS
_____ BTU/HR Total Calculated Envelope and Infiltration Losses ("Maximum Allowable Heating Equipment Output" on Energy Worksheet; "Total Building Heating Load" on WIScheck report)

14. EST. BUILDING COST
$800,000.00

I agree to comply with all applicable codes, statutes and ordinances and with the conditions of this permit; understand that the issuance of the permit creates no legal liability, express or implied, on the state or municipality; and certify that all the above information is accurate. If I am an owner applying for an erosion control or construction permit, I have read the cautionary statement regarding contractor financial responsibility on the reverse side of the last ply. I expressly grant the building inspector, or the inspector's authorized agent, permission to enter the premises for which this permit is sought at all reasonable hours and for any proper purpose to inspect the work which is being done.

APPLICANT'S SIGNATURE Greg Norman (GREG NORMAN) **DATE SIGNED** 9/20/13

APPROVAL CONDITIONS This permit is issued pursuant to the following conditions. Failure to comply may result in suspension or revocation of this permit or other penalty. ☐ **See attached for conditions of approval.**

1) The Applicant Installing A Code Complt Erosion Control System Prior to Starting This Project.
2) The Applicant providing the Village with an "As Built" Survey Immediately After the Foundation System is complete to Verify Setback Compliance.
3) The Applicant Complying with All Applicable Code Requirements

ISSUING JURISDICTION ☐ Town of ☑ Village of ☐ City of ☐ County of ☐ State Inspection Agency #: Fox Point | Municipality Number of Dwelling Location: 41126

FEES:		PERMIT(S) ISSUED	WIS PERMIT SEAL #	PERMIT ISSUED BY:
Plan Review	$ 4000	☑ Construction		Name Scott Miller
Inspection	$	☐ HVAC		Date 9/11/13 Tel. (414) 351-6900
Wis. Permit Seal	$ 30.00	☐ Electrical		
Other Occup.	$ 100.00	☐ Plumbing		Cert No. 70229
Sewer Conn.	200.00	☐ Erosion Control		
Total	$ 7,930.00			

SBD-5823 (R.4/02) Distribution: ☐Ply 1 - Issuing Jurisdiction ☐Ply 2 - Municipality Forwards to State If New Dwelling ☐Ply 3 - Inspector ☐Ply 4 - Applicant

Search for Individual or Company by Category here:

Credential Type: Dwelling Contractor Qualifier

Credential Status (required): Not Expired

Zip (or first three digits): 53092

Last or Business Name: norman

Search

1 record(s) were returned by your search.

ID	Name	City,State,Zip	Credential Type	Expiration
1003500	NORMAN, GREGORY A	MEQUON WI 53092	Dwelling Contractor Qualifier	03/14/15

Search for Individual or Company by Credential ID here:

Specific Credential ID 831638

Search

1 record(s) were returned by your search.

ID	Name	City,State,Zip	Credential Type	Expiration
831638	APPLEBROOK CONSTRUCTION,	REESEVILLE WI 53579	Dwelling Contractor	04/08/14

Christine Symchych, 7240 N. Barnett Lane, proposed patio and retaining wall. It was the consensus of the Building Board to approve this application subject to the applicant complying with the administrative requirements of Village code. More specifically, the applicant must obtain an Erosion Control Permit and provide the Building Inspector with the Village Engineer's Certification for this project as required by Section 17.4 of the Village code.

Anne Zizzo, 1526 E. Goodrich Lane, proposed window and door modifications including interior/exterior alterations. Contractor: Brookwater Construction. It was the consensus of the Building Board to approve this application subject to the following conditions:

1. The applicant providing the Village with a drawing of the existing building elevations.
2. The applicant providing the Village with a drawing of the revised (proposed) building elevations.
3. The applicant providing the Village with a header schedule which shows that the structural members are properly sized.

Jon & Abigail Bloom, 7125 N. Barnett Lane, proposed exterior changes to front elevation of home. Contractor: Rossi Construction. It was the consensus of the Building Board to approve this application.

Tom & Susan Parks, 1469 E. Lilac Lane, proposed interior/exterior improvements to existing detached garage. Contractor: RP Custom Homes, Inc. It was the consensus of the Building Board to approve this application subject to the following conditions:

1. The applicant providing the Village with documentation showing that the structural columns are properly sized.

Trent & Jackie Graham, 6702 N. Lake Drive, proposed retaining wall(s) as described in the application. (Tabled from June 7, 2013 Building Board meeting). It was the consensus of the Building Board to approve this application subject to the following conditions:

1. The applicant obtaining an Erosion Control Permit for this project required by Chapter #38 of the Village code.
2. The Village Inspector receiving the Village Engineer's certification for this project as required by Section 17.4 of the Village code.
3. The Village Attorney rendering an opinion indicating that the revised application is in compliance with the Village's Bluff Ordinance.

Rick Stratton, 1015 E. Quarles Place, proposed new single family dwelling with attached garage. It was the consensus of the Building Board to approve this application subject to the following conditions:

1. The applicant providing the Village with a signed and sealed survey showing that this project complies with Village code.

2. The applicant providing the Village with wall bracing information as required by the Uniform Dwelling Code (UDC).
3. The Director of Public Works/Village Engineer approving a storm water and drainage plan as required by Village code.
4. The applicant obtaining a fill permit.
5. The applicant submitting a revised building section to the Village which shows that this home will be constructed in accordance with the Village code.
6. The roof system shall consist of a "Galvlume Plus" standing metal seam system as described during the meeting.

Adjourn

On motion of Stuart Rothman, seconded by David Seno, and unanimously carried the Building Board adjourned at 9:11 a.m.

Respectfully submitted,

Scott Miller
Building Inspector

VILLAGE OF FOX POINT
MILWAUKEE COUNTY
WISCONSIN

VILLAGE HALL
7200 N. SANTA MONICA BLVD.
FOX POINT 53217-3505
414-351-8900
FAX 414-351-8909

September 23, 2013

Mr. Frederick Stratton
Ms. Keri Sarajian
3009 North Hackett Avenue
Milwaukee, WI 53211

Re: Grading and Drainage Plan for 1015 East Quarles Place

Dear Mr. Stratton and Ms. Sarajian:

I am writing with respect to the grading and drainage plan application submitted by you on or about June 24, 2013, and revised August 22, 2013, by CJ Engineering. Kapur & Associates was retained by the Village to review the grading and drainage plan and analysis and has provided me with their comments which were dated August 19 and September 4, 2013. CJ Engineering has addressed the comments and I find that the grading and drainage plan complies with the applicable portions of the Village Code. Therefore, I approve the grading and drainage plan subject to the following:

1. A culvert permit is required prior to commencement of construction. I have included a copy of the application with this letter.

2. All erosion control features must be installed prior to the start of construction.

Should you have any further questions regarding this matter, feel free to contact me at 414-351-8900.

Sincerely,

Scott Brandmeier, P.E., Esq.
Director of Public Works
Village of Fox Point

cc: Scott Miller, Building Inspector
Yuriy Amelyan, Kapur & Associates

VILLAGE OF FOX POINT

MILWAUKEE COUNTY

WISCONSIN

VILLAGE HALL
7200 N. SANTA MONICA BLVD.
FOX POINT 53217-3505
414-351-8900
FAX 414-351-8909

September 18, 2013

Mr. Frederick Stratton
Ms. Keri Sarajian
3009 North Hackett Avenue
Milwaukee, WI 53211

Dear Mr. Stratton and Ms. Sarajian:

I am writing to inform you that, based on the recommendation from the Director of Public Works/Village Engineer, I have approved your application for a Solid Fill Permit for 1015 East Quarles Place (this letter shall serve as the permit), subject to the following conditions:

1. You and your builder, currently identified as Butler Chase of Mequon, Wisconsin, and any of its subcontractors must comply with all applicable provisions of Section 18, Fox Point Village Code (FPC), entitled "Regulation of Solid Fill and Grades".
2. The expiration date of the permit is December 17, 2013. It may be renewed not more than two times in a calendar year per Section 18.6, FPC.
3. As all fill is from on-site sources, no trucking of fill to the site is permitted.
4. Compliance with the tree protection zone fencing plan and tree replanting plan approved by the Village Forester on August 29, 2013.
5. The hours of site operation/equipment operation/truck arrival and departure are limited to 7:00 a.m. to 5:00 p.m., Monday through Friday excluding holidays.
6. Monthly progress reports must be submitted to the Director of Public Works by the 10th calendar day of each month for the preceding month's fill activities. The progress reports must contain the following information:
   a. Status of filling (percent complete);
   b. Amount of fill material placed and removed from the work site in the past month and since commencement of the project; and
   c. Updated filling project schedule.
7. Clean-up of dirt and debris left on Village streets as a result of excavation and filling operations must be performed at the end of each working day. Street sweeping must be done as needed but, at minimum, no less than once per week. Any exception to this must be approved by the Director of Public Works.
8. Erosion control measures for the fill area, such as the use of silt fencing, must remain in place until vegetation is established at the property. Vegetation must be adequately established by July 31, 2014.

Mr. Rick Stratton
Ms. Keri Sarajian
1015 East Quarles Place
Fill Permit
September 17, 2013

9. Any proposed changes or amendments to the Fill Application and supplemental information that you submitted must be reviewed and approved by the Village before such changes or amendments are executed.
10. The Village reserves the right to request additional information and/or to impose additional requirements should the Village deem necessary.

Please contact Director of Public Works Scott Brandmeier if further assistance is needed with respect to this permit.

Sincerely,

Melissa Bohse, Village Manager

cc: Director of Public Works
Police Chief
Village Inspector

1015 E QUARLES PL.

MiTek USA, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO 63017-5746
314-434-1200

Re: r-5902-13
Stratton Residence

The truss drawing(s) referenced below have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Accurate Housing Systems, Inc..

Pages or sheets covered by this seal: I21692323 thru I21692323

My license renewal date for the state of Wisconsin is July 31, 2014.

Wisconsin COA: 726-011

Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.

January 14,2014

Fox, Steve

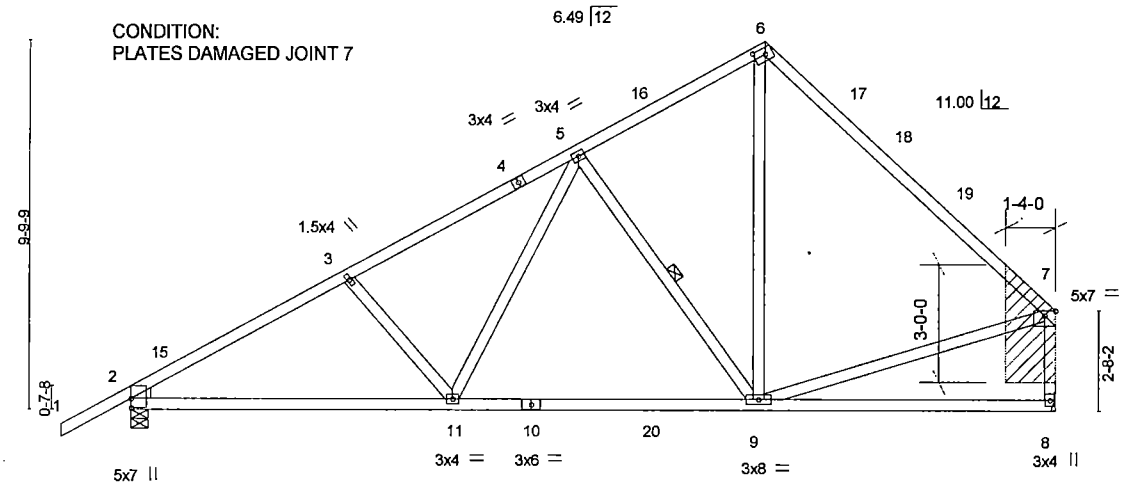
The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI 1.

Job	Truss	Truss Type	Qty	Ply	Stratton Residence	
R-5902-13	A2	ROOF SPECIAL	9	1	UNITS: 1.0 ENG: AJ Job Reference (optional)	I21692323

Accurate Housing Systems Inc., East Troy, WI — 7.430 s Jul 25 2013 MiTek Industries, Inc. Tue Jan 14 13:40:06 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-1jucTndoUWMv4aM0RxHXjqQBKm5?TULry9PE42zvQTd

-1-10-8	5-10-3	11-11-12	16-11-8	24-8-12
1-10-8	5-10-3	6-1-9	4-11-13	7-9-4

Scale = 1:61.6



ATTACH 1/2" PLYWOOD OR OSB GUSSET (15/32" APA RATED SHEATHING 32/16 EXP 1)
TO EACH FACE OF TRUSS WITH 10d (3" X .131") NAILS DRIVEN THROUGH BOTH SHEETS
OF PLYWOOD AND CLINCHED PER THE FOLLOWING NAIL SCHEDULE:
2 x 4's - 2 ROWS: SPACED @ 0-4-0 O.C.
NAILS TO BE DRIVEN FROM BOTH FACES. STAGGER SPACING FROM FRONT TO BACK FACE FOR A
NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

8-7-8	16-11-8	24-8-12
8-7-8	8-4-0	7-9-4

Plate Offsets (X,Y): [6:0-3-9,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.84	Vert(LL)	-0.27	9-11	>999	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.80	Vert(TL)	-0.41	9-11	>715	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.33	Horz(TL)	0.05	8	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.04	9-11	>999	240	Weight: 110 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud *Except*
7-8: 2x4 SPF No.2
WEDGE
Left: 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 2-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-9

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1449/0-5-8 (min. 0-2-4), 8=1288/Mechanical
Max Horz 2=267(LC 5)
Max Uplift 2=-75(LC 6), 8=-7(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2057/66, 3-5=-1782/83, 5-6=-987/134, 6-7=-1259/98, 7-8=-1231/70
BOT CHORD 2-11=-148/1711, 9-11=-38/1214
WEBS 3-11=-379/108, 5-11=0/574, 5-9=-777/73, 6-9=0/690, 7-9=-9/698

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-10-8 to 1-1-8, Interior(1) 1-1-8 to 13-11-8, Exterior(2) 13-11-8 to 16-11-8, Interior(1) 19-11-8 to 21-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Plates checked for a plus or minus 5 degree rotation about its center.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

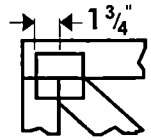
LOAD CASE(S) Standard

January 14,2014

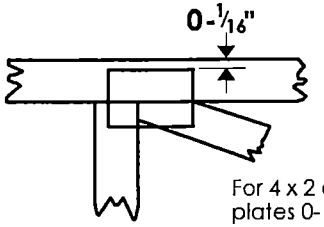
MiTek
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

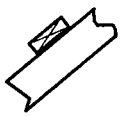
***Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

4 x 4

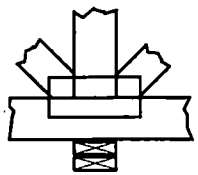
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

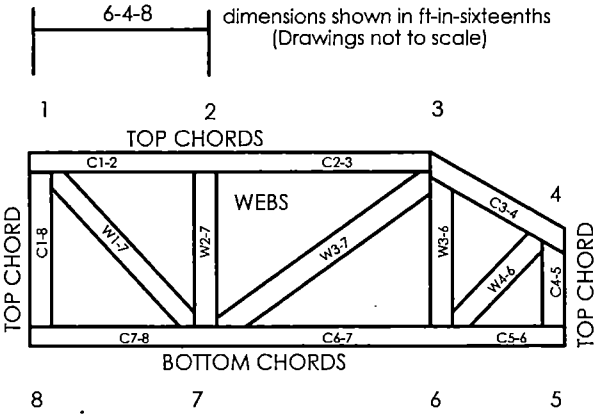


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Southern Pine lumber designations are as follows:

SYP represents values as published by AWC in the 2005/2012 NDS
SP represents ALSC approved/new values with effective date of June 1, 2013

MiTek

MiTek Engineering Reference Sheet: MII-7473 rev. 02/26/2013

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

DELIVERY SHIPLIST

ACCURATE HOUSING SYSTEMS INC
2624 Corporate Circle, East Troy, WI 53120
P: 262-642-3800 F: 262-642-2716
www.accuratehousing.com

ORDER DATE	12/02/13
REQUESTED DATE	01/10/14
DELIVERY DATE	**01/10/14**

DELIVERY INFO (ASSUMED ROLL-OFF UNLESS NOTED):
Roll off delivery

ORDER #

R-5902-13

DESIGNER: Justin Nagl Ext. 7013

SOLD TO: Wisconsin Building Supply -

SHIP TO: 1015 E. Quarles Pl
Fox Point, WI

JOB NAME: Stratton Residence LOT # SUBDIVISION:
MODEL: ELEVATION: RAKE SHEATH MAT:

SPECIAL INSTRUCTIONS:
NEW PRICE REFLECTS DELETING A1E, B1E & D1E GABLES PER FRAMER VIA BOB YURK ON 12/30/13.

**** C1E STRUCTURAL GABLE IS NOT DELETED AND WILL STILL BE PROVIDED ***

ROOF GIRDERS ARE BEING SHIPPED PRE-NAILED? *YES* (if incorrect, AHS was not informed and charge-backs cannot be accepted. Contact lumber yard to correct on future projects.)

PROFILE	QTY	PITCH		TYPE	BASE	O/A	LUMBER		OVERHANG		CANTILEVER		STUB		
	PLY	TOP	BOT	ID	SPAN	SPAN	TOP	BOT	LEFT	RIGHT	LEFT	RIGHT	LEFT	RIGHT	
	9	6.49	0.00	ROOF A2	24-08-12	24-08-12	2 X 4	2 X 4	01-10-08						
	1	6.49	0.00	ROOF A3	24-08-12	24-08-12	2 X 4	2 X 4							
	1 / 2 Ply	6.49	0.00	ROOF A4G	24-10-12	24-10-12	2 X 6	2 X							
	1	6.49	0.00	VALLEY AV1	10-00-01	10-00-01	2 X 8	2 X 4		04-08-10					
	1	6.49	0.00	VALLEY AV2	08-00-01	08-00-01	2 X 8	2 X 4		04-08-10					
	1	6.49	0.00	VALLEY AV3	06-00-00	06-00-00	2 X 4	2 X 4							
	1	6.49	0.00	VALLEY AV4	03-11-15	03-11-15	2 X 4	2 X 4							
	1	6.49	0.00	VALLEY AV5	01-11-15	01-11-15	2 X 4	2 X 4							
	6	11.00	0.00	ROOF B2	22-02-03	22-02-03	2 X 4	2 X 4							
	1	6.50	0.00	GABLE C1E	34-10-09	34-10-09	2 X 4	2 X 4	01-10-08						
	4	6.50	0.00	ROOF C2	34-10-09	34-10-09	2 X 4	2 X 4	01-10-08						
	7	6.50	0.00	ROOF C3	34-10-09	34-10-09	2 X 4	2 X 4					00-04-00		
	1	4.81	0.00	VALLEY CV1	17-03-00	17-03-00	2 X 4	2 X 4							
	1	4.81	0.00	VALLEY CV2	12-08-02	12-08-02	2 X 4	2 X 4							
	1	4.81	0.00	VALLEY CV3	08-01-04	08-01-04	2 X 4	2 X 4							
	3	11.00	0.00	COMMON D2	10-09-12	10-09-12	2 X 4	2 X 4		01-07-08					
	9	11.00	0.00	COMMON D3	10-09-12	10-09-12	2 X 4	2 X 4							
	1	11.00	0.00	COMMON D4	10-09-12	10-09-12	2 X 4	2 X 4							
	5	11.00	0.00	ROOF D5	10-09-12	10-09-12	2 X 4	2 X 4							
	1	11.00	0.00	ROOF D6	10-09-12	10-09-12	2 X 6	2 X 4							

DELIVERY SHIPLIST

ACCURATE HOUSING SYSTEMS INC
2624 Corporate Circle, East Troy, WI 53120
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Roll off delivery

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MODEL: ELEVATION: RAKE SHEATH MAT:

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**** C1E STRUCTURAL GABLE IS NOT DELETED AND WILL STILL BE PROVIDED ***

ROOF GIRDERS ARE BEING SHIPPED PRE-NAILED? *YES* (if incorrect, AHS was not informed and charge-backs cannot be accepted. Contact lumber yard to correct on future projects.)

PROFILE	QTY / PLY	PITCH TOP	PITCH BOT	TYPE ID	BASE SPAN	O/A SPAN	LUMBER TOP	LUMBER BOT	OVERHANG LEFT	OVERHANG RIGHT	CANTILEVER LEFT	CANTILEVER RIGHT	STUB LEFT	STUB RIGHT	
	1 / 4 Ply	0.00	0.00	FLAT GIRDER EG	26-03-08	26-03-08	2 X 8	2 X 8							
EG NOTES: INSTALL SCREWS IN GIRDERS AT PLANT! 2 ROWS @ 2' O.C. TOP CHORD & BOTTOM CHORD. MARK UP!															
	1 / 3 Ply	0.00	0.00	FLAT GIRDER FG	39-00-00	39-00-00	2 X 8	2 X							
FG NOTES: MARK UP!															

ITEMS

QTY	ITEM TYPE	SIZE	LENGTH FT-IN-16	PART NUMBER	NOTES	
60	SIMPSON HANGERS	*INSTALLED* SDS .25" X 6" scre			Do NOT Ship Loose! Screws already installed in trusses!	
55	SIMPSON HANGERS	H2.5T		SHEET U-1	Standard double-plate hurricane clips included.	
10	SIMPSON HANGERS	HTS30C				
6	SIMPSON HANGERS	HUS28		SHEET R-1		
19	SIMPSON HANGERS	LUS24		SHEET R-1		
16	SIMPSON HANGERS	LUS28		SHEET R-1		

Total No. of Components: 64

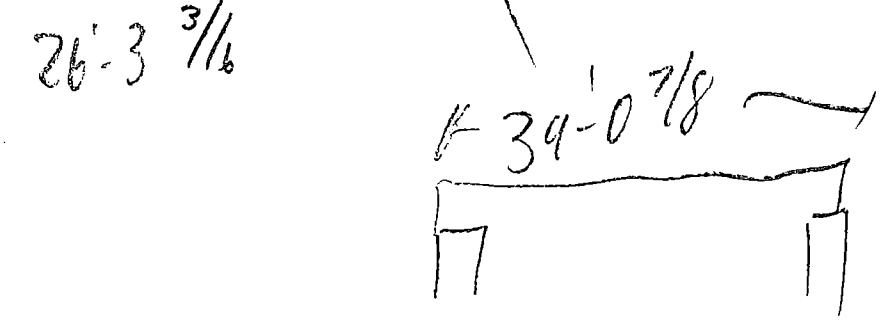
Total Package Weight: 7862

SHIPMENT VERIFIED BY:

OFFICE:_____
YARD:_____
DRIVER:_____

TRUSS LABEL CLARIFICATION:
FORMAT: JOB #-BATCH#* TRUSS ID WEB/CHORD NUMBER
(*Batch number may be found on included "BATCH LAYOUT", if included)

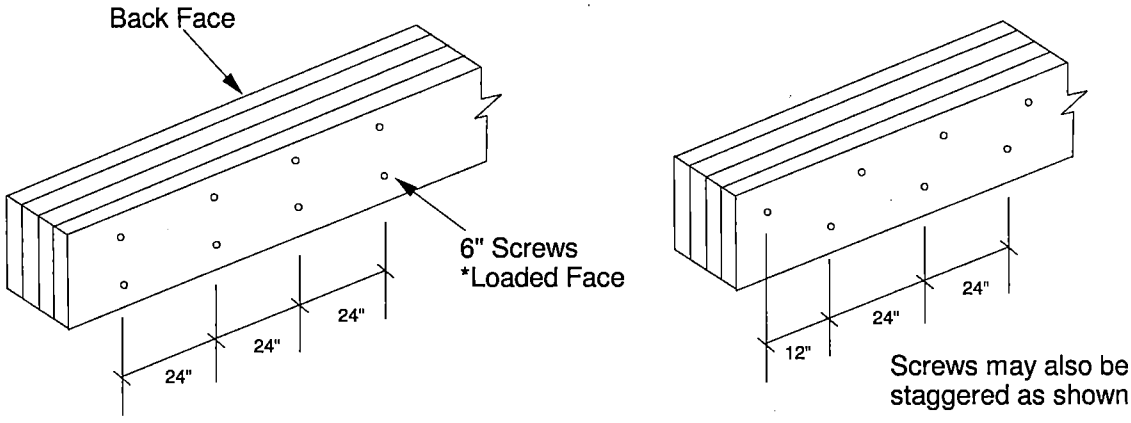
EX. 1: R123412-2 C4 W3 = (ROOF JOB #1234-12 - BATCH 2 TRUSS C4 WEB 3)
EX. 2: F001012-8 F3 T2 = (FLOOR JOB #0010-12 - BATCH 8 TRUSS F3 TOP CHORD 2)



October 16, 2013	STANDARD BOLT TO SCREW TRUSS CONNECTION DETAIL	ST-4PLY SCREW

MiTek USA, Inc.

Four ply girder trusses are to be connected together using the nailing or screw schedule provided by MiTek 20/20 software. In addition to the nailing typically specified, 1/2" dia. bolts are sometimes specified throughout certain chords as indicated on the truss design drawing. In lieu of these bolts, the following wood screws may be used: USP WS6, FastenMaster Trusslok-Z 6", *Simpson SDS1/4 x 6, Simpson SDW22600 (for assemblies between 6" and 6 3/16"), or Simpson SDW22638 (for assemblies btween 6 3/16" and 6 3/8" thick).
These screws are to be installed in two rows spaced 24"o.c. in 2x 6 and larger chords (use one row in 2x 4 chords) as shown in the detail below.
These connections are intended to provide clamping force to aid in allowing the four ply assembly to act as a unit and are not included in the calculation of ply to ply load transfer.

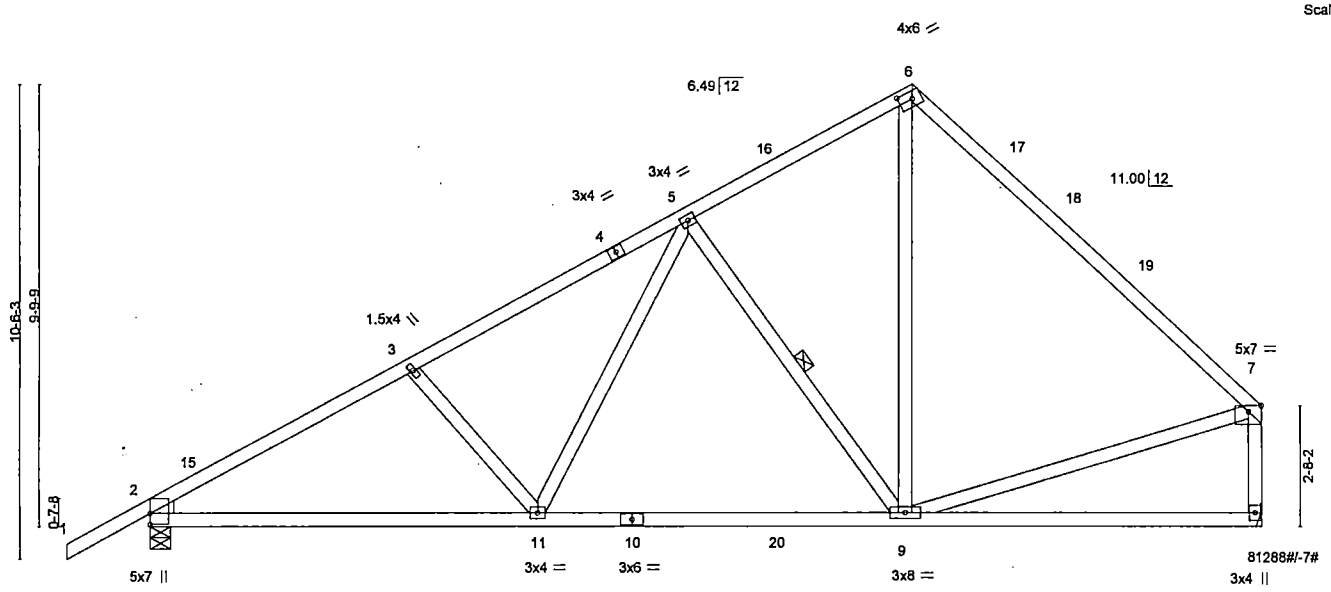


It is vitally important that the plies are tightly clamped together during the installation of the screws to prevent gaps between the plies.
USP WS6, FastenMaster Trussloc-Z6" and Simpson SDW screws may be installed from either face.
*Note that Simpson SDS Screws must be installed from the loaded face.

For trusses where screws are specified for the ply to ply connection instead of nails, the bolts called out in the connection notes on the truss design may be omitted.

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	A2	ROOF SPECIAL	9	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:13 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-T8vUbKjGuKtU55l9NBi5viN7uvS6?XKBrMTBfGzzRne



	0-5-8(0-2-4)	8-7-8	16-11-8	24-8-12
		8-7-8	8-4-0	7-9-4

Plate Offsets (X,Y): [6:0-3-9,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.84	Vert(LL)	-0.27	9-11	>999	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.80	Vert(TL)	-0.41	9-11	>715	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.33	Horz(TL)	0.05	8	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.04	9-11	>999	240	Weight: 110 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud *Except*
W6: 2x4 SPF No.2
WEDGE
Left: 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 2-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-9

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1449/0-5-8, 8=1288/Mechanical
Max Horz 2=267(LC 5)
Max Uplift2=-75(LC 6), 8=-7(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-15=-2057/40, 3-15=-2035/66, 3-4=-1782/65, 4-5=-1606/83, 5-16=-987/108, 6-16=-816/134, 6-17=-942/98, 17-18=-1009/74, 18-19=-1051/68, 7-19=-1259/61, 7-8=-1231/70
BOT CHORD 2-11=-148/1711, 10-11=-38/1214, 10-20=-38/1214, 9-20=-38/1214
WEBS 3-11=-379/108, 5-11=0/574, 5-9=-777/73, 6-9=0/690, 7-9=-9/698

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-10-8 to 1-1-8, Interior(1) 1-1-8 to 13-11-8, Exterior(2) 13-11-8 to 16-11-8, Interior(1) 19-11-8 to 21-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Plates checked for a plus or minus 5 degree rotation about its center.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.
8) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	A3	Roof Special	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:15 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-PX0F00kWQx8CLOvYVbkZ?7TTNj7zTRIUlgylk8zzRnc

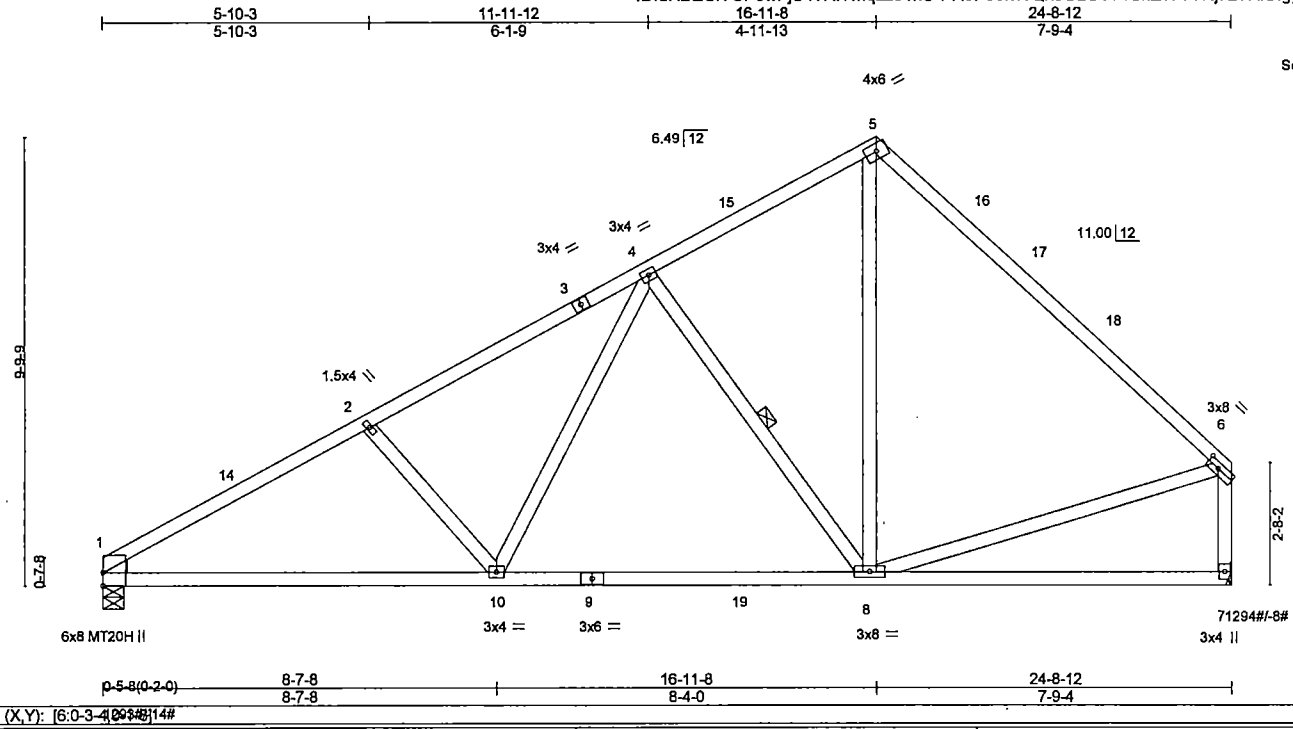


Plate Offsets (X,Y): [6:0-3-4,0-3-8],[14:0-3-8,0-3-14#]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.85	Vert(LL)	-0.26	8-10	>999	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.77	Vert(TL)	-0.39	8-10	>747	180	MT20H	148/108
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.33	Horz(TL)	0.05	7	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.03	8-10	>999	240	Weight: 108 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud *Except*
W6: 2x4 SPF No.2
WEDGE
Left: 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 2-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-8

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1293/0-5-8, 7=1294/Mechanical
Max Horz 1=251(LC 5)
Max Uplift1=-14(LC 6), 7=-8(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-14=-2090/61, 2-14=-1911/87, 2-3=-1811/85, 3-4=-1597/102, 4-15=-991/111, 5-15=-821/136, 5-16=-948/102, 16-17=-1015/78, 17-18=-1057/71, 6-18=-1265/64, 6-7=-1236/74
BOT CHORD 1-10=-164/1746, 9-10=-39/1225, 9-19=-39/1225, 8-19=-39/1225
WEBS 2-10=-398/117, 4-10=0/602, 4-8=-788/79, 5-8=0/694, 6-8=-10/702

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 13-11-8, Exterior(2) 13-11-8 to 16-11-8, Interior(1) 19-11-8 to 21-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) All plates are MT20 plates unless otherwise indicated.
4) Plates checked for a plus or minus 5 degree rotation about its center.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.
9) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	A4G	Roof Special Girder	1	2	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:17 2014 Page 1
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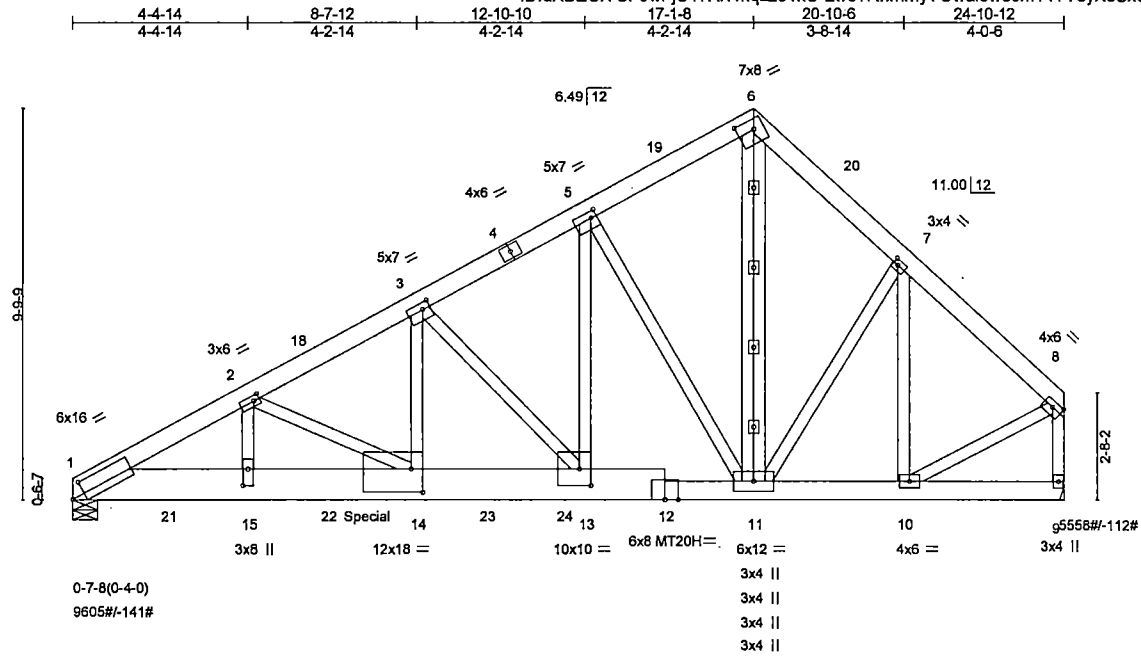


Plate Offsets (X,Y): [1:0-3-15,0-3-12], [2:0-1-12,0-1-8], [3:0-2-4,0-2-0], [5:0-1-12,0-2-0], [6:0-5-0,0-3-0], [7:0-1-8,0-1-8], [8:0-3-0,0-1-12], [13:0-3-8,0-5-0], [14:0-3-8,0-7-0], [15:0-5-0,0-1-8]

LOADING (psf)		SPACING 2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase 1.15	TC	0.84	Vert(LL)	-0.20	14	>999	360	MT20	197/144
TCDL	10.0	Lumber Increase 1.15	BC	0.84	Vert(TL)	-0.39	14	>757	180	MT20H	148/108
BCLL	0.0 *	Rep Stress Incr NO	WB	0.90	Horz(TL)	0.09	9	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002	(Matrix-M)		Wind(LL)	0.08	14	>999	240	Weight: 411 lb	FT = 10%

LUMBER
TOP CHORD 2x6 SPF No.2
BOT CHORD 2x6 SPF No.2 *Except*
B1: 2x10 SP 2400F 2.0E
WEBS 2x4 SPF Stud *Except*
W3,W5: 2x4 SPF No.2, W4: 2x4 SPF 1650F 1.5E, W6: 2x4 SPF 2400F 2.0E

BRACING
TOP CHORD Sheathed or 2-5-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=9605/0-7-8, 9=5558/Mechanical
Max Horz 1=250(LC 4)
Max Uplift1=-141(LC 5), 9=-112(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-17661/261, 2-18=-13779/205, 3-18=-13734/220, 3-4=-8620/184, 4-5=-8546/196, 5-19=-4651/179, 6-19=-4567/187, 6-20=-5450/209, 7-20=-5625/190, 7-8=-4897/131, 8-9=-5440/127
BOT CHORD 1-21=-237/15475, 15-21=-237/15475, 15-22=-237/15475, 14-22=-237/15475, 14-23=-173/12089, 23-24=-173/12089, 13-24=-173/12089, 12-13=-100/7528, 11-12=-102/7536, 10-11=-5/3477
WEBS 2-15=-7/3379, 2-14=-3847/112, 3-14=-70/6987, 3-13=-6689/127, 5-13=-70/7351, 5-11=-6804/123, 6-11=-152/5054, 7-11=-84/1117, 7-10=-1833/54, 8-10=-21/3939

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x10 - 2 rows staggered at 0-5-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-3-0 oc, Except member 14-2 2x4 - 1 row at 0-9-0 oc, member 3-14 2x4 - 1 row at 0-9-0 oc, member 13-3 2x4 - 1 row at 0-9-0 oc, member 5-13 2x4 - 1 row at 0-9-0 oc, member 11-5 2x4 - 1 row at 0-9-0 oc, member 11-6 2x4 - 1 row at 0-9-0 oc, member 6-11 2x4 - 1 row at 0-9-0 oc, member 11-7 2x4 - 1 row at 0-9-0 oc, member 7-10 2x4 - 1 row at 0-9-0 oc, member 10-8 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
5) All plates are MT20 plates unless otherwise indicated.
6) Plates checked for a plus or minus 5 degree rotation about its center.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) Refer to girder(s) for truss to truss connections.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=141, 9=112.
11) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
12) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 300 lb down and 21 lb up at 17-1-8 on top chord, and 1934 lb down and 26 lb up at 2-4-12, 1934 lb down and 26 lb up at 4-4-12, 1934 lb down and 26 lb up at 6-4-12, 1934 lb down and 26 lb up at 8-4-12, and 1934 lb down and 26 lb up at 10-4-12, and 1934 lb down and 26 lb up at 12-4-2 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
14) Special hanger(s) or other connection device(s) shall be provided at 7-4-7 from the left end sufficient to connect truss(es) to back face of bottom chord. The design/selection of such special connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced) + Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-19=-80, 6-19=-130(F=-50), 6-8=-130(F=-50), 1-12=-20, 9-12=-50(F=-30)
Concentrated Loads (lb)
Vert: 6=-300(F) 15=-1934(B) 14=-1934(B) 21=-1934(B) 22=-1934(B) 23=-1934(B) 24=-1934(B)

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	AV1	Valley	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:18 2014 Page 1
ID:dKBEClYCF3wFjS4?RITmqzze1kC-q6iNe1nOjsWnCse7AklGcl43xwJygrFw_eByKTzzRnZ

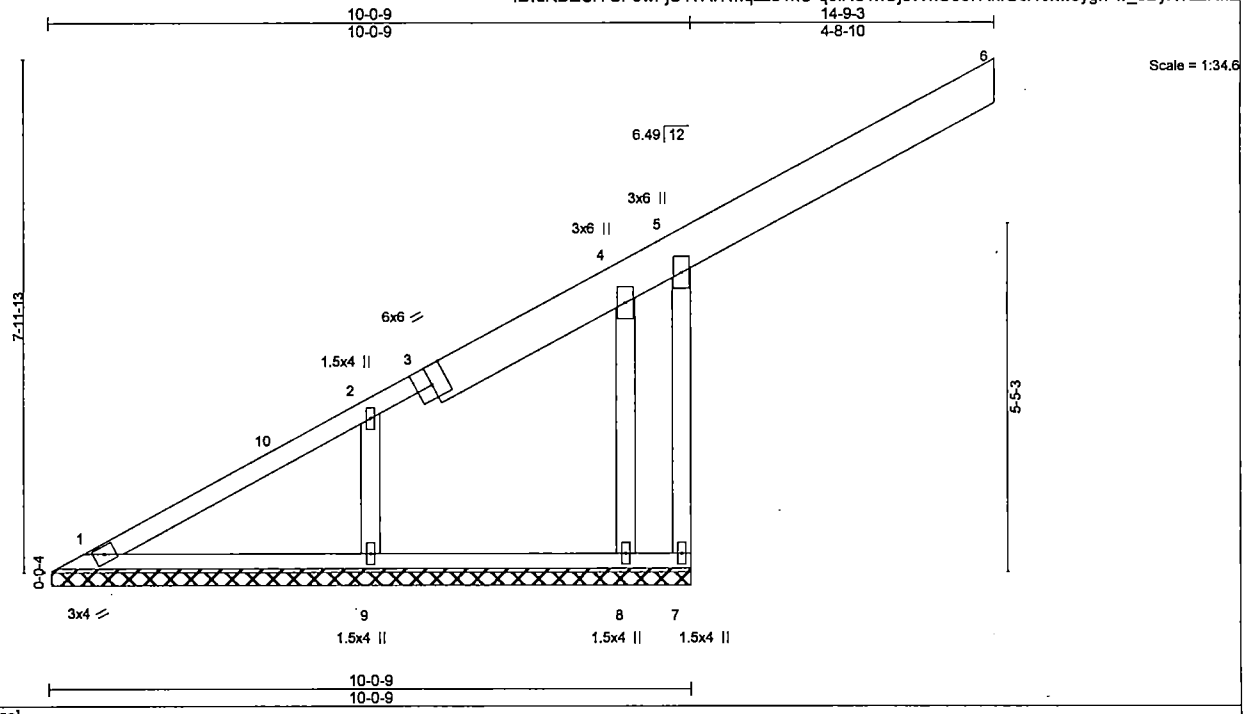


Plate Offsets (X,Y): [3:0-3-0,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.51	Vert(LL)	0.00	5	n/r	90	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.18	Vert(TL)	-0.08	5-6	n/r	90		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.16	Horz(TL)	0.00	7	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix)		Wind(LL)	0.09	5-6	n/r	120	Weight: 62 lb	FT = 10%

LUMBER
TOP CHORD 2x8 SP 2400F 2.0E *Except*
T1: 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud
OTHERS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 10-0-1.
(lb) - Max Horz 1=230(LC 5)
Max Uplift All uplift 100 lb or less at joint(s) 9 except 7=-397(LC 7), 8=-299(LC 1)
Max Grav All reactions 250 lb or less at joint(s) 8, 1 except 7=1038(LC 1), 9=399(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 4-5=-253/313, 5-7=-1047/595
WEBS 2-9=-300/117, 4-8=-289/361

NOTES
1) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-7-0 to 3-7-0, Interior(1) 3-7-0 to 10-6-4, Exterior(2) 10-6-4 to 14-9-3 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Plates checked for a plus or minus 5 degree rotation about its center.
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 7=397, 8=299.
7) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

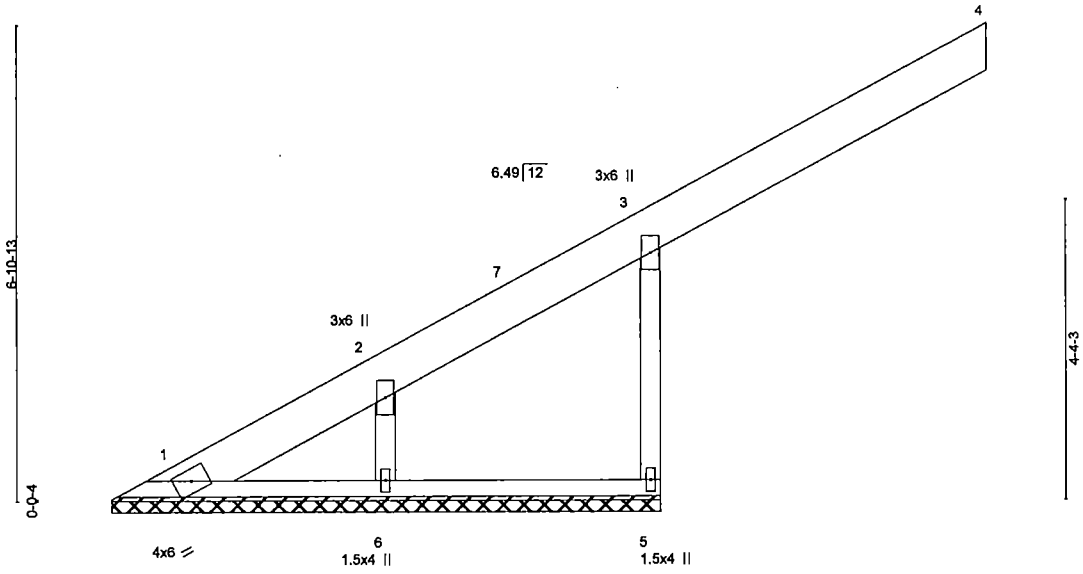
LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	AV2	Valley	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:19 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-IIGmsNn0UAeep0DJkRpV9zdHhKfqPKc3DlwVtvzzRnY

8-0-8 / 8-0-8 — 12-9-2 / 4-8-10

Scale: 3/8"=1'



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.31	Vert(LL)	0.00	3	n/r	90	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.14	Vert(TL)	-0.08	3-4	n/r	90		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.02	Horz(TL)	0.00	5	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix)		Wind(LL)	0.09	3-4	n/r	120	Weight: 57 lb	FT = 10%

LUMBER
TOP CHORD 2x8 SP 2400F 2.0E
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud
OTHERS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=179/8-0-1, 5=846/8-0-1, 6=64/8-0-1
Max Horz 1=197(LC 5)
Max Uplift5=-264(LC 7), 6=-13(LC 6)
Max Grav 1=179(LC 1), 5=846(LC 1), 6=170(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 3-5=-815/435

NOTES
1) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-10-14 to 4-0-4, Interior(1) 4-0-4 to 8-6-4, Exterior(2) 8-6-4 to 12-9-2 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Plates checked for a plus or minus 5 degree rotation about its center.
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6 except (jt=lb) 5=264.
7) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	AV3	Valley	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:20 2014 Page 1
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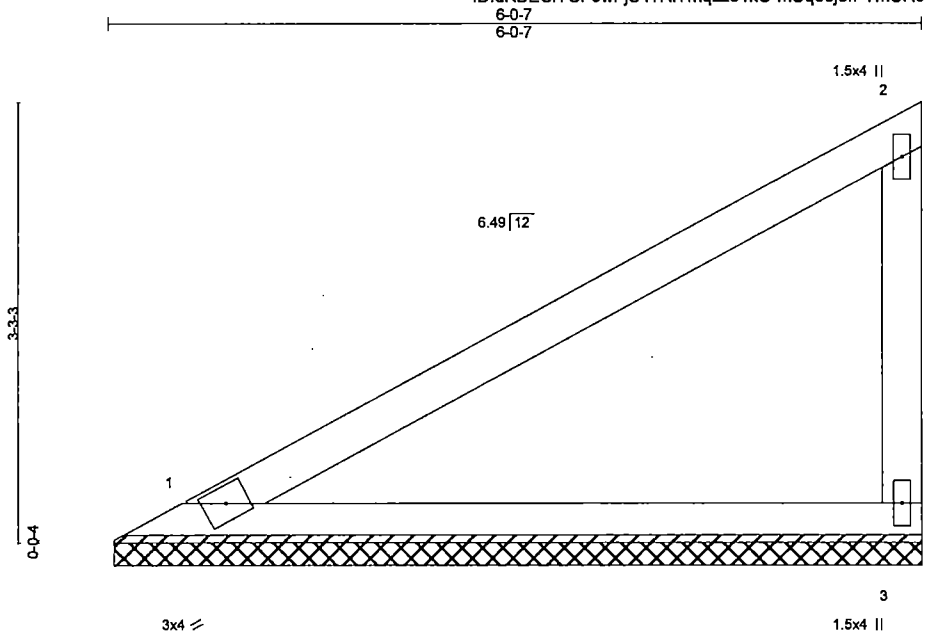

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LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.67	Vert(LL)	n/a	-	n/a	999	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.31	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix)							Weight: 17 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 6-0-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=265/6-0-0, 3=265/6-0-0
Max Horz 1=88(LC 5)
Max Uplift1=-6(LC 6), 3=-30(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES
1) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Plates checked for a plus or minus 5 degree rotation about its center.
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
7) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	AV4	Valley	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:21 2014 Page 1
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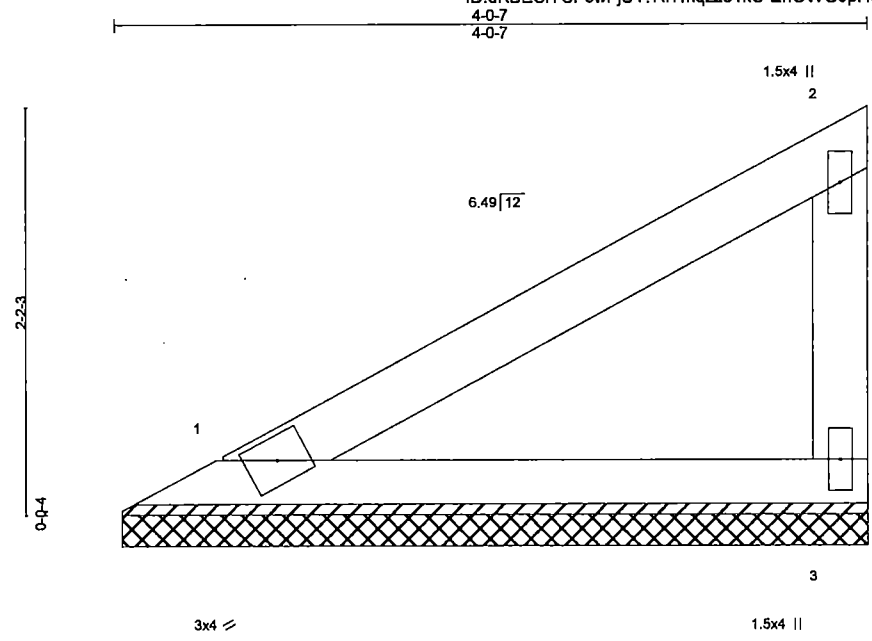

Scale: 1"=1'

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.26	Vert(LL)	n/a	-	n/a	999	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.12	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix)							Weight: 11 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 4-0-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=165/3-11-15, 3=165/3-11-15
Max Horz 1=55(LC 5)
Max Uplift1=-4(LC 6), 3=-19(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES
1) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Plates checked for a plus or minus 5 degree rotation about its center.
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
7) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	AV5	Valley	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:22 2014 Page 1
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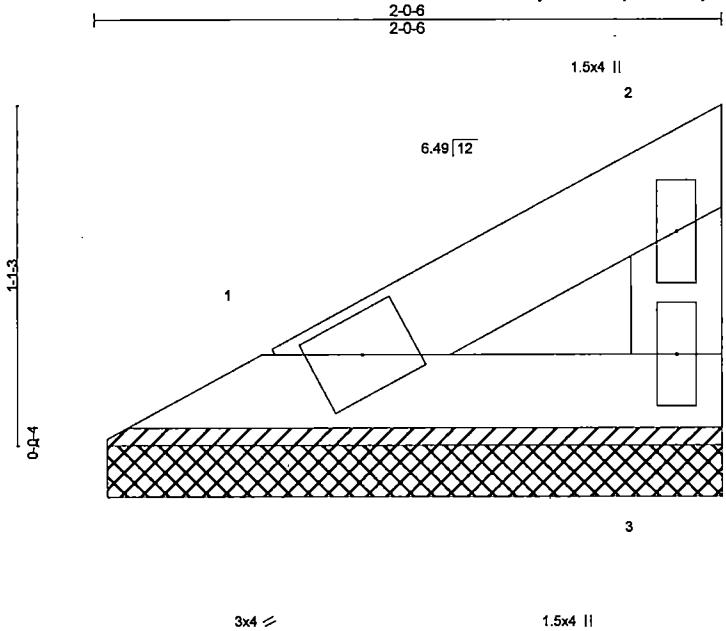


Scale = 1:7.2

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.04 65#/-2#	Vert(LL)	n/a	-	n/a	999 65#/-7#	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.02	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix)							Weight: 5 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 2-0-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=65/1-11-15, 3=65/1-11-15
Max Horz 1=22(LC 5)
Max Uplift1=-2(LC 6), 3=-7(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES
1) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Plates checked for a plus or minus 5 degree rotation about its center.
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
7) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

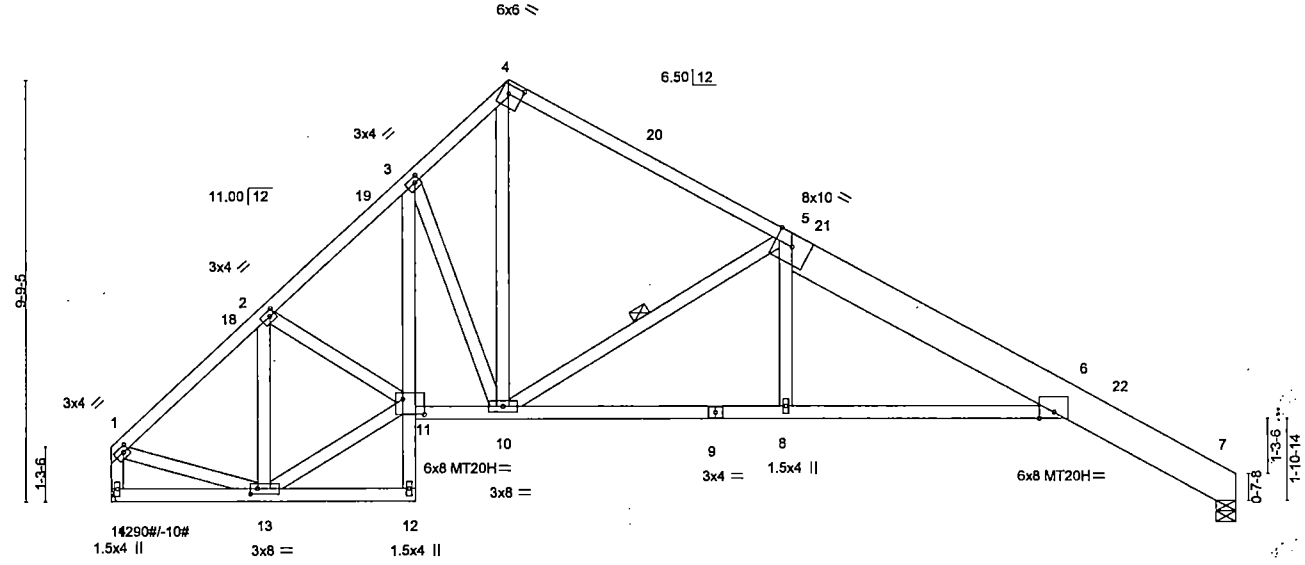
LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	B2	Roof Special	6	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:25 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-6Sd16Qsn30OnXxgT4iwwPEtC4IYApwdybENq4ZzzRnS

3-6-14	7-1-0	9-3-4	15-8-12	22-2-4	26-2-0
3-6-14	3-6-3	2-2-4	6-5-8	6-5-8	3-11-12

Scale = 1:51.5



3-6-14	7-1-0	9-3-4	15-8-12	22-2-4	26-2-0	0-5-8(0-2-4)
3-6-14	3-6-3	2-2-4	6-5-8	6-5-8	3-11-12	1299#/-30#

Plate Offsets (X,Y): [1:0-1-8,0-1-8], [2:0-1-8,0-1-8], [3:0-1-8,0-1-8], [4:0-3-13,0-2-8], [11:0-6-0,0-4-4], [13:0-2-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.77	Vert(LL)	-0.33	8-17	>952	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.78	Vert(TL)	-0.67	8-17	>461	180	MT20H	148/108
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.50	Horz(TL)	0.43	7	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.14	8-17	>999	240	Weight: 149 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2 *Except*
T3: 2x10 SP 2400F 2.0E
BOT CHORD 2x4 SPF No.2 *Except*
B2: 2x4 SPF Stud
WEBS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 4-2-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-10

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 14=1290/Mechanical, 7=1299/0-5-8
Max Horz 14=-236(LC 4)
Max Uplift14=-10(LC 6), 7=-30(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-18=-1349/50, 2-18=-1183/66, 2-19=-1797/67, 3-19=-1651/92, 3-4=-1450/131, 4-20=-1154/107, 5-20=-1302/90, 5-21=-2016/70, 6-21=-2573/68, 6-22=-453/56, 7-22=-600/40, 1-14=-1252/50
BOT CHORD 3-11=-37/527, 10-11=0/1250, 9-10=0/2124, 8-9=0/2124, 6-8=-6/2118
WEBS 2-13=-786/39, 11-13=-11/1070, 2-11=0/371, 3-10=-555/98, 4-10=-58/1160, 5-10=-1288/82, 5-8=0/285, 1-13=0/912

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-1-12 to 3-1-12 , Interior(1) 3-1-12 to 6-3-4, Exterior(2) 6-3-4 to 9-3-4, Interior(1) 12-3-4 to 22-11-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) All plates are MT20 plates unless otherwise indicated.
4) Plates checked for a plus or minus 5 degree rotation about its center.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14, 7.
10) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

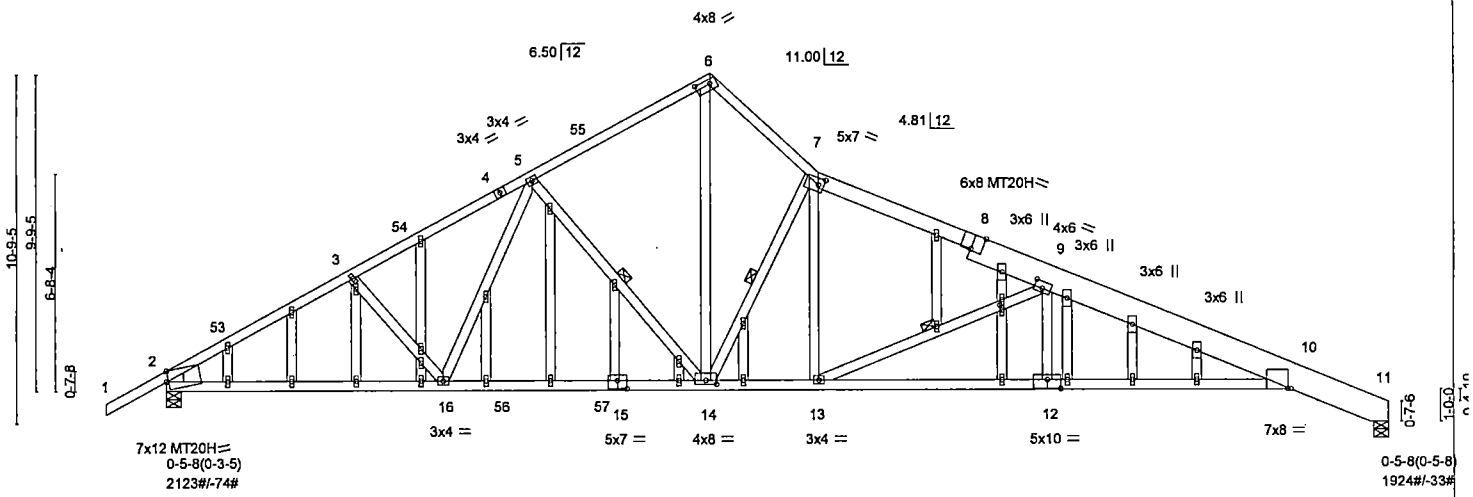
LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	C1E	GABLE	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:28 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-X1J9ISugMxmMPOP2lqTd0sVgtyXJ0DsOHCcUhuzzRnP

-1-10-8	5-9-15	11-4-6	16-10-12	20-3-3	27-3-10	34-4-1	37-11-0
1-10-8	5-9-15	5-6-7	5-6-7	3-4-7	7-0-7	7-0-7	3-6-15

Scale = 1:68.6



8-7-2	16-10-12	20-3-3	27-3-10	34-4-1	37-11-0
8-7-2	8-3-10	3-4-7	7-0-7	7-0-7	3-6-15

Plate Offsets (X,Y): [2:0-0-13,Edge], [6:0-5-8,0-1-12], [7:0-2-0,0-2-8], [8:0-4-0,Edge], [9:0-3-0,0-2-8], [10:0-1-2,Edge], [12:0-5-0,0-3-4], [14:0-4-0,0-1-8], [15:0-3-8,0-3-0], [25:0-1-13,0-0-12]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.92	Vert(LL)	-0.49	14-16	>924	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.95	Vert(TL)	-0.94	12-13	>481	180	MT20H	148/108
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.72	Horz(TL)	0.41	11	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.19	12-52	>999	240	Weight: 235 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2 *Except*
T4: 2x6 SPF No.2, T1: 2x4 SPF 2100F 1.8E, T5: 2x10 SP 2400F 2.0E
BOT CHORD 2x4 SPF 1650F 1.5E
WEBS 2x4 SPF Stud *Except*
W4: 2x4 SPF No.2
OTHERS 2x4 SPF Stud
WEDGE
Left: 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 5-14, 7-14, 9-13

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=2123/0-5-8, 11=1924/0-5-8
Max Horz 2=-190(LC 4)
Max Uplift2=-74(LC 6), 11=-33(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-53=-3334/52, 3-53=-3281/79, 3-54=-3059/74, 4-54=-2991/77, 4-5=-2888/95, 5-55=-2366/122, 6-55=-2248/150, 6-7=-2697/164, 7-8=-3184/115, 8-9=-3268/87, 9-10=-4830/102, 10-11=-705/44
BOT CHORD 2-16=0/2806, 16-56=0/2452, 56-57=0/2452, 15-57=0/2452, 14-15=0/2452, 13-14=0/2883, 12-13=-7/4589, 10-12=-47/4589
WEBS 3-16=-303/118, 5-16=0/492, 5-14=-766/83, 6-14=-95/2510, 7-14=-1950/112, 7-13=-9/827, 9-13=-1899/155, 9-12=0/321

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-10-8 to 1-11-0, Interior(1) 1-11-0 to 13-1-4, Exterior(2) 13-1-4 to 16-10-12, Interior(1) 20-3-3 to 33-10-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are MT20 plates unless otherwise indicated.
5) All plates are 1.5x4 MT20 unless otherwise indicated.
6) Plates checked for a plus or minus 5 degree rotation about its center.
7) Gable studs spaced at 2-0-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
10) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11.
12) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
13) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

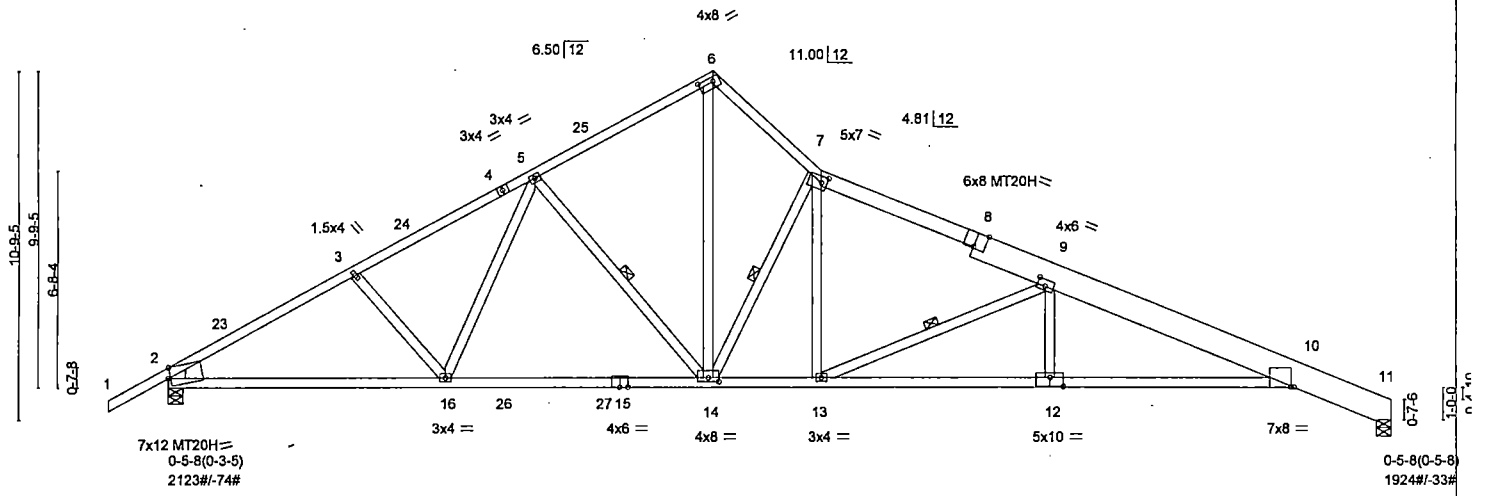
LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	C2	ROOF SPECIAL	4	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:30 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-TQRw98wwuY14eiZQtFV56Ha0NmDnU7MhlV5bmnzzRnN

-1-10-8	5-9-15	11-4-6	16-10-12	20-3-3	27-3-10	34-4-1	37-11-0
1-10-8	5-9-15	5-6-7	5-6-7	3-4-7	7-0-7	7-0-7	3-6-15

Scale = 1:68.6



8-7-2	16-10-12	20-3-3	27-3-10	34-4-1	37-11-0
8-7-2	8-3-10	3-4-7	7-0-7	7-0-7	3-6-15

Plate Offsets (X,Y): [2:0-0-13,Edge], [6:0-5-8,0-1-12], [7:0-2-0,0-2-8], [8:0-4-0,Edge], [9:0-3-0,0-2-8], [10:0-1-2,Edge], [12:0-5-0,0-3-4], [14:0-4-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.92	Vert(LL)	-0.49	14-16	>924	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.95	Vert(TL)	-0.94	12-13	>481	180	MT20H	148/108
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.72	Horz(TL)	0.41	11	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.19	12-22	>999	240	Weight: 194 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2 *Except*
T4: 2x6 SPF No.2, T1: 2x4 SPF 2100F 1.8E, T5: 2x10 SP 2400F 2.0E
BOT CHORD 2x4 SPF 1650F 1.5E
WEBS 2x4 SPF Stud *Except*
W4: 2x4 SPF No.2
WEDGE
Left: 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 5-14, 7-14, 9-13

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=2123/0-5-8, 11=1924/0-5-8
Max Horz 2=-190(LC 4)
Max Uplift2=-74(LC 6), 11=-33(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-23=-3334/52, 3-23=-3281/79, 3-24=-3059/74, 4-24=-2991/77, 4-5=-2888/95, 5-25=-2366/122, 6-25=-2248/150, 6-7=-2697/164, 7-8=-3184/115, 8-9=-3268/87, 9-10=-4830/102, 10-11=-705/44
BOT CHORD 2-16=0/2806, 16-26=0/2452, 26-27=0/2452, 15-27=0/2452, 14-15=0/2452, 13-14=0/2883, 12-13=-7/4589, 10-12=-47/4589
WEBS 3-16=-303/118, 5-16=0/492, 5-14=-766/83, 6-14=-95/2510, 7-14=-1950/112, 7-13=-9/827, 9-13=-1899/155, 9-12=0/321

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-10-8 to 1-11-0, Interior(1) 1-11-0 to 13-1-4, Exterior(2) 13-1-4 to 16-10-12, Interior(1) 20-3-3 to 33-10-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) All plates are MT20 plates unless otherwise indicated.
4) Plates checked for a plus or minus 5 degree rotation about its center.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
7) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11.
9) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

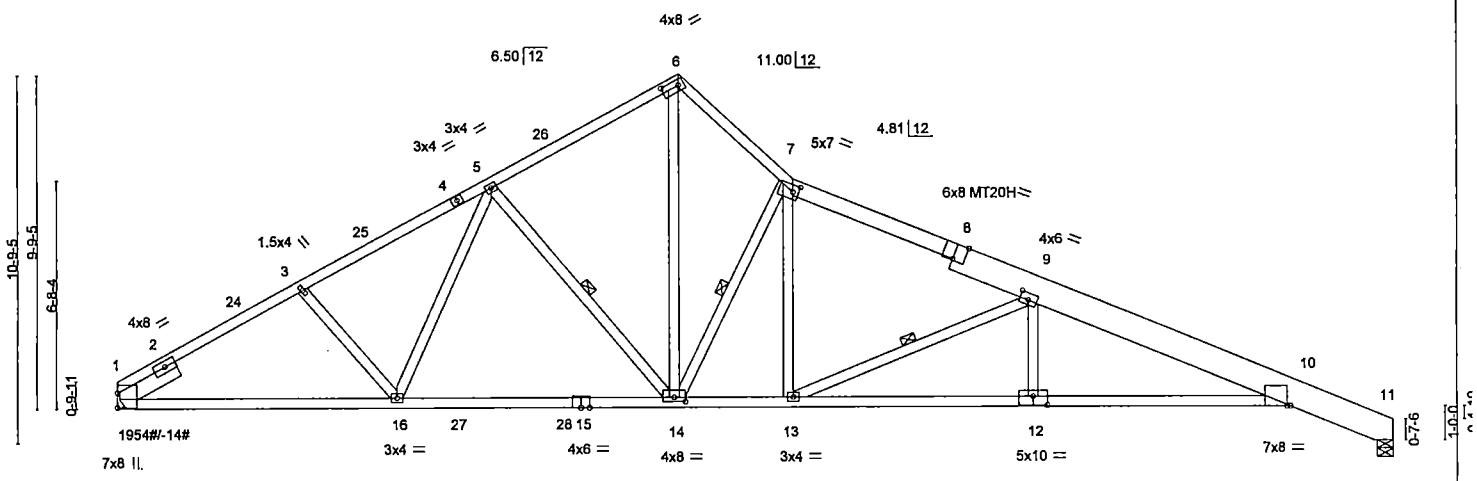
LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	C3	ROOF SPECIAL	7	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:32 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-PoYgaqxAQ9Hnt?ip_gYZBigM_avMy1v_CpahqfzzRnL

5-5-15	11-0-6	16-6-12	19-11-3	26-11-10	34-0-1	37-7-0
5-5-15	5-6-7	5-6-7	3-4-7	7-0-7	7-0-7	3-6-15

Scale = 1:65.2



0-5-8(0-5-7)

8-3-2	16-6-12	19-11-3	26-11-10	34-0-1	37-7-0
8-3-2	8-3-10	3-4-7	7-0-7	7-0-7	3-6-15

1910#/-33#

Plate Offsets (X,Y): [1:Edge,0-0-0], [6:0-6-0,0-2-0], [7:0-2-0,0-2-8], [8:0-4-0,Edge], [9:0-3-0,0-2-8], [10:0-1-2,Edge], [12:0-5-0,0-3-0], [14:0-4-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.92	Vert(LL)	-0.47	14-16	>957	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.94	Vert(TL)	-0.92	12-13	>487	180	MT20H	148/108
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.72	Horz(TL)	0.40	11	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.19	12-23	>999	240	Weight: 194 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2 *Except*
T4: 2x6 SPF No.2, T1: 2x4 SPF 2400F 2.0E, T5: 2x10 SP 2400F 2.0E
BOT CHORD 2x4 SPF 1650F 1.5E
WEBS 2x4 SPF Stud *Except*
W4: 2x4 SPF No.2
SLIDER Left 2x6 SPF No.2 2-0-0

BRACING
TOP CHORD Sheathed or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
2-2-0 oc bracing: 12-23.
WEBS 1 Row at midpt 5-14, 7-14, 9-13

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1954/Mechanical, 11=1910/0-5-8
Max Horz 1=-209(LC 4)
Max Uplift1=-14(LC 6), 11=-33(LC 7)

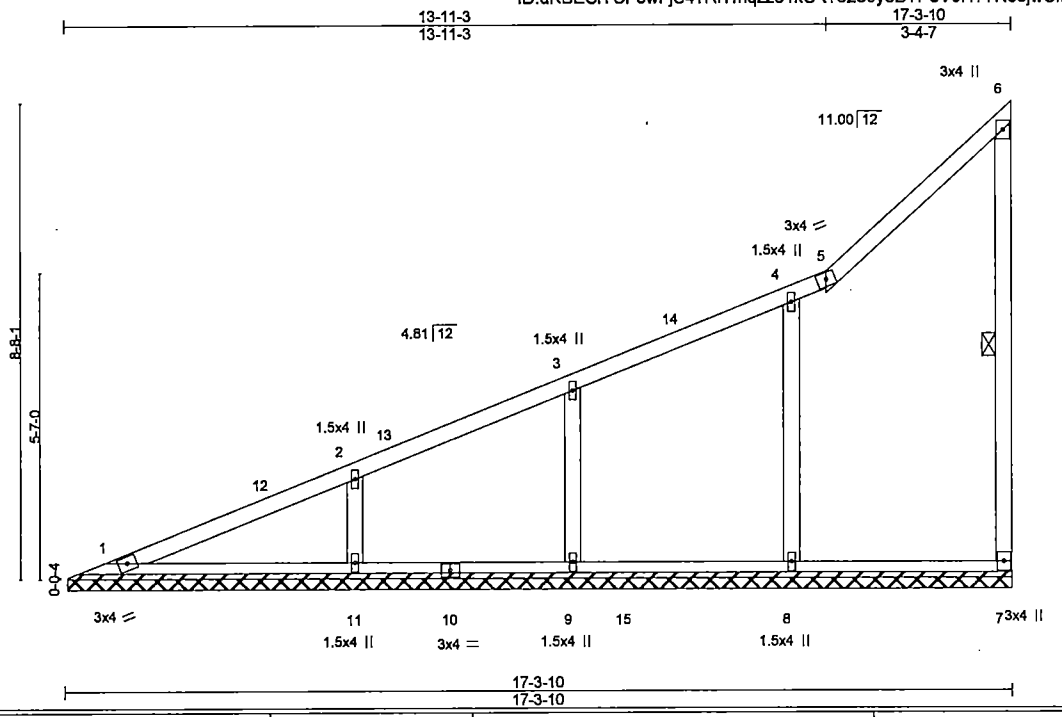
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-890/0, 2-24=-3159/89, 3-24=-3073/102, 3-25=-2954/96, 4-25=-2892/99, 4-5=-2782/116, 5-26=-2332/127, 6-26=-2213/156, 6-7=-2656/171, 7-8=-3144/123, 8-9=-3228/94, 9-10=-4785/110, 10-11=-700/45
BOT CHORD 1-16=-67/2656, 16-27=0/2398, 27-28=0/2398, 15-28=0/2398, 14-15=0/2398, 13-14=0/2846, 12-13=-14/4547, 10-12=-51/4547
WEBS 5-16=0/428, 5-14=-738/91, 6-14=-101/2469, 7-14=-1935/116, 7-13=-10/826, 9-13=-1894/155, 9-12=0/321

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-4-0 to 4-1-2, Interior(1) 4-1-2 to 13-1-11, Exterior(2) 13-1-11 to 16-10-12, Interior(1) 20-3-3 to 33-11-2 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) All plates are MT20 plates unless otherwise indicated.
4) Plates checked for a plus or minus 5 degree rotation about its center.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
7) Refer to girder(s) for truss to truss connections.
8) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11.
10) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	CV1	Valley	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:33 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-t?62o9yoBTPeV9H?YN3ojwCfMzRZhcO7RTJFM5zzRnK



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.36	Vert(LL)	n/a	-	n/a	999	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.17	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.19	Horz(TL)	0.00	7	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix)							Weight: 61 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud
OTHERS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 6-7

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 17-3-0.
(lb) - Max Horz 1=264(LC 5)
Max Uplift All uplift 100 lb or less at joint(s) 7, 8, 9, 11
Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 8=619(LC 1), 9=406(LC 1), 11=488(LC 1)

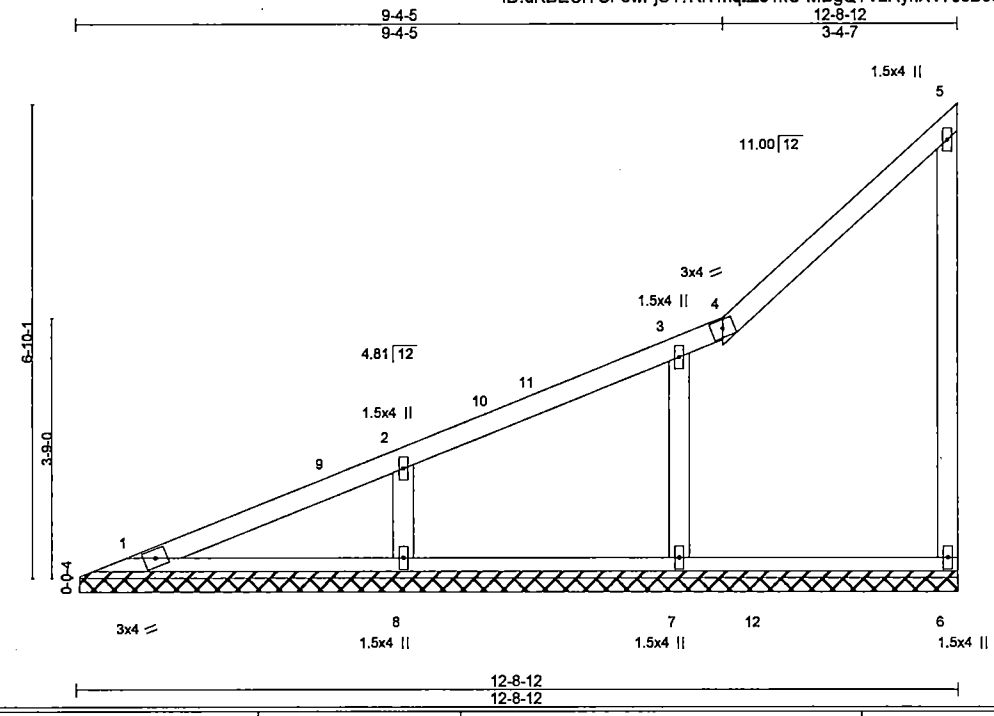
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 4-8=-350/106, 3-9=-296/96, 2-11=-384/98

NOTES
1) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-9-6 to 3-9-6, Interior(1) 3-9-6 to 13-11-3, Exterior(2) 13-11-3 to 17-1-14 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Plates checked for a plus or minus 5 degree rotation about its center.
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 8, 9, 11.
7) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	CV2	Valley	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:34 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-MBgQ?VzRynXV7JsB65a1G7lo8NmoQ58Hg73ovYzzRnJ



LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.49	Vert(LL)	n/a	-	n/a	999	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.17	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.10	Horz(TL)	0.00	6	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix)							Weight: 42 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud
OTHERS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 12-8-2.
(lb) - Max Horz 1=203(LC 5)
Max Uplift All uplift 100 lb or less at joint(s) 1, 6, 7, 8
Max Grav All reactions 250 lb or less at joint(s) 1, 6 except 7=473(LC 1), 8=437(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 3-7=-330/119, 2-8=-361/105

NOTES
1) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-9-6 to 3-9-6, Interior(1) 3-9-6 to 9-4-5, Exterior(2) 9-4-5 to 12-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Plates checked for a plus or minus 5 degree rotation about its center.
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 6, 7, 8.
7) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	CV3	Valley	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:35 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-qNEpDr_3j4fMkTROgo5GpLI0Pn7I9YjQunoMR_zzRnl

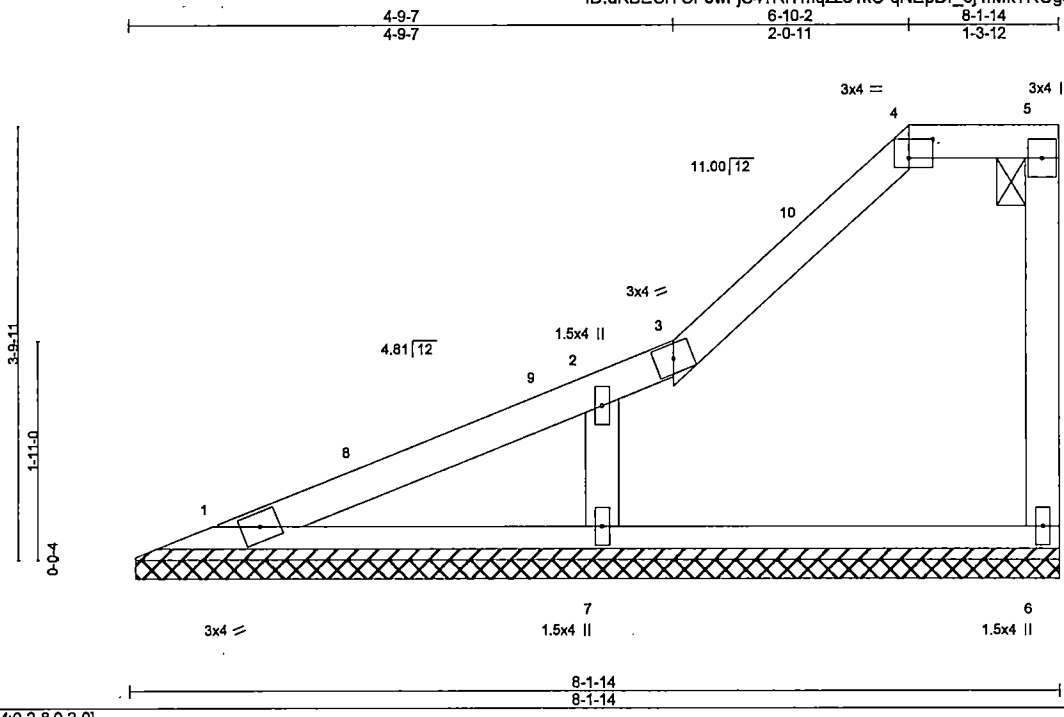


Plate Offsets (X,Y): [4:0-2-8,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.26	Vert(LL)	n/a	-	n/a	999	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.09	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.00	6	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix)							Weight: 24 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud
OTHERS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=136/8-1-4, 6=164/8-1-4, 7=422/8-1-4
Max Horz 1=111(LC 5)
Max Uplift1=-12(LC 4), 6=-26(LC 5), 7=-50(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-7=-333/116

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-9-6 to 3-9-6, Interior(1) 3-9-6 to 4-9-7, Exterior(2) 4-9-7 to 6-10-2 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) Plates checked for a plus or minus 5 degree rotation about its center.
5) Gable requires continuous bottom chord bearing.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 6, 7.
9) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	D2	COMMON	3	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:36 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-IZoBQB_hUOnDMd0aDWcVLYq5WBNOu_5Z7RYvzQzzRnH

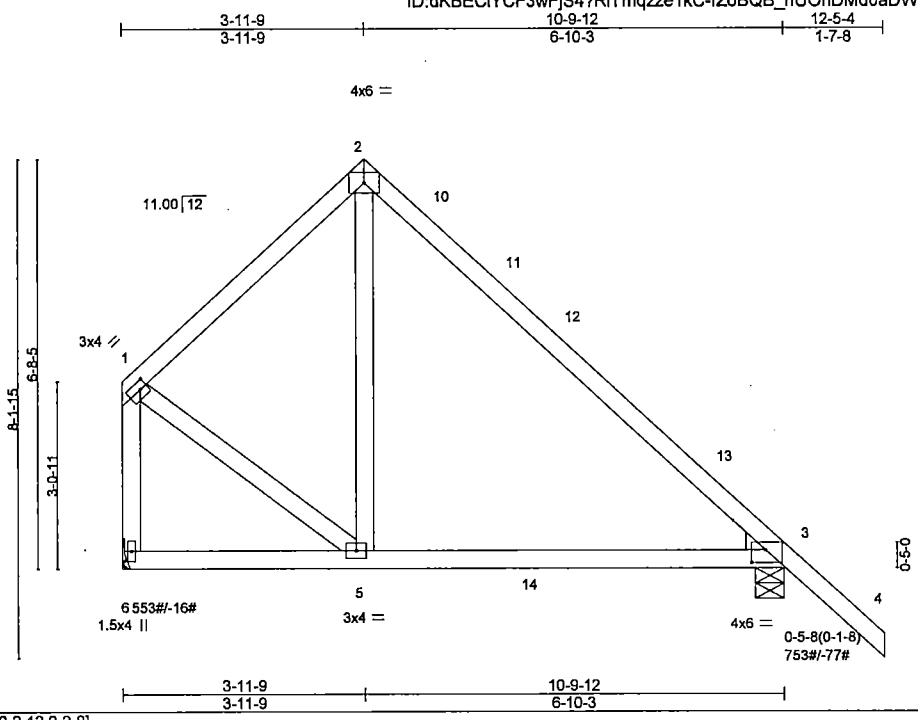


Plate Offsets (X,Y): [1:0-1-8,0-1-8], [3:0-2-12,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.69	Vert(LL)	-0.09	5-9	>999	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.48	Vert(TL)	-0.17	5-9	>739	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.13	Horz(TL)	-0.01	3	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.05	5-9	>999	240	Weight: 47 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud
WEDGE
Right: 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 5-4-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=553/Mechanical, 3=753/0-5-8
Max Horz 6=-204(LC 4)
Max Uplift 6=-16(LC 6), 3=-77(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-412/72, 10-11=-298/44, 11-12=-329/41, 12-13=-351/37, 3-13=-1036/110, 1-6=-556/39
BOT CHORD 3-14=-453/2148
WEBS 1-5=-16/303

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-1-12 to 6-11-9 , Interior(1) 6-11-9 to 9-5-4, Exterior(2) 9-5-4 to 12-5-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Plates checked for a plus or minus 5 degree rotation about its center.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 3.
8) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	D3	Common	9	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:38 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-Eyvxrt0x0?1xbwAzLxfzQzvQj_2bMuZsbl102JzzRnF

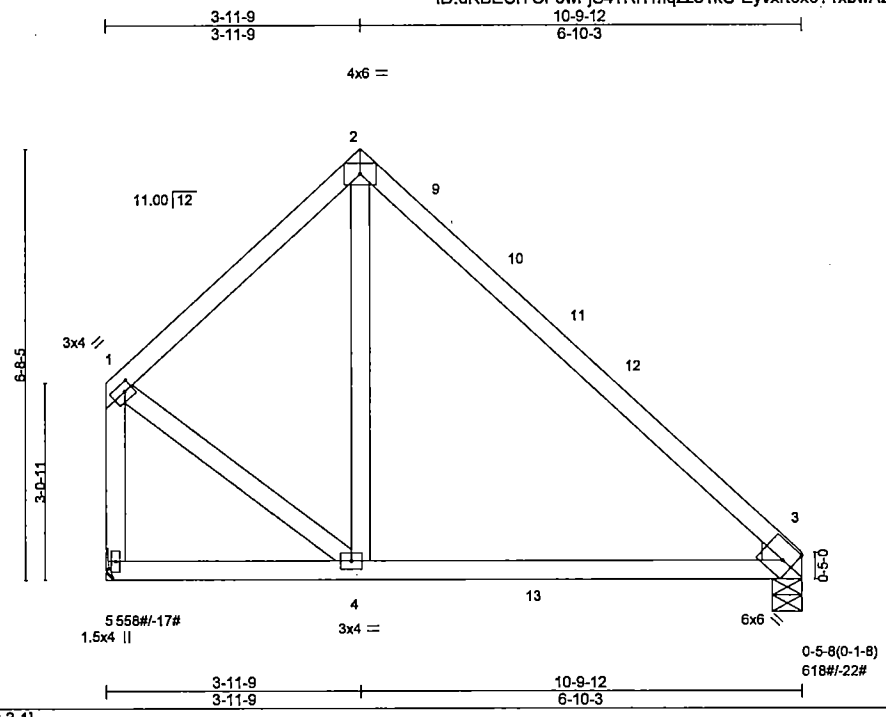


Plate Offsets (X,Y): [1:0-1-8,0-1-8], [3:Edge,0-3-1]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.71	Vert(LL)	-0.10	4-8	>999	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.57	Vert(TL)	-0.19	4-8	>672	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.13	Horz(TL)	-0.01	3	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.07	4-8	>999	240	Weight: 45 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud
WEDGE
Right: 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 4-10-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=558/Mechanical, 3=618/0-5-8
Max Horz 5=-181(LC 4)
Max Uplift5=-17(LC 6), 3=-22(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-418/80, 9-10=-303/49, 10-11=-335/45, 11-12=-355/41, 3-12=-1213/275, 1-5=-564/47
BOT CHORD 3-13=-810/2522
WEBS 1-4=-17/308

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-1-12 to 6-11-9 , Interior(1) 6-11-9 to 7-9-12, Exterior(2) 7-9-12 to 10-9-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Plates checked for a plus or minus 5 degree rotation about its center.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.
8) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	D4	Common	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:39 2014 Page 1
ID:dKBEClYCF3wFjS4?RITmqzze1kC-i8TJ2D1ZmJ9oD4l9veACzBSdCOOY5LU0pPmZalzzRnE

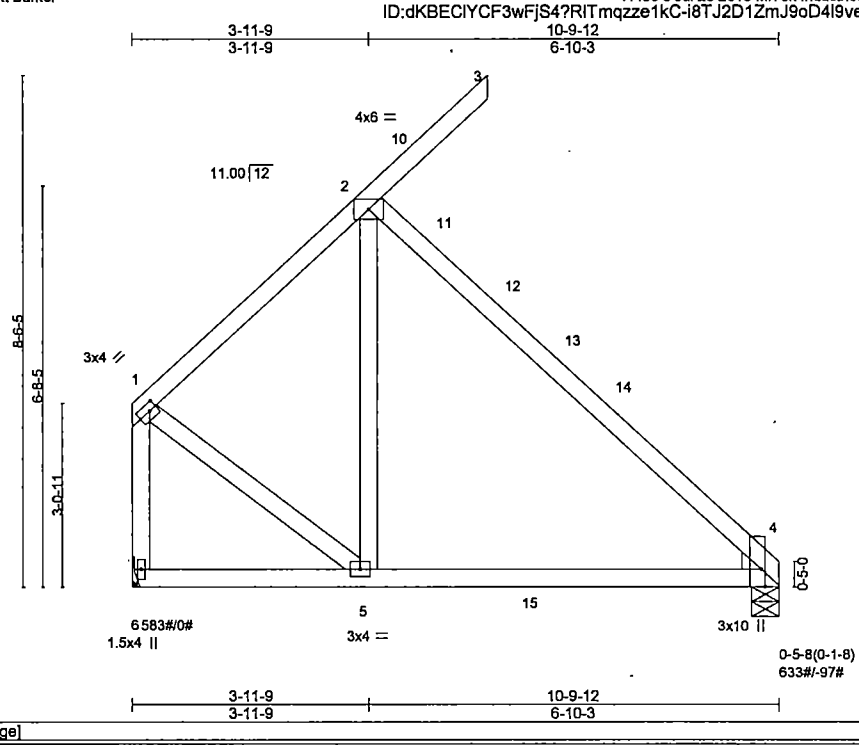


Plate Offsets (X,Y): [1:0-1-8,0-1-8], [4:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.60	Vert(LL)	-0.09	5-9	>999	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.52	Vert(TL)	-0.18	5-9	>714	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.15	Horz(TL)	-0.01	4	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.07	5-9	>999	240	Weight: 48 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud
WEDGE
Right: 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 5-1-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=583/Mechanical, 4=633/0-5-8
Max Horz 6=208(LC 5)
Max Uplift4=-97(LC 7)

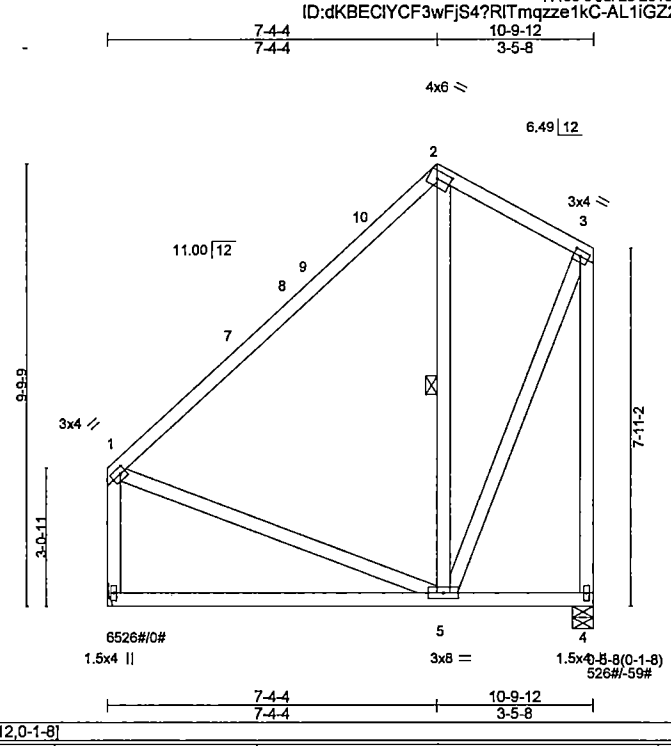
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-442/72, 2-11=-321/337, 11-12=-336/326, 12-13=-388/322, 13-14=-390/317, 4-14=-1145/379, 1-6=-586/20
BOT CHORD 5-15=-116/286, 4-15=-852/2300
WEBS 1-5=0/357

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-1-12 to 6-11-9 , Interior(1) 6-11-9 to 7-9-12, Exterior(2) 7-9-12 to 10-9-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Plates checked for a plus or minus 5 degree rotation about its center.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
8) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	D5	Roof Special	5	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:40 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-AL1iGZ2BXdHfrEJLSLhRWO?msol6qoo923W76BzzRnD



Scale = 1:49.2

Plate Offsets (X,Y): [1:0-1-8,0-1-8], [2:0-3-9,0-2-0], [3:0-1-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.73	Vert(LL)	-0.10	5-6	>999	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.43	Vert(TL)	-0.25	5-6	>502	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.15	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.00	5	>999	240	Weight: 68 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF 2400F 2.0E *Except*
T2: 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 2-5

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=526/Mechanical, 4=526/0-5-8
Max Horz 6=299(LC 5)
Max Uplift4=-59(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-7=-341/54, 1-6=-466/68, 3-4=-525/59
BOT CHORD 5-6=-272/224
WEBS 3-5=-90/347

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 4-4-4, Exterior(2) 4-4-4 to 7-4-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Plates checked for a plus or minus 5 degree rotation about its center.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
8) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	D6	Roof Special	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:42 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-7j9ShE3S3EXN4YTkamjvbp44UcQGIg5SVN?DB4zzRnB

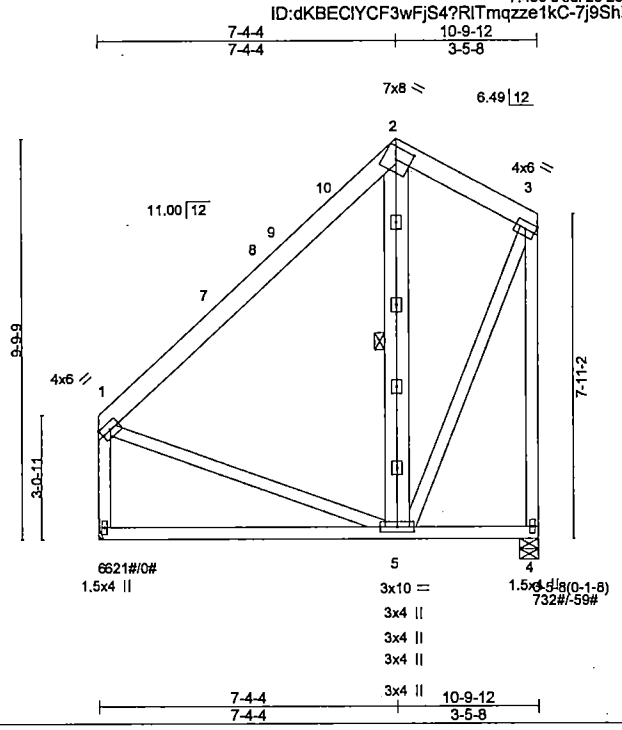


Plate Offsets (X,Y): [2:0-4-5,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.85	Vert(LL)	-0.09	5-6	>999	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.45	Vert(TL)	-0.24	5-6	>536	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.23	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.00	5	>999	240	Weight: 87 lb	FT = 10%

LUMBER
TOP CHORD 2x6 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 2-5

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=621/Mechanical, 4=732/0-5-8
Max Horz 6=296(LC 5)
Max Uplift 4=-59(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-7=-441/59, 7-8=-285/64, 8-9=-252/69, 2-3=-288/144, 1-6=-561/74, 3-4=-726/61
BOT CHORD 5-6=-270/222
WEBS 2-5=-414/106, 3-5=-91/523

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 4-4-4, Exterior(2) 4-4-4 to 7-4-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Plates checked for a plus or minus 5 degree rotation about its center.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
8) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 300 lb down and 21 lb up at 7-4-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.
11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-2=-80, 2-3=-80, 4-6=-20
Concentrated Loads (lb)
Vert: 2=-300(F)

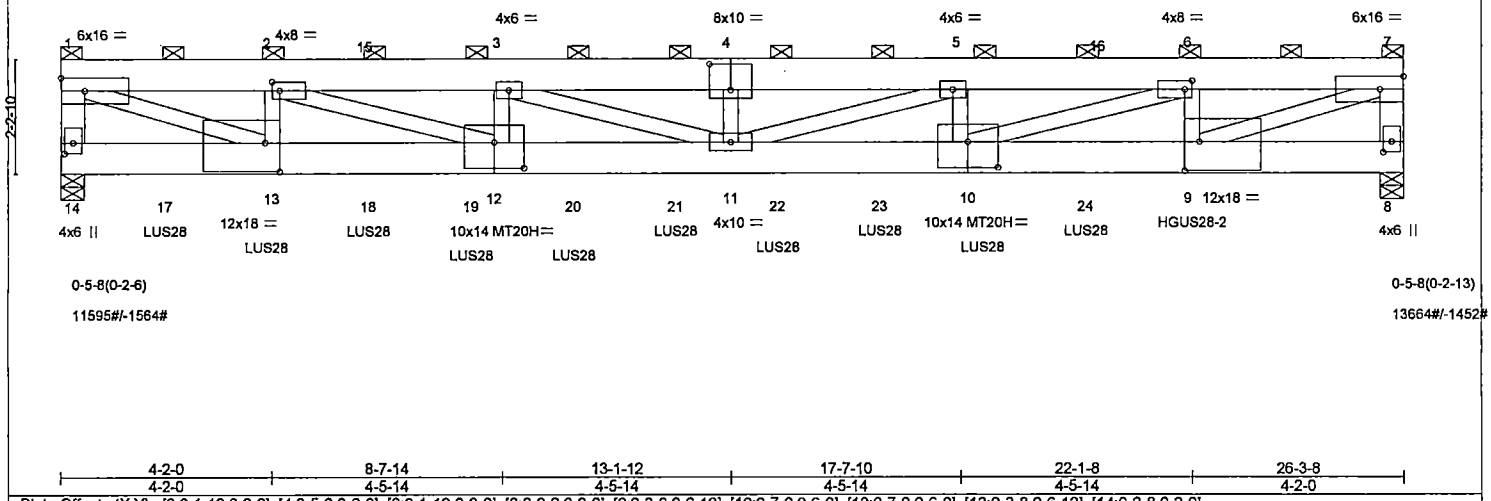
Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	EG	FLAT GIRDER	1	4	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:44 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-36HC5w5ibrn4Jrd6hBlNgE9UkP28mP0lzhUKFzzzRn9

4-2-0	8-7-14	13-1-12	17-7-10	22-1-8	26-3-8
4-2-0	4-5-14	4-5-14	4-5-14	4-5-14	4-2-0

Scale = 1:43.2

MARK "UP" ON TRUSSES IN SHOP!!



4-2-0	8-7-14	13-1-12	17-7-10	22-1-8	26-3-8
4-2-0	4-5-14	4-5-14	4-5-14	4-5-14	4-2-0

Plate Offsets (X,Y): [2:0-1-12,0-2-0], [4:0-5-0,0-6-0], [6:0-1-12,0-2-0], [8:0-2-8,0-2-0], [9:0-3-8,0-6-12], [10:0-7-0,0-6-0], [12:0-7-0,0-6-0], [13:0-3-8,0-6-12], [14:0-2-8,0-2-0]

LOADING (psf)		SPACING	8-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.61	Vert(LL)	-0.58	11	>536	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.75	Vert(TL)	-0.93	11	>332	180	MT20H	148/108
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.90	Horz(TL)	0.10	8	n/a	n/a		
BCDL	10.0	Code	IRC2006/TPI2002		(Matrix-M)	Wind(LL)	0.25	11	>999	240	Weight: 798 lb	FT = 10%

LUMBER
TOP CHORD 2x8 SP 2400F 2.0E
BOT CHORD 2x8 SP 2400F 2.0E
WEBS 2x4 SPF Stud *Except*
W1: 2x6 SPF No.2, W2: 2x4 SPF 2100F 1.8E, W4: 2x4 SPF 1650F 1.5E

BRACING
TOP CHORD 2-0-0 oc purlins (6-0-0 max.), except end verticals (Switched from sheeted: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 14=11595/0-5-8, 8=13664/0-5-8
Max Horz 14=-186(LC 3)
Max Uplift14=-1564(LC 3), 8=-1452(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-14=-10448/1474, 1-2=-24362/3338, 2-15=-40899/5487, 3-15=-40899/5487, 3-4=-48131/6290, 4-5=-48131/6290, 5-16=-44088/5485, 6-16=-44088/5485, 6-7=-29335/3292, 7-8=-12644/1458
BOT CHORD 14-17=-372/1767, 13-17=-372/1767, 13-18=-3419/24362, 18-19=-3419/24362, 12-19=-3419/24362, 12-20=-5622/41245, 20-21=-5622/41245, 11-21=-5622/41245, 11-22=-5584/44346, 22-23=-5584/44346, 10-23=-5584/44346, 10-24=-3308/29335, 9-24=-3308/29335, 8-9=-259/2101
WEBS 1-13=-3326/24417, 2-13=-7115/1088, 2-12=-2324/17640, 3-12=-3837/607, 3-11=-823/7317, 4-11=-1253/316, 5-11=-837/4021, 5-10=-2871/605, 6-10=-2370/15737, 6-9=-5987/1112, 7-9=-3295/29431

NOTES
1) Special connection required to distribute web loads equally between all plies.
2) 4-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x8 - 2 rows staggered at 0-9-0 oc.
Bottom chords connected as follows: 2x8 - 4 rows staggered at 0-4-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc, Except member 6-9 2x4 - 2 rows staggered at 0-4-0 oc.
Attach TC w/ 1/2" diam. bolts (ASTM A-307) in the center of the member w/washers at 4-0-0 oc.
Attach BC w/ 1/2" diam. bolts (ASTM A-307) in the center of the member w/washers at 4-0-0 oc.
3) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
4) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
5) Provide adequate drainage to prevent water ponding.
6) All plates are MT20 plates unless otherwise indicated.
7) Plates checked for a plus or minus 5 degree rotation about its center.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 14=1564, 8=1452.
11) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
12) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
13) Use Simpson Strong-Tie LUS28 (6-10d Girder, 4-10d Truss) or equivalent spaced at 2-0-0 oc max. starting at 2-0-12 from the left end to 20-0-12 to connect truss(es) A2 (1 ply 2x4 SPF), A3 (1 ply 2x4 SPF) to back face of bottom chord.
14) Use Simpson Strong-Tie HGUS28-2 (36-10d Girder, 12-10d Truss) or equivalent at 22-1-8 from the left end to connect truss(es) A4G (2 ply 2x6 SPF) to back face of bottom chord.
15) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced) + Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-7=-320, 8-14=-80
Concentrated Loads (lb)
Vert: 13=-968(B) 10=-968(B) 9=-5238(B) 17=-968(B) 18=-968(B) 19=-968(B) 20=-968(B) 21=-968(B) 22=-968(B) 23=-968(B) 24=-974(B)

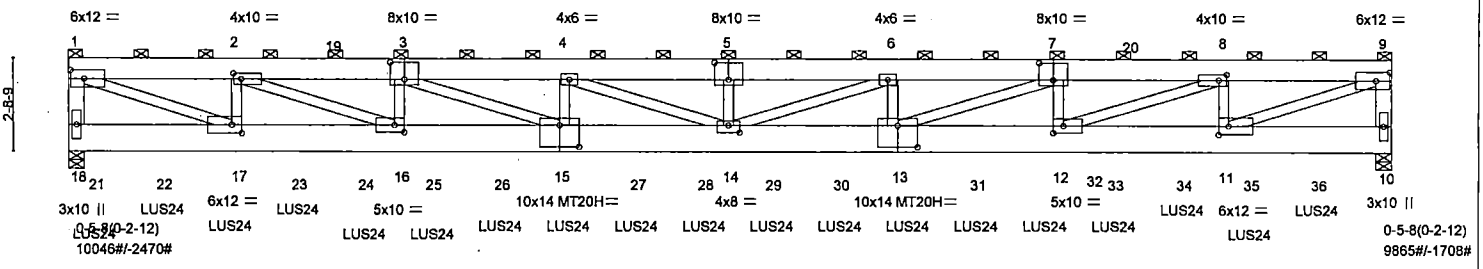
Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	FG	FLAT GIRDER	1	3	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:48 2014 Page 1
ID:dKBEClYCF3wFjS4?RITmqzze1kC-xtWjxl8Df4lWoTxuw1qJq4KAc0PkiFKLuJSYOkzzRn5

4-11-13	9-9-14	14-7-15	19-6-0	24-4-1	29-2-2	34-0-3	39-0-0
4-11-13	4-10-1	4-10-1	4-10-1	4-10-1	4-10-1	4-10-1	4-11-13

Scale = 1:65.0

MARK "UP" ON TRUSSES IN SHOP!!



4-11-13	9-9-14	14-7-15	19-6-0	24-4-1	29-2-2	34-0-3	39-0-0
4-11-13	4-10-1	4-10-1	4-10-1	4-10-1	4-10-1	4-10-1	4-11-13

Plate Offsets (X,Y): [1:0-4-12,0-3-0], [2:0-2-12,0-2-0], [3:0-5-0,0-6-0], [5:0-5-0,0-6-0], [7:0-5-0,0-6-0], [8:0-2-12,0-2-0], [9:0-4-12,0-3-0], [11:0-3-8,0-3-0], [12:0-3-8,0-2-8], [13:0-7-0,0-7-8], [14:0-4-0,0-2-8], [15:0-7-0,0-7-8], [16:0-3-8,0-2-8], [17:0-3-8,0-3-0]

LOADING (psf)		SPACING	8-2-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.62	Vert(LL)	-1.22	14	>378	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.71	Vert(TL)	-1.66	14	>279	180	MT20H	148/108
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.82	Horz(TL)	0.15	10	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.51	14	>911	240	Weight: 995 lb	FT = 10%

LUMBER
TOP CHORD 2x8 SP 2400F 2.0E
BOT CHORD 2x10 SP 2400F 2.0E
WEBS 2x4 SPF Stud *Except*
W1: 2x6 SPF No.2, W2: 2x4 SPF 2100F 1.8E, W4: 2x4 SPF 1650F 1.5E

BRACING
TOP CHORD 2-0-0 oc purlins (5-6-11 max.), except end verticals (Switched from sheeted: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 18=10046/0-5-8, 10=9865/0-5-8
Max Horz 18=239(LC 4)
Max Uplift18=-2470(LC 3), 10=-1708(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-18=-9092/2177, 1-2=-20034/4838, 2-19=-33963/8025, 3-19=-33963/8025, 3-4=-42692/9814, 4-5=-45323/10006, 5-6=-45323/10006, 6-7=-42571/8820, 7-20=-33720/6552, 8-20=-33720/6552, 8-9=-19875/3698, 9-10=-9024/1689
BOT CHORD 18-21=-436/1111, 21-22=-436/1111, 17-22=-436/1111, 17-23=-4942/20034, 23-24=-4942/20034, 16-24=-4942/20034, 16-25=-8209/34338, 25-26=-8209/34338, 15-26=-8209/34338, 15-27=-9951/42881, 27-28=-9951/42881, 14-28=-9951/42881, 14-29=-8952/42763, 29-30=-8952/42763, 13-30=-8952/42763, 13-31=-6690/34100, 31-32=-6690/34100, 12-32=-6690/34100, 12-33=-3712/19875, 33-34=-3712/19875, 11-34=-3712/19875, 11-35=-248/1104, 35-36=-248/1104, 10-36=-248/1104
WEBS 1-17=-4931/20569, 2-17=-7282/1686, 2-16=-3485/15101, 3-16=-5224/1127, 3-15=-1903/9146, 4-15=-2640/458, 4-14=-198/2647, 5-14=-1434/330, 6-14=-1255/2775, 6-13=-2684/872, 7-13=-2408/9274, 7-12=-5285/1339, 8-12=-3125/15010, 8-11=-7247/1578, 9-11=-3765/20403

NOTES
1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x8 - 2 rows staggered at 0-9-0 oc.
Bottom chords connected as follows: 2x10 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) Plates checked for a plus or minus 5 degree rotation about its center.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 18=2470, 10=1708.
10) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
12) Use Simpson Strong-Tie LUS24 (4-10d Girder, 2-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 0-9-4 from the left end to 36-9-4 to connect truss(es) D2 (1 ply 2x4 SPF), D3 (1 ply 2x4 SPF), D4 (1 ply 2x4 SPF), D5 (1 ply 2x4 SPF), D6 (1 ply 2x4 SPF) to front face of bottom chord.
13) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced) + Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-9=-327, 10-18=-82
Concentrated Loads (lb)
Vert: 15=-224(F) 17=-219(F) 13=-249(F) 21=-219(F) 22=-219(F) 23=-224(F) 24=-224(F) 25=-224(F) 26=-224(F) 27=-224(F) 28=-224(F) 29=-224(F) 30=-224(F) 31=-193(F) 32=-193(F) 33=-193(F) 34=-193(F) 35=-193(F) 36=-287(F)

MiTek USA, Inc.
14515 North Outer Forty Drive
Suite 300
Chesterfield, MO 63017-5746
314-434-1200

Re: r-5902-13
Stratton Residence

The truss drawing(s) referenced below have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Accurate Housing Systems, Inc..

Pages or sheets covered by this seal: I21692323 thru I21692323

My license renewal date for the state of Wisconsin is July 31, 2014.

Wisconsin COA: 726-011

Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.

January 14,2014

Fox, Steve

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI 1.

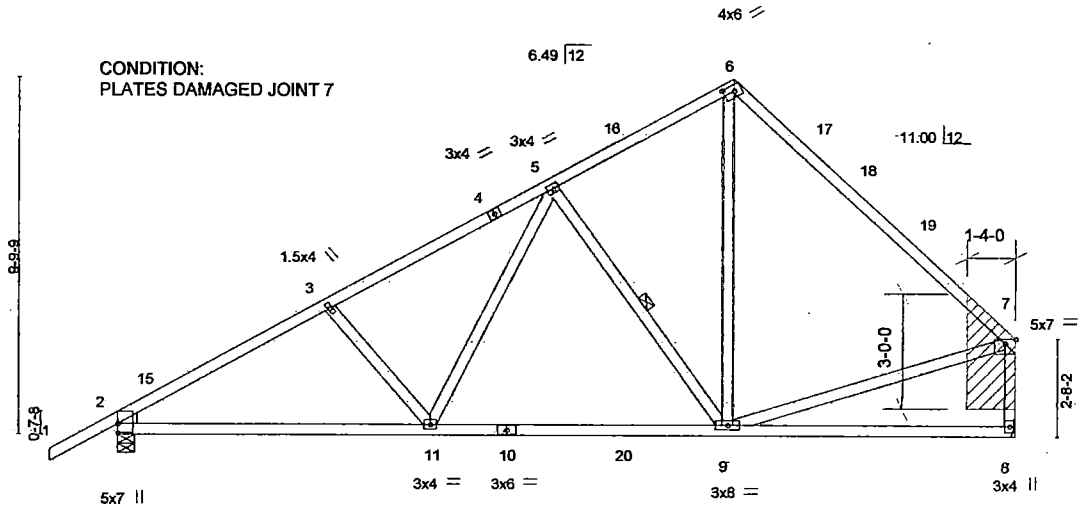
Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	A2	ROOF SPECIAL	9	1	UNITS: 1.O ENG: AJ I21692323
					Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI

7.430 s Jul 25 2013 MiTek Industries, Inc. Tue Jan 14 13:40:06 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-1jucTndoUWMv4aM0RxHXjqQBKm5?TULry9PE42zvQTd

-1-10-8	5-10-3	11-11-12	16-11-8	24-8-12
1-10-8	5-10-3	6-1-9	4-11-13	7-9-4

Scale = 1:61.6



ATTACH 1/2" PLYWOOD OR OSB GUSSET (15/32" APA RATED SHEATHING 32/16 EXP 1)
TO EACH FACE OF TRUSS WITH 10d (3" X .131") NAILS DRIVEN THROUGH BOTH SHEETS
OF PLYWOOD AND CLINCHED PER THE FOLLOWING NAIL SCHEDULE:
2 x 4's - 2 ROWS: SPACED @ 0-4-0 O.C.
NAILS TO BE DRIVEN FROM BOTH FACES. STAGGER SPACING FROM FRONT TO BACK FACE FOR A
NET 0-2-0 O.C. SPACING IN THE MAIN MEMBER. USE A MIN. 0-3-0 MEMBER END DISTANCE.

8-7-8	16-11-8	24-8-12
8-7-8	8-4-0	7-9-4

Plate Offsets (X,Y): [6:0-3-9,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.84	Vert(LL)	-0.27	9-11	>999	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.80	Vert(TL)	-0.41	9-11	>715	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.33	Horz(TL)	0.05	8	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.04	9-11	>999	240	Weight: 110 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud *Except*
7-8: 2x4 SPF No.2
WEDGE
Left: 2x4 SPF Stud

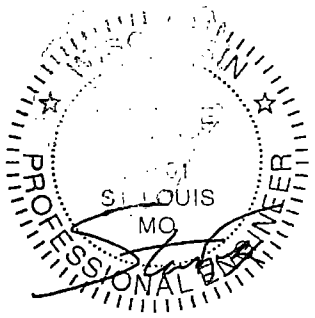
BRACING
TOP CHORD Sheathed or 2-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-9

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1449/0-5-8 (min. 0-2-4), 8=1288/Mechanical
Max Horz 2=267(LC 5)
Max Uplift 2=-75(LC 6), 8=-7(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2057/66, 3-5=-1782/83, 5-6=-987/134, 6-7=-1259/98, 7-8=-1231/70
BOT CHORD 2-11=-148/1711, 9-11=-38/1214
WEBS 3-11=-379/108, 5-11=0/574, 5-9=-777/73, 6-9=0/690, 7-9=-9/698

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-10-8 to 1-1-8, Interior(1) 1-1-8 to 13-11-8, Exterior(2) 13-11-8 to 16-11-8, Interior(1) 19-11-8 to 21-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Plates checked for a plus or minus 5 degree rotation about its center.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.



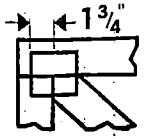
January 14,2014

LOAD CASE(S) Standard

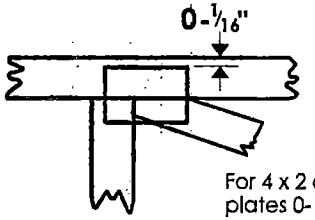
MiTek
14515 N. Outer Forty, Suite #300
Chesterfield, MO 63017

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

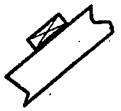
*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

4 x 4

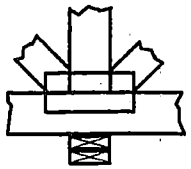
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

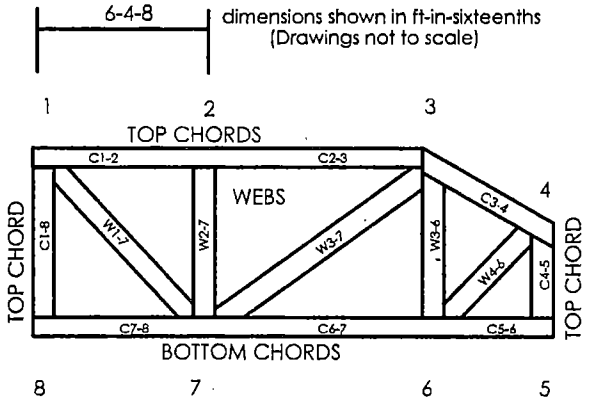


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Southern Pine lumber designations are as follows:

SYP represents values as published by AWC in the 2005/2012 NDS
SP represents ALSC approved/new values with effective date of June 1, 2013

MiTek Engineering Reference Sheet: Mll-7473 rev. 02/26/2013

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

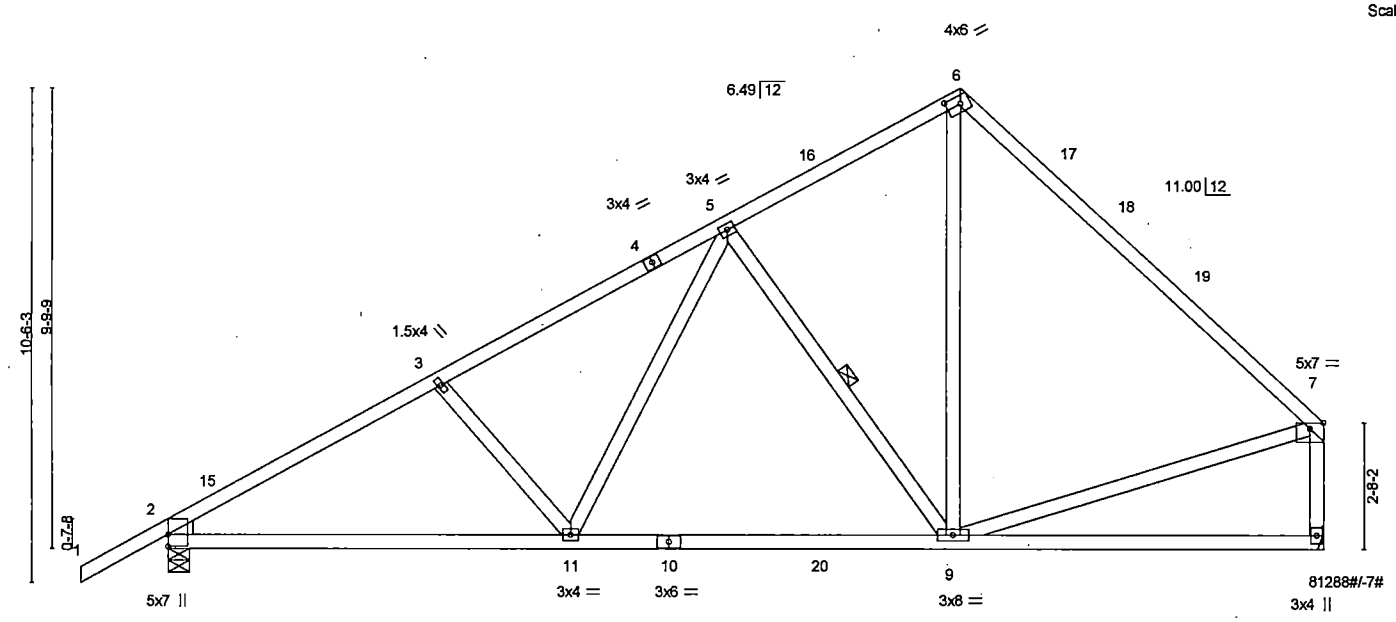
1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	A2	ROOF SPECIAL	9	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:13 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-T8vUbKjGuKtU55l9NBi5viN7uvS6?XKBrMTBfGzzRne

-1-10-8 | 5-10-3 | 11-11-12 | 16-11-8 | 24-8-12
1-10-8 | 5-10-3 | 6-1-9 | 4-11-13 | 7-9-4

Scale = 1:49.2



0-5-8(0-2-4) | 8-7-8 | 16-11-8 | 24-8-12
8-7-8 | 8-4-0 | 7-9-4

Plate Offsets (X,Y): [6:0-3-9,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.84	Vert(LL)	-0.27	9-11	>999	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.80	Vert(TL)	-0.41	9-11	>715	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.33	Horz(TL)	0.05	8	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.04	9-11	>999	240	Weight: 110 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud *Except*
W6: 2x4 SPF No.2
WEDGE
Left: 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 2-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-9

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1449/0-5-8, 8=1288/Mechanical
Max Horz 2=267(LC 5)
Max Uplift2=-75(LC 6), 8=-7(LC 7)

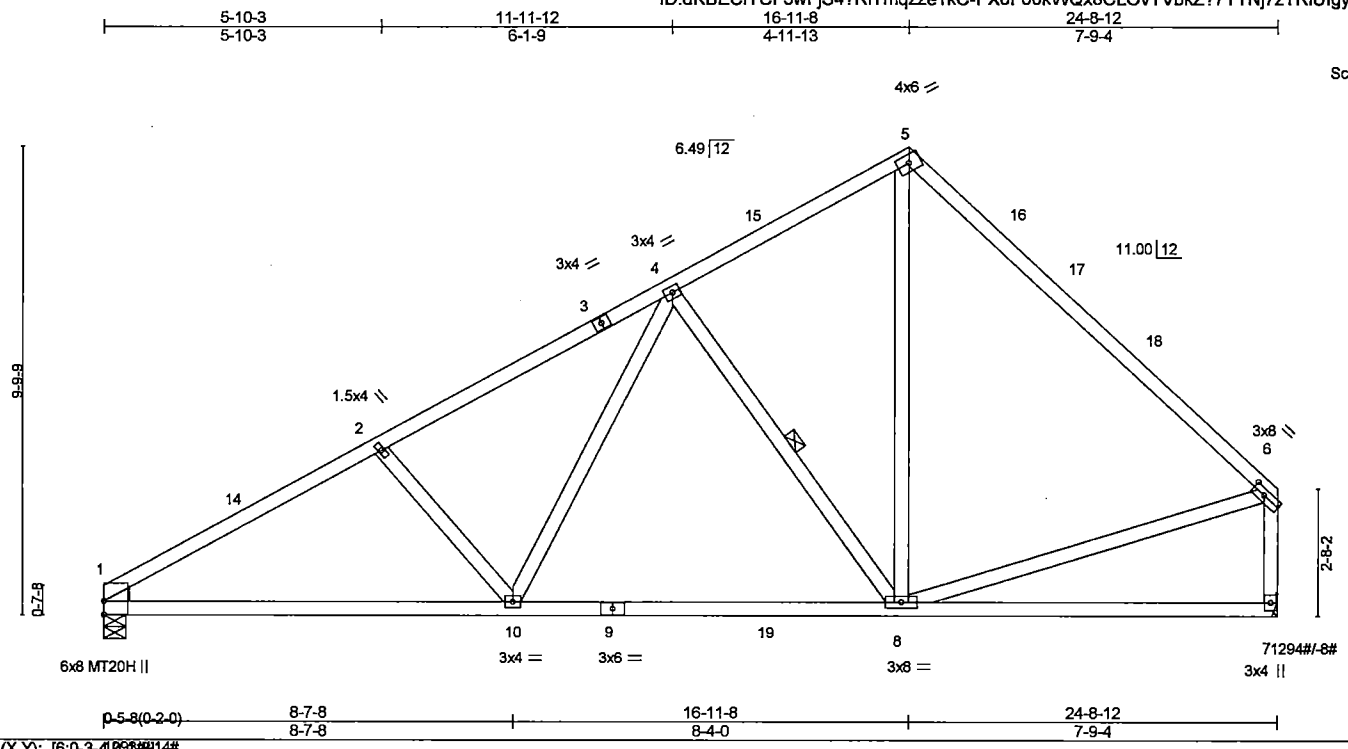
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-15=-2057/40, 3-15=-2035/66, 3-4=-1782/65, 4-5=-1606/83, 5-16=-987/108, 6-16=-816/134, 6-17=-942/98, 17-18=-1009/74, 18-19=-1051/68, 7-19=-1259/61, 7-8=-1231/70
BOT CHORD 2-11=-148/1711, 10-11=-38/1214, 10-20=-38/1214, 9-20=-38/1214
WEBS 3-11=-379/108, 5-11=0/574, 5-9=-777/73, 6-9=0/690, 7-9=-9/698

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-10-8 to 1-1-8, Interior(1) 1-1-8 to 13-11-8, Exterior(2) 13-11-8 to 16-11-8, Interior(1) 19-11-8 to 21-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Plates checked for a plus or minus 5 degree rotation about its center.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.
8) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	A3	Roof Special	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:15 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-PX0F00kWQx8CLOvYVbkZ?7TTNj7zTRIUlgylk8zzRnc



	8-7-8	16-11-8	24-8-12
p-5-8(0-2-0)	8-7-8	8-4-0	7-9-4

Plate Offsets (X,Y): [6:0-3-4,0-3-8], [8:0-3-14,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.85	Vert(LL)	-0.26	8-10	>999	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.77	Vert(TL)	-0.39	8-10	>747	180	MT20H	148/108
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.33	Horz(TL)	0.05	7	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.03	8-10	>999	240	Weight: 108 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud *Except*
W6: 2x4 SPF No.2
WEDGE
Left: 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 2-2-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-8

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1293/0-5-8, 7=1294/Mechanical
Max Horz 1=251(LC 5)
Max Uplift1=-14(LC 6), 7=-8(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-14=-2090/61, 2-14=-1911/87, 2-3=-1811/85, 3-4=-1597/102, 4-15=-991/111, 5-15=-821/136, 5-16=-948/102, 16-17=-1015/78, 17-18=-1057/71, 6-18=-1265/64, 6-7=-1236/74
BOT CHORD 1-10=-164/1746, 9-10=-39/1225, 9-19=-39/1225, 8-19=-39/1225
WEBS 2-10=-398/117, 4-10=0/602, 4-8=-788/79, 5-8=0/694, 6-8=-10/702

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 13-11-8, Exterior(2) 13-11-8 to 16-11-8, Interior(1) 19-11-8 to 21-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) All plates are MT20 plates unless otherwise indicated.
4) Plates checked for a plus or minus 5 degree rotation about its center.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7.
9) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	A4G	Roof Special Girder	1	2	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:17 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-Lw8?RhmmyYOwai3wc0m14YYoyXoSxCLnm_RPo1zzRna

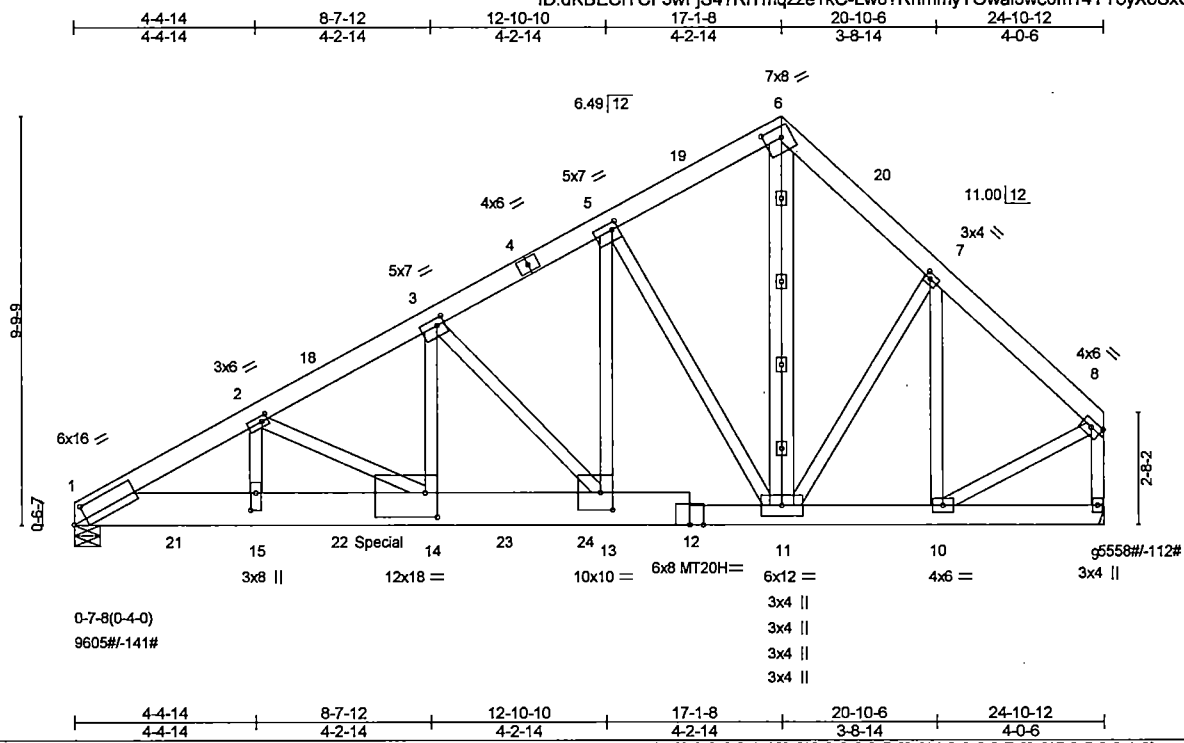


Plate Offsets (X,Y): [1:0-3-15,0-3-12], [2:0-1-12,0-1-8], [3:0-2-4,0-2-0], [5:0-1-12,0-2-0], [6:0-5-0,0-3-0], [7:0-1-8,0-1-8], [8:0-3-0,0-1-12], [13:0-3-8,0-5-0], [14:0-3-8,0-7-0], [15:0-5-0,0-1-8]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.84	Vert(LL)	-0.20	14	>999	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.84	Vert(TL)	-0.39	14	>757	180	MT20H	148/108
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.90	Horz(TL)	0.09	9	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.08	14	>999	240	Weight: 411 lb	FT = 10%

BRACING
TOP CHORD Sheathed or 2-5-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x6 SPF No.2
BOT CHORD 2x6 SPF No.2 *Except*
B1: 2x10 SP 2400F 2.0E
WEBS 2x4 SPF Stud *Except*
W3,W5: 2x4 SPF No.2, W4: 2x4 SPF 1650F 1.5E, W6: 2x4 SPF 2400F 2.0E

REACTIONS (lb/size) 1=9605/0-7-8, 9=5558/Mechanical
Max Horz 1=250(LC 4)
Max Uplift1=-141(LC 5), 9=-112(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-17661/261, 2-18=-13779/205, 3-18=-13734/220, 3-4=-8620/184, 4-5=-8546/196, 5-19=-4651/179, 6-19=-4567/187, 6-20=-5450/209, 7-20=-5625/190, 7-8=-4897/131, 8-9=-5440/127
BOT CHORD 1-21=-237/15475, 15-21=-237/15475, 15-22=-237/15475, 14-22=-237/15475, 14-23=-173/12089, 23-24=-173/12089, 13-24=-173/12089, 12-13=-100/7528, 11-12=-102/7536, 10-11=-5/3477
WEBS 2-15=-7/3379, 2-14=-3847/112, 3-14=-70/6987, 3-13=-6689/127, 5-13=-70/7351, 5-11=-6804/123, 6-11=-152/5054, 7-11=-84/1117, 7-10=-1833/54, 8-10=-21/3939

NOTES
1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x10 - 2 rows staggered at 0-5-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-3-0 oc, Except member 14-2 2x4 - 1 row at 0-9-0 oc, member 3-14 2x4 - 1 row at 0-9-0 oc, member 13-3 2x4 - 1 row at 0-9-0 oc, member 5-13 2x4 - 1 row at 0-9-0 oc, member 11-5 2x4 - 1 row at 0-9-0 oc, member 11-6 2x4 - 1 row at 0-9-0 oc, member 6-11 2x4 - 1 row at 0-9-0 oc, member 11-7 2x4 - 1 row at 0-9-0 oc, member 7-10 2x4 - 1 row at 0-9-0 oc, member 10-8 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
5) All plates are MT20 plates unless otherwise indicated.
6) Plates checked for a plus or minus 5 degree rotation about its center.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) Refer to girder(s) for truss to truss connections.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=141, 9=112.
11) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
12) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 300 lb down and 21 lb up at 17-1-8 on top chord, and 1934 lb down and 26 lb up at 2-4-12, 1934 lb down and 26 lb up at 4-4-12, 1934 lb down and 26 lb up at 6-4-12, 1934 lb down and 26 lb up at 8-4-12, and 1934 lb down and 26 lb up at 10-4-12, and 1934 lb down and 26 lb up at 12-4-2 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
cial hanger(s) or other connection device(s) shall be provided at 7-4-7 from the left end sufficient to connect truss(es) to back face of bottom chord. The gn/selection of such special connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced) + Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-19=-80, 6-19=-130(F=-50), 6-8=-130(F=-50), 1-12=-20, 9-12=-50(F=-30)
Concentrated Loads (lb)
Vert: 6=-300(F) 15=-1934(B) 14=-1934(B) 21=-1934(B) 22=-1934(B) 23=-1934(B) 24=-1934(B)

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	AV1	Valley	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:18 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-q6iNe1nOjsWnCse7AkIGcl43xwJygrFw_eByKTzzRnZ

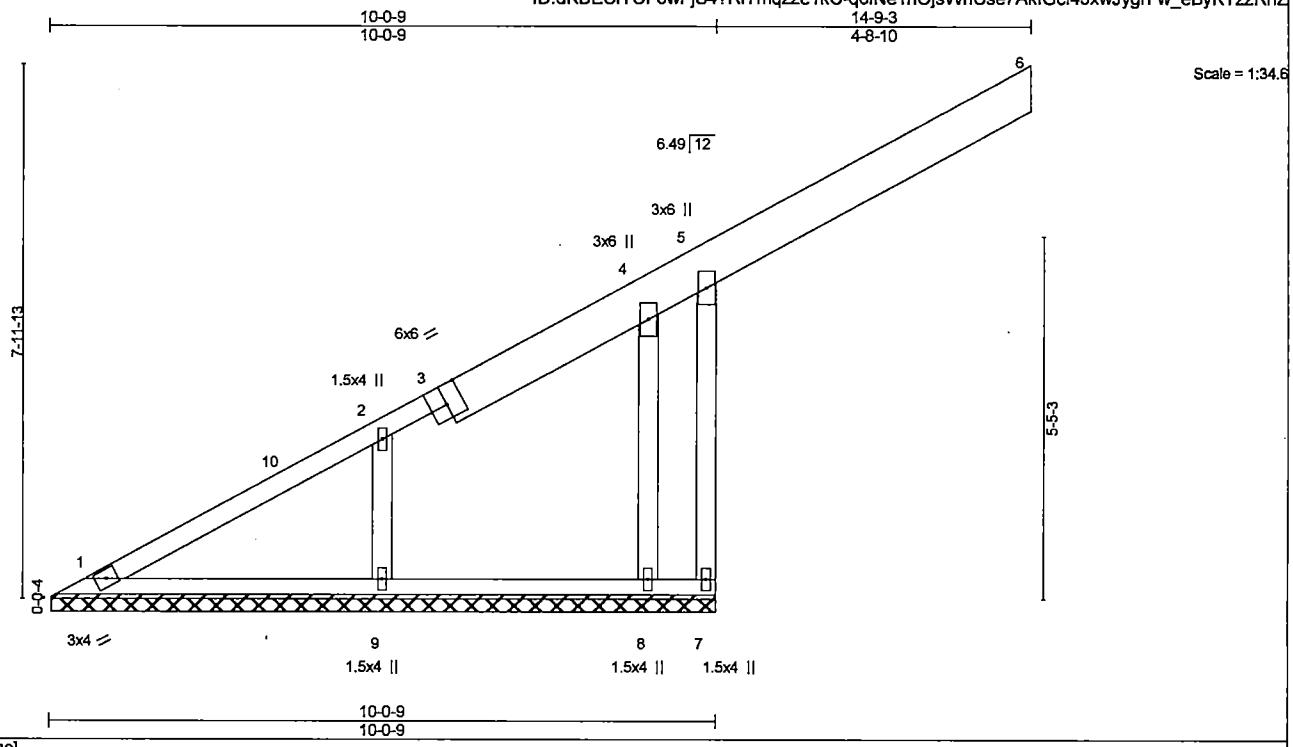


Plate Offsets (X,Y): [3:0-3-0,Edge]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.51	Vert(LL)	0.00	5	n/r	90	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.18	Vert(TL)	-0.08	5-6	n/r	90		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.16	Horz(TL)	0.00	7	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix)		Wind(LL)	0.09	5-6	n/r	120	Weight: 62 lb	FT = 10%

LUMBER
TOP CHORD 2x8 SP 2400F 2.0E *Except*
T1: 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud
OTHERS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 10-0-1.
(lb) - Max Horz 1=230(LC 5)
Max Uplift All uplift 100 lb or less at joint(s) 9 except 7=-397(LC 7), 8=-299(LC 1)
Max Grav All reactions 250 lb or less at joint(s) 8, 1 except 7=1038(LC 1), 9=399(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 4-5=-253/313, 5-7=-1047/595
WEBS 2-9=-300/117, 4-8=-289/361

NOTES
1) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-7-0 to 3-7-0, Interior(1) 3-7-0 to 10-6-4, Exterior(2) 10-6-4 to 14-9-3 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Plates checked for a plus or minus 5 degree rotation about its center.
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 7=397, 8=299.
7) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

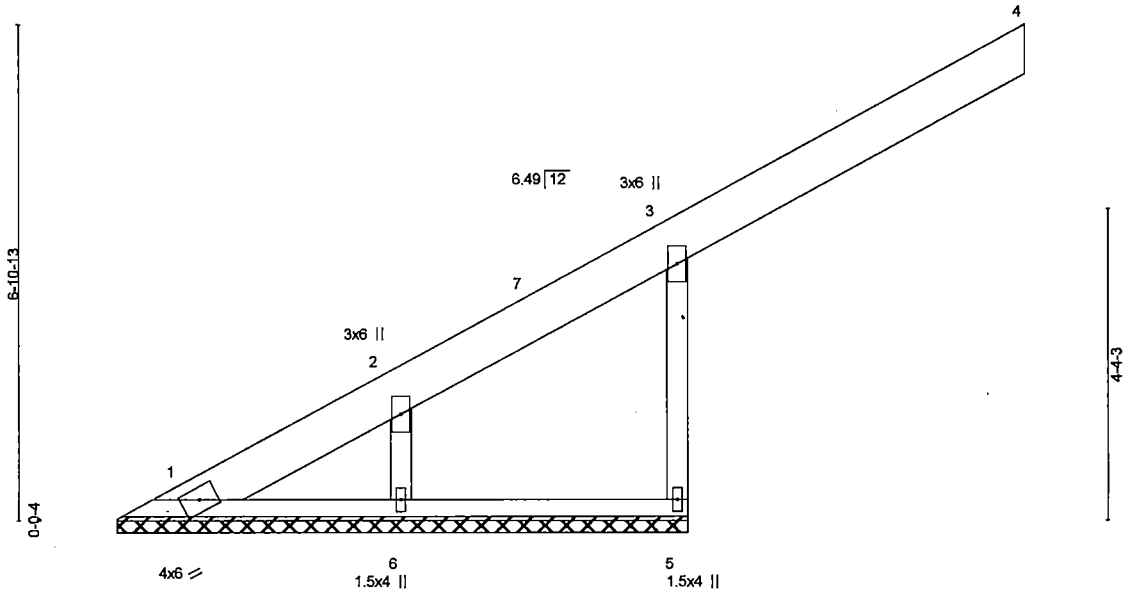
LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	AV2	Valley	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:19 2014 Page 1
ID:dKBEClYCF3wFjS4?RITmqzze1kC-IIGmsNn0UAeep0DJkRpV9zdHhKfqPKc3DIwVtvzzRnY

8-0-8 / 8-0-8 — 12-9-2 / 4-8-10

Scale: 3/8"=1'



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 30.0	Plates Increase 1.15	TC 0.31	Vert(LL) 0.00 3 n/r 90	MT20 197/144
TCDL 10.0	Lumber Increase 1.15	BC 0.14	Vert(TL) -0.08 3-4 n/r 90	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.02	Horz(TL) 0.00 5 n/a n/a	
BCDL 10.0	Code IRC2006/TPI2002	(Matrix)	Wind(LL) 0.09 3-4 n/r 120	Weight: 57 lb FT = 10%

LUMBER
TOP CHORD 2x8 SP 2400F 2.0E
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud
OTHERS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=179/8-0-1, 5=846/8-0-1, 6=64/8-0-1
Max Horz 1=197(LC 5)
Max Uplift 5=-264(LC 7), 6=-13(LC 6)
Max Grav 1=179(LC 1), 5=846(LC 1), 6=170(LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 3-5=-815/435

NOTES
1) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-10-14 to 4-0-4, Interior(1) 4-0-4 to 8-6-4, Exterior(2) 8-6-4 to 12-9-2 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Plates checked for a plus or minus 5 degree rotation about its center.
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6 except (jt=lb) 5=264.
7) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	AV3	Valley	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:20 2014 Page 1
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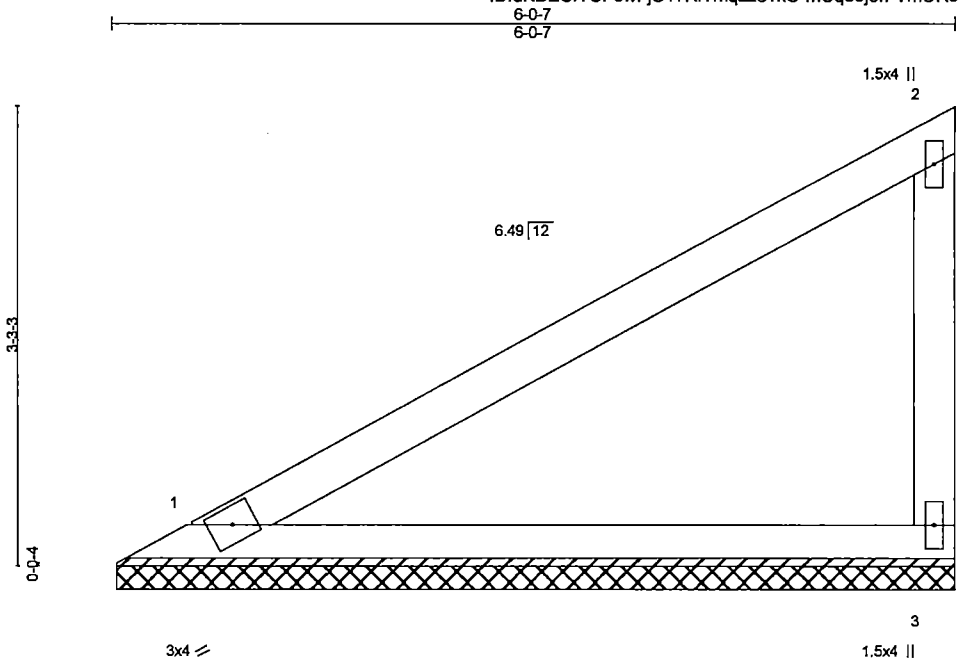

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LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.67	Vert(LL)	n/a	-	n/a	999	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.31	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix)							Weight: 17 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 6-0-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=265/6-0-0, 3=265/6-0-0
Max Horz 1=88(LC 5)
Max Uplift1=-6(LC 6), 3=-30(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES
1) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Plates checked for a plus or minus 5 degree rotation about its center.
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
7) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	AV4	Valley	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:21 2014 Page 1
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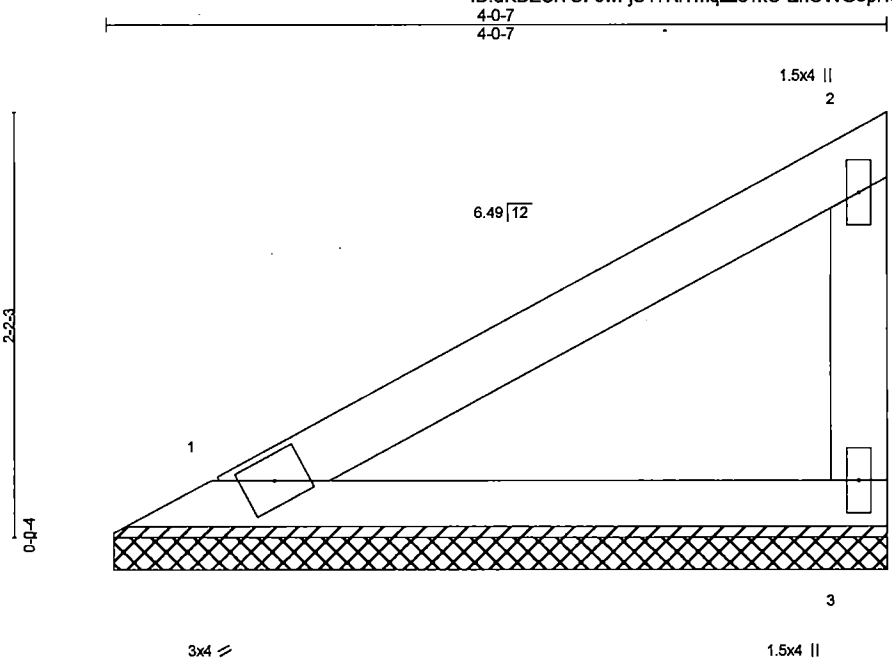

Scale: 1"=1'

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.26	Vert(LL)	n/a	-	n/a	999	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.12	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	0.00	3	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix)							Weight: 11 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 4-0-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=165/3-11-15, 3=165/3-11-15
Max Horz 1=55(LC 5)
Max Uplift1=-4(LC 6), 3=-19(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES
1) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Plates checked for a plus or minus 5 degree rotation about its center.
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
7) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	AV5	Valley	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:22 2014 Page 1
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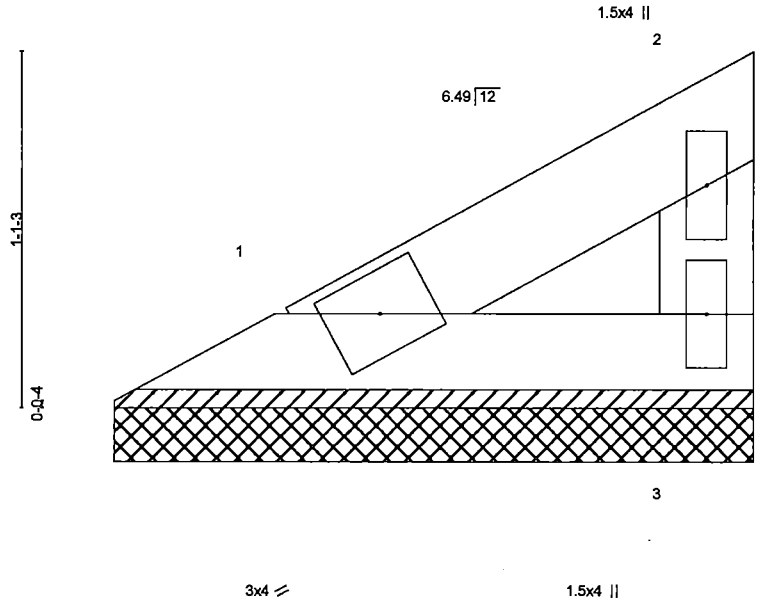

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LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES GRIP
TCLL 30.0	Plates Increase 1.15	TC 0.04 65#/-2#	Vert(LL) n/a - n/a 999	MT20 197/144
TCDL 10.0	Lumber Increase 1.15	BC 0.02	Vert(TL) n/a - n/a 999 65#/-7#	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(TL) 0.00 3 n/a n/a	
BCDL 10.0	Code IRC2006/TPI2002	(Matrix)		Weight: 5 lb FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 2-0-6 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=65/1-11-15, 3=65/1-11-15
Max Horz 1=22(LC 5)
Max Uplift1=-2(LC 6), 3=-7(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES
1) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Plates checked for a plus or minus 5 degree rotation about its center.
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
7) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	B2	Roof Special	6	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:25 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-6Sd16Qsn30OnXxgT4iwwPEtC4IYApwdybENq4ZzzRnS

3-6-14	7-1-0	9-3-4	15-8-12	22-2-4	26-2-0
3-6-14	3-6-3	2-2-4	6-5-8	6-5-8	3-11-12

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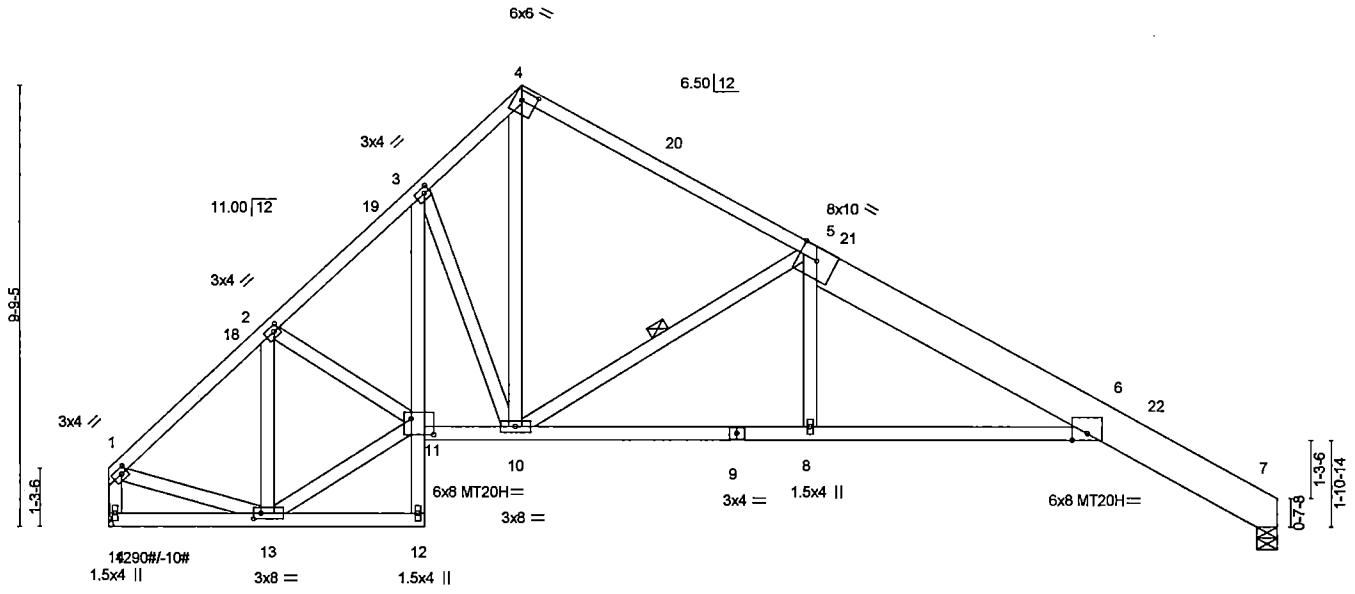

3-6-14	7-1-0	9-3-4	15-8-12	22-2-4	26-2-0	0-5-8(0-2-4)
3-6-14	3-6-3	2-2-4	6-5-8	6-5-8	3-11-12	1299#/-30#

Plate Offsets (X,Y): [1:0-1-8,0-1-8], [2:0-1-8,0-1-8], [3:0-1-8,0-1-8], [4:0-3-13,0-2-8], [11:0-6-0,0-4-4], [13:0-2-0,0-1-8]

LOADING (psf)		SPACING 2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase 1.15	TC 0.77	Vert(LL)	-0.33	8-17	>952	360	MT20	197/144
TCDL	10.0	Lumber Increase 1.15	BC 0.78	Vert(TL)	-0.67	8-17	>461	180	MT20H	148/108
BCLL	0.0 *	Rep Stress Incr YES	WB 0.50	Horz(TL)	0.43	7	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002	(Matrix-M)	Wind(LL)	0.14	8-17	>999	240	Weight: 149 lb	FT = 10%

R
IORD 2x4 SPF No.2 *Except*
T3: 2x10 SP 2400F 2.0E
BOT CHORD 2x4 SPF No.2 *Except*
B2: 2x4 SPF Stud
WEBS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 4-2-15 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-10

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 14=1290/Mechanical, 7=1299/0-5-8
Max Horz 14=-236(LC 4)
Max Uplift14=-10(LC 6), 7=-30(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-18=-1349/50, 2-18=-1183/66, 2-19=-1797/67, 3-19=-1651/92, 3-4=-1450/131, 4-20=-1154/107, 5-20=-1302/90, 5-21=-2016/70, 6-21=-2573/68, 6-22=-453/56, 7-22=-600/40, 1-14=-1252/50
BOT CHORD 3-11=-37/527, 10-11=0/1250, 9-10=0/2124, 8-9=0/2124, 6-8=-6/2118
WEBS 2-13=-786/39, 11-13=-11/1070, 2-11=0/371, 3-10=-555/98, 4-10=-58/1160, 5-10=-1288/82, 5-8=0/285, 1-13=0/912

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-1-12 to 3-1-12 , Interior(1) 3-1-12 to 6-3-4, Exterior(2) 6-3-4 to 9-3-4, Interior(1) 12-3-4 to 22-11-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) All plates are MT20 plates unless otherwise indicated.
4) Plates checked for a plus or minus 5 degree rotation about its center.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14, 7.
10) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	C1E	GABLE	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:28 2014 Page 1
ID:dKBECfYCF3wFjS4?RITmqzze1kC-X1J9lSugMxmMPOP2lqTd0sVgtyXJ0DsOHCcUhuzzRnP

-1-10-8	5-9-15	11-4-6	16-10-12	20-3-3	27-3-10	34-4-1	37-11-0
1-10-8	5-9-15	5-6-7	5-6-7	3-4-7	7-0-7	7-0-7	3-6-15

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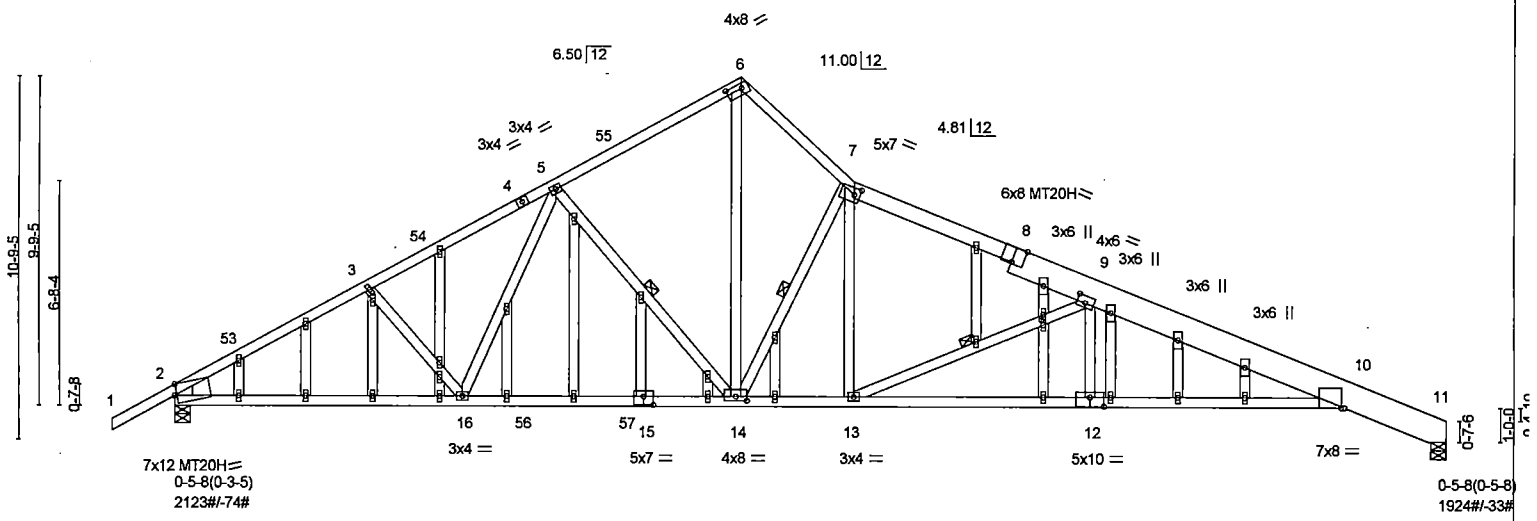

8-7-2	16-10-12	20-3-3	27-3-10	34-4-1	37-11-0
8-7-2	8-3-10	3-4-7	7-0-7	7-0-7	3-6-15

Plate Offsets (X,Y): [2:0-0-13,Edge], [6:0-5-8,0-1-12], [7:0-2-0,0-2-8], [8:0-4-0,Edge], [9:0-3-0,0-2-8], [10:0-1-2,Edge], [12:0-5-0,0-3-4], [14:0-4-0,0-1-8], [15:0-3-8,0-3-0], [25:0-1-13,0-0-12]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.92	Vert(LL)	-0.49	14-16	>924	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.95	Vert(TL)	-0.94	12-13	>481	180	MT20H	148/108
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.72	Horz(TL)	0.41	11	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.19	12-52	>999	240	Weight: 235 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2 *Except*
T4: 2x6 SPF No.2, T1: 2x4 SPF 2100F 1.8E, T5: 2x10 SP 2400F 2.0E
BOT CHORD 2x4 SPF 1650F 1.5E
WEBS 2x4 SPF Stud *Except*
W4: 2x4 SPF No.2
OTHERS 2x4 SPF Stud
WEDGE
Left: 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 5-14, 7-14, 9-13

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=2123/0-5-8, 11=1924/0-5-8
Max Horz 2=-190(LC 4)
Max Uplift2=-74(LC 6), 11=-33(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-53=-3334/52, 3-53=-3281/79, 3-54=-3059/74, 4-54=-2991/77, 4-5=-2888/95, 5-55=-2366/122, 6-55=-2248/150, 6-7=-2697/164, 7-8=-3184/115, 8-9=-3268/87, 9-10=-4830/102, 10-11=-705/44
BOT CHORD 2-16=0/2806, 16-56=0/2452, 56-57=0/2452, 15-57=0/2452, 14-15=0/2452, 13-14=0/2883, 12-13=-7/4589, 10-12=-47/4589
WEBS 3-16=-303/118, 5-16=0/492, 5-14=-766/83, 6-14=-95/2510, 7-14=-1950/112, 7-13=-9/827, 9-13=-1899/155, 9-12=0/321

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-10-8 to 1-11-0, Interior(1) 1-11-0 to 13-1-4, Exterior(2) 13-1-4 to 16-10-12, Interior(1) 20-3-3 to 33-10-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are MT20 plates unless otherwise indicated.
5) All plates are 1.5x4 MT20 unless otherwise indicated.
6) Plates checked for a plus or minus 5 degree rotation about its center.
7) Gable studs spaced at 2-0-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
10) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11.
12) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
13) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	C2	ROOF SPECIAL	4	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:30 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-TQRw98wwuY14eiZQtFV56Ha0NmDnU7MhlV5bmnzzRnN

-1-10-8	5-9-15	11-4-6	16-10-12	20-3-3	27-3-10	34-4-1	37-11-0
1-10-8	5-9-15	5-6-7	5-6-7	3-4-7	7-0-7	7-0-7	3-6-15

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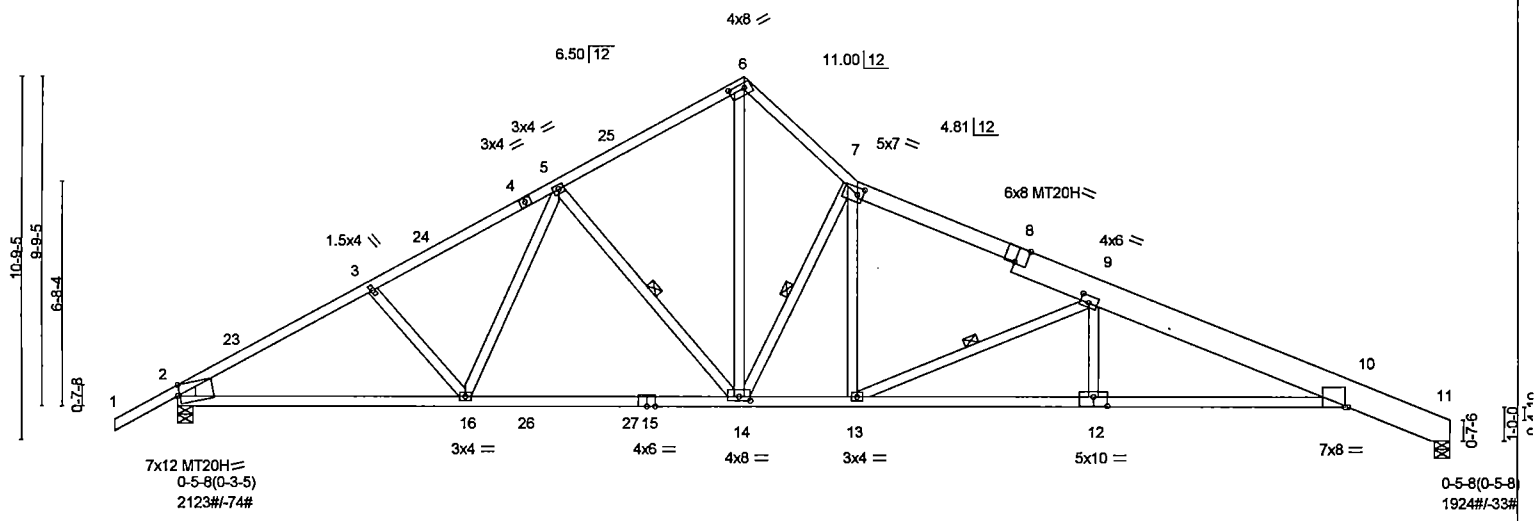


8-7-2	16-10-12	20-3-3	27-3-10	34-4-1	37-11-0
8-7-2	8-3-10	3-4-7	7-0-7	7-0-7	3-6-15

Plate Offsets (X,Y): [2:0-0-13,Edge], [6:0-5-8,0-1-12], [7:0-2-0,0-2-8], [8:0-4-0,Edge], [9:0-3-0,0-2-8], [10:0-1-2,Edge], [12:0-5-0,0-3-4], [14:0-4-0,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.92	Vert(LL)	-0.49	14-16	>924	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.95	Vert(TL)	-0.94	12-13	>481	180	MT20H	148/108
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.72	Horz(TL)	0.41	11	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.19	12-22	>999	240	Weight: 194 lb	FT = 10%

:R
HORD 2x4 SPF No.2 *Except*
T4: 2x6 SPF No.2, T1: 2x4 SPF 2100F 1.8E, T5: 2x10 SP 2400F 2.0E
BOT CHORD 2x4 SPF 1650F 1.5E
WEBS 2x4 SPF Stud *Except*
W4: 2x4 SPF No.2
WEDGE
Left: 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS 1 Row at midpt 5-14, 7-14, 9-13

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=2123/0-5-8, 11=1924/0-5-8
Max Horz 2=-190(LC 4)
Max Uplift2=-74(LC 6), 11=-33(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-23=-3334/52, 3-23=-3281/79, 3-24=-3059/74, 4-24=-2991/77, 4-5=-2888/95, 5-25=-2366/122, 6-25=-2248/150, 6-7=-2697/164, 7-8=-3184/115, 8-9=-3268/87, 9-10=-4830/102, 10-11=-705/44
BOT CHORD 2-16=0/2806, 16-26=0/2452, 26-27=0/2452, 15-27=0/2452, 14-15=0/2452, 13-14=0/2883, 12-13=-7/4589, 10-12=-47/4589
WEBS 3-16=-303/118, 5-16=0/492, 5-14=-766/83, 6-14=-95/2510, 7-14=-1950/112, 7-13=-9/827, 9-13=-1899/155, 9-12=0/321

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-10-8 to 1-11-0, Interior(1) 1-11-0 to 13-1-4, Exterior(2) 13-1-4 to 16-10-12, Interior(1) 20-3-3 to 33-10-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) All plates are MT20 plates unless otherwise indicated.
4) Plates checked for a plus or minus 5 degree rotation about its center.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
7) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11.
9) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

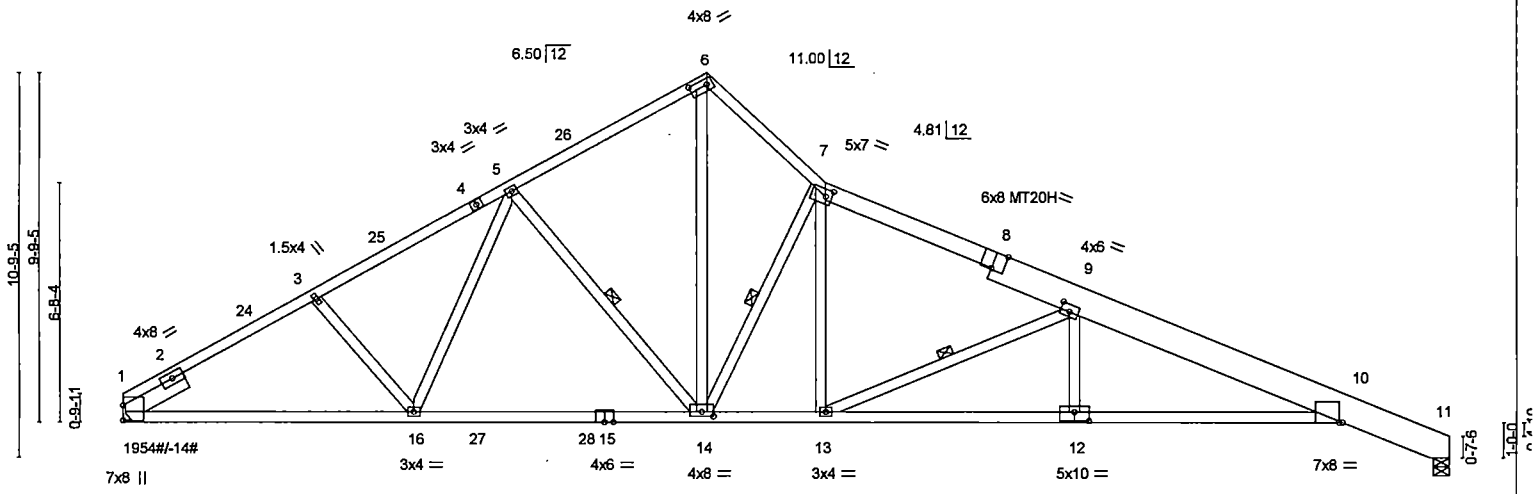
LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	C3	ROOF SPECIAL	7	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:32 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-PoYgaqxAQ9Hnt?ip_gYZBigM_avMy1v_CpahqfzzRnL

5-5-15 | 11-0-6 | 16-6-12 | 19-11-3 | 26-11-10 | 34-0-1 | 37-7-0
5-5-15 | 5-6-7 | 5-6-7 | 3-4-7 | 7-0-7 | 7-0-7 | 3-6-15

Scale = 1:65.2



0-5-8(0-5-7)

8-3-2 | 16-6-12 | 19-11-3 | 26-11-10 | 34-0-1 | 37-7-0 | 1910#/-33#
8-3-2 | 8-3-10 | 3-4-7 | 7-0-7 | 7-0-7 | 3-6-15

Plate Offsets (X,Y): [1:Edge,0-0-0], [6:0-6-0,0-2-0], [7:0-2-0,0-2-8], [8:0-4-0,Edge], [9:0-3-0,0-2-8], [10:0-1-2,Edge], [12:0-5-0,0-3-0], [14:0-4-0,0-1-8]

LOADING (psf)		SPACING 2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase 1.15	TC 0.92	Vert(LL)	-0.47	14-16	>957	360	MT20	197/144
TCDL	10.0	Lumber Increase 1.15	BC 0.94	Vert(TL)	-0.92	12-13	>487	180	MT20H	148/108
BCLL	0.0 *	Rep Stress Incr YES	WB 0.72	Horz(TL)	0.40	11	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002	(Matrix-M)	Wind(LL)	0.19	12-23	>999	240	Weight: 194 lb	FT = 10%

R
IORD 2x4 SPF No.2 *Except*
T4: 2x6 SPF No.2, T1: 2x4 SPF 2400F 2.0E, T5: 2x10 SP 2400F 2.0E
BOT CHORD 2x4 SPF 1650F 1.5E
WEBS 2x4 SPF Stud *Except*
W4: 2x4 SPF No.2
SLIDER Left 2x6 SPF No.2 2-0-0

BRACING
TOP CHORD Sheathed or 2-2-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
2-2-0 oc bracing: 12-23.
WEBS 1 Row at midpt 5-14, 7-14, 9-13

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=1954/Mechanical, 11=1910/0-5-8
Max Horz 1=-209(LC 4)
Max Uplift1=-14(LC 6), 11=-33(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-890/0, 2-24=-3159/89, 3-24=-3073/102, 3-25=-2954/96, 4-25=-2892/99, 4-5=-2782/116, 5-26=-2332/127, 6-26=-2213/156, 6-7=-2656/171, 7-8=-3144/123, 8-9=-3228/94, 9-10=-4785/110, 10-11=-700/45
BOT CHORD 1-16=-67/2656, 16-27=0/2398, 27-28=0/2398, 15-28=0/2398, 14-15=0/2398, 13-14=0/2846, 12-13=-14/4547, 10-12=-51/4547
WEBS 5-16=0/428, 5-14=-738/91, 6-14=-101/2469, 7-14=-1935/116, 7-13=-10/826, 9-13=-1894/155, 9-12=0/321

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-4-0 to 4-1-2, Interior(1) 4-1-2 to 13-1-11, Exterior(2) 13-1-11 to 16-10-12, Interior(1) 20-3-3 to 33-11-2 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) All plates are MT20 plates unless otherwise indicated.
4) Plates checked for a plus or minus 5 degree rotation about its center.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
7) Refer to girder(s) for truss to truss connections.
8) Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 11.
10) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	CV1	Valley	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:33 2014 Page 1
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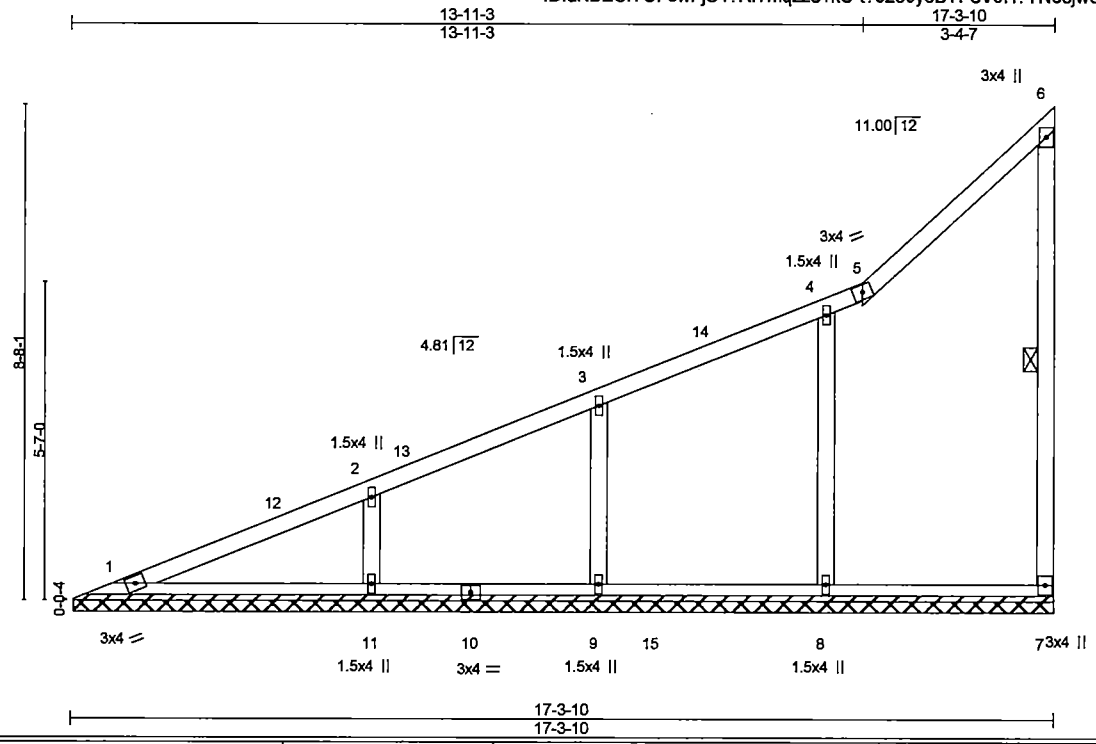

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.36	Vert(LL)	n/a	-	n/a	999	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.17	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.19	Horz(TL)	0.00	7	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix)							Weight: 61 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud
OTHERS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 6-7

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 17-3-0.
(lb) - Max Horz 1=264(LC 5)
Max Uplift All uplift 100 lb or less at joint(s) 7, 8, 9, 11
Max Grav All reactions 250 lb or less at joint(s) 1, 7 except 8=619(LC 1), 9=406(LC 1), 11=488(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 4-8=-350/106, 3-9=-296/96, 2-11=-384/98

NOTES
1) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-9-6 to 3-9-6, Interior(1) 3-9-6 to 13-11-3, Exterior(2) 13-11-3 to 17-1-14 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Plates checked for a plus or minus 5 degree rotation about its center.
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 8, 9, 11.
7) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	CV2	Valley	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:34 2014 Page 1
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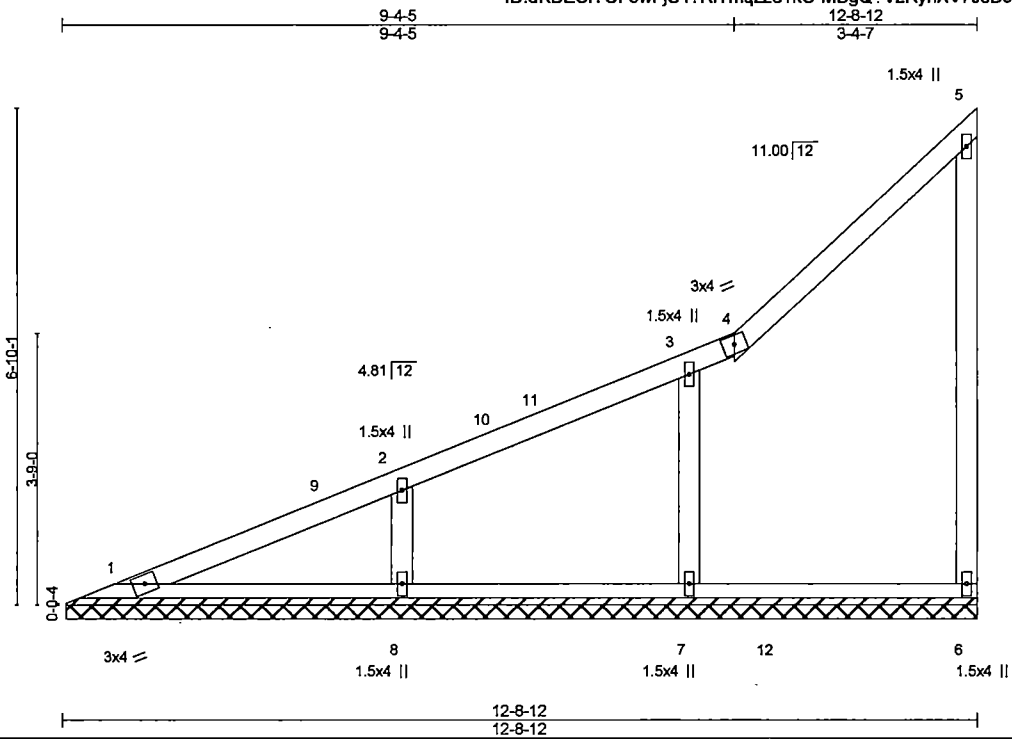

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.49	Vert(LL)	n/a	-	n/a	999	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.17	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.10	Horz(TL)	0.00	6	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix)							Weight: 42 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud
OTHERS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 12-8-2.
(lb) - Max Horz 1=203(LC 5)
Max Uplift All uplift 100 lb or less at joint(s) 1, 6, 7, 8
Max Grav All reactions 250 lb or less at joint(s) 1, 6 except 7=473(LC 1), 8=437(LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 3-7=-330/119, 2-8=-361/105

NOTES
1) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-9-6 to 3-9-6, Interior(1) 3-9-6 to 9-4-5, Exterior(2) 9-4-5 to 12-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Plates checked for a plus or minus 5 degree rotation about its center.
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 6, 7, 8.
7) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
8) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	CV3	Valley	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:35 2014 Page 1
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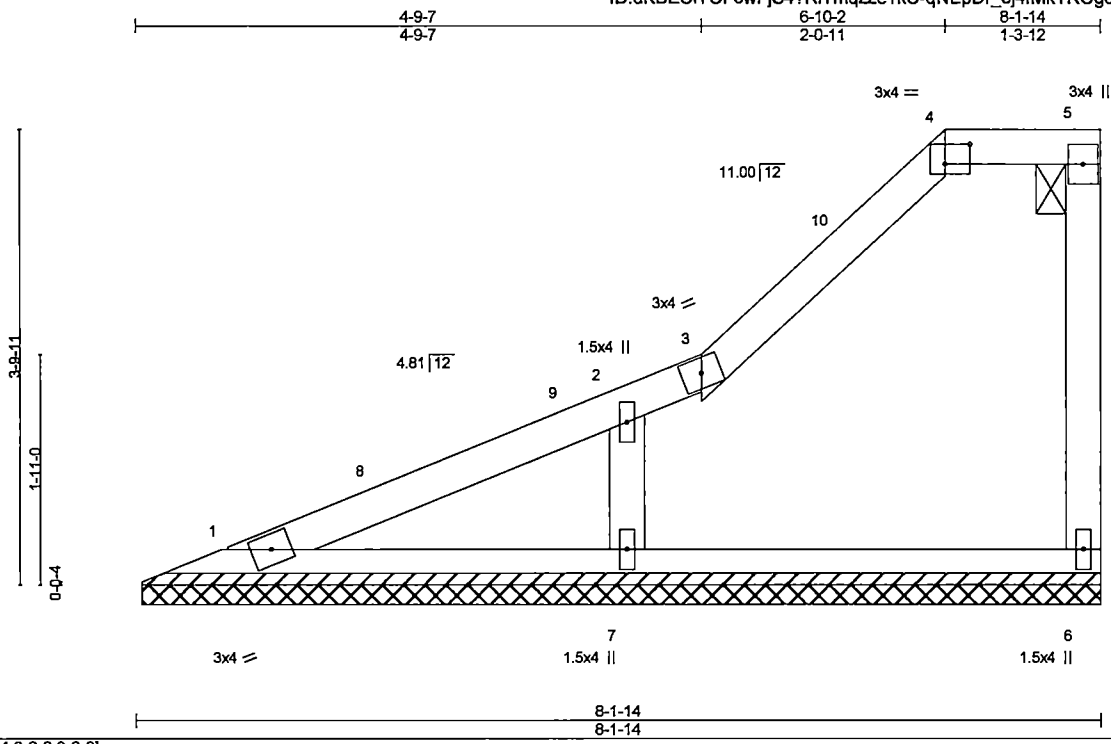

Plate Offsets (X,Y): [4:0-2-8,0-2-0]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.26	Vert(LL)	n/a	-	n/a	999	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.09	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.07	Horz(TL)	0.00	6	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix)							Weight: 24 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud
OTHERS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=136/8-1-4, 6=164/8-1-4, 7=422/8-1-4
Max Horz 1=111(LC 5)
Max Uplift1=-12(LC 4), 6=-26(LC 5), 7=-50(LC 6)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-7=-333/116

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-9-6 to 3-9-6, Interior(1) 3-9-6 to 4-9-7, Exterior(2) 4-9-7 to 6-10-2 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Provide adequate drainage to prevent water ponding.
4) Plates checked for a plus or minus 5 degree rotation about its center.
5) Gable requires continuous bottom chord bearing.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 6, 7.
9) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
10) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	D2	COMMON	3	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:36 2014 Page 1
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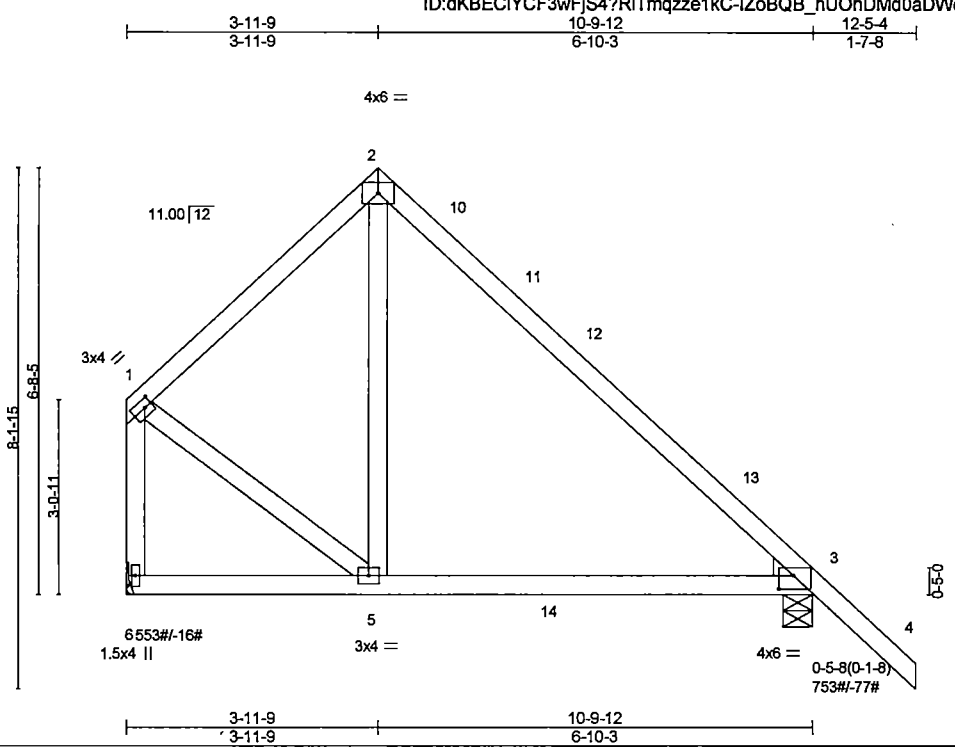

Plate Offsets (X,Y): [1:0-1-8,0-1-8], [3:0-2-12,0-2-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.69	Vert(LL)	-0.09	5-9	>999	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.48	Vert(TL)	-0.17	5-9	>739	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.13	Horz(TL)	-0.01	3	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.05	5-9	>999	240	Weight: 47 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud
WEDGE
Right: 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 5-4-14 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=553/Mechanical, 3=753/0-5-8
Max Horz 6=-204(LC 4)
Max Uplift6=-16(LC 6), 3=-77(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-412/72, 10-11=-298/44, 11-12=-329/41, 12-13=-351/37, 3-13=-1036/110, 1-6=-556/39
BOT CHORD 3-14=-453/2148
WEBS 1-5=-16/303

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-1-12 to 6-11-9 , Interior(1) 6-11-9 to 9-5-4, Exterior(2) 9-5-4 to 12-5-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Plates checked for a plus or minus 5 degree rotation about its center.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 3.
8) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	D3	Common	9	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:38 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-Eyvxrt0x0?1xbwAzLxfzQzvQj_2bMuZsbl102JzzRnF

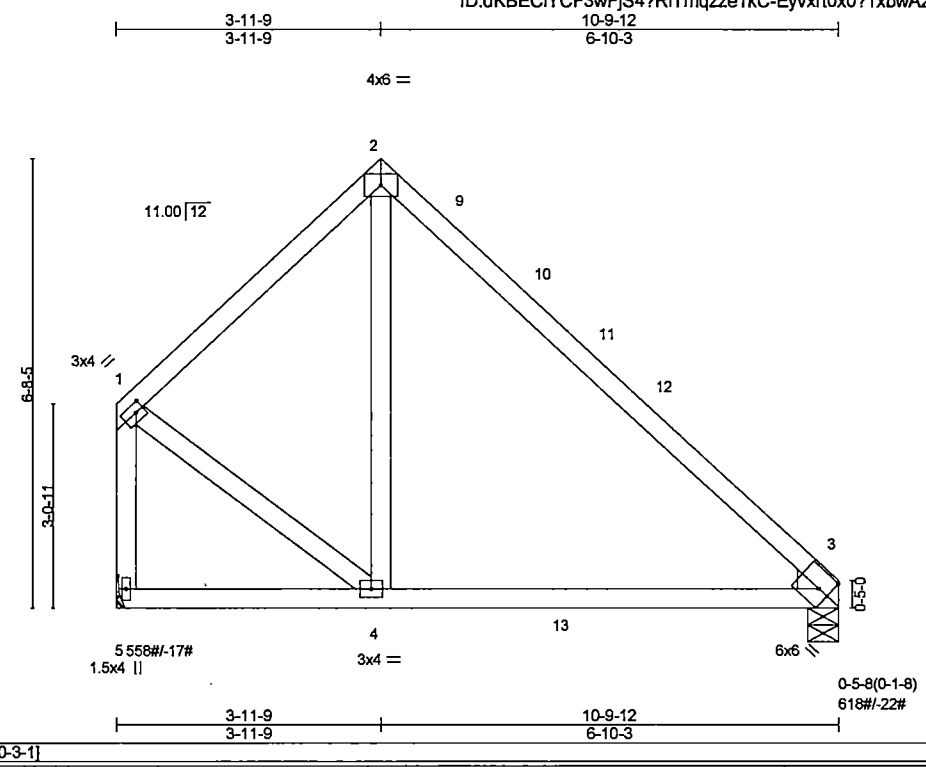

Plate Offsets (X,Y): [1:0-1-8,0-1-8], [3:Edge,0-3-1]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.71	Vert(LL)	-0.10	4-8	>999	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.57	Vert(TL)	-0.19	4-8	>672	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.13	Horz(TL)	-0.01	3	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.07	4-8	>999	240	Weight: 45 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud
WEDGE
Right: 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 4-10-5 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 5=558/Mechanical, 3=618/0-5-8
Max Horz 5=-181(LC 4)
Max Uplift5=-17(LC 6), 3=-22(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-418/80, 9-10=-303/49, 10-11=-335/45, 11-12=-355/41, 3-12=-1213/275, 1-5=-564/47
BOT CHORD 3-13=-810/2522
WEBS 1-4=-17/308

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-1-12 to 6-11-9 , Interior(1) 6-11-9 to 7-9-12, Exterior(2) 7-9-12 to 10-9-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Plates checked for a plus or minus 5 degree rotation about its center.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.
8) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	D4	Common	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:39 2014 Page 1
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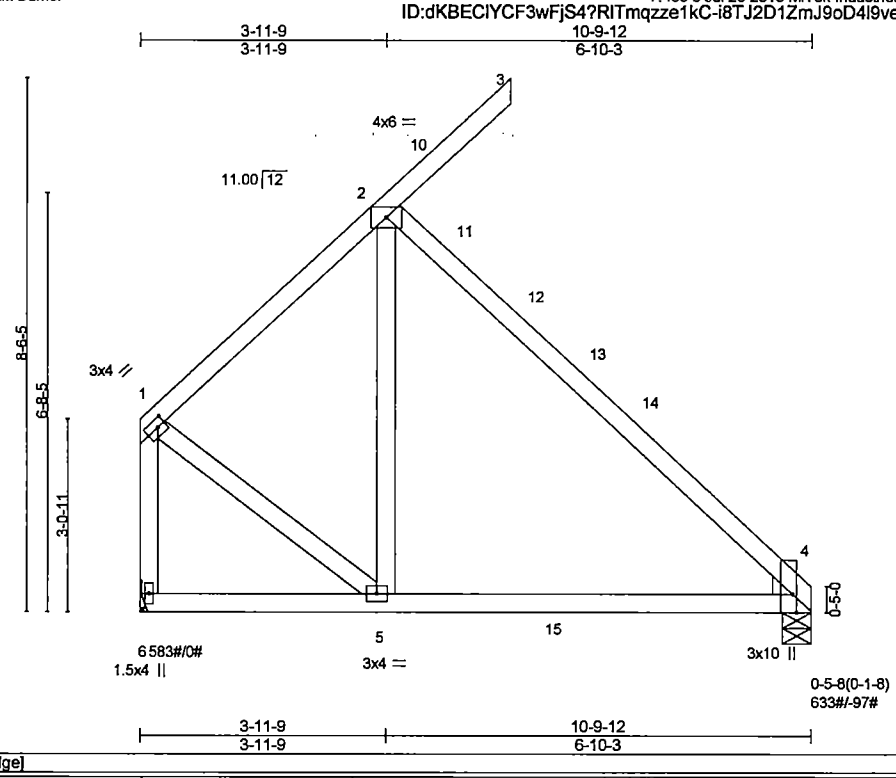


Plate Offsets (X,Y): [1:0-1-8,0-1-8], [4:0-3-8,Edge]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.60	Vert(LL)	-0.09	5-9	>999	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.52	Vert(TL)	-0.18	5-9	>714	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.15	Horz(TL)	-0.01	4	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.07	5-9	>999	240	Weight: 48 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud
WEDGE
Right: 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 5-1-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=583/Mechanical, 4=633/0-5-8
Max Horz 6=208(LC 5)
Max Uplift4=-97(LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-442/72, 2-11=-321/337, 11-12=-336/326, 12-13=-388/322, 13-14=-390/317, 4-14=-1145/379, 1-6=-586/20
BOT CHORD 5-15=-116/286, 4-15=-852/2300
WEBS 1-5=0/357

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-1-12 to 6-11-9 , Interior(1) 6-11-9 to 7-9-12, Exterior(2) 7-9-12 to 10-9-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Plates checked for a plus or minus 5 degree rotation about its center.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
8) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	D5	Roof Special	5	1	Job Reference (optional)

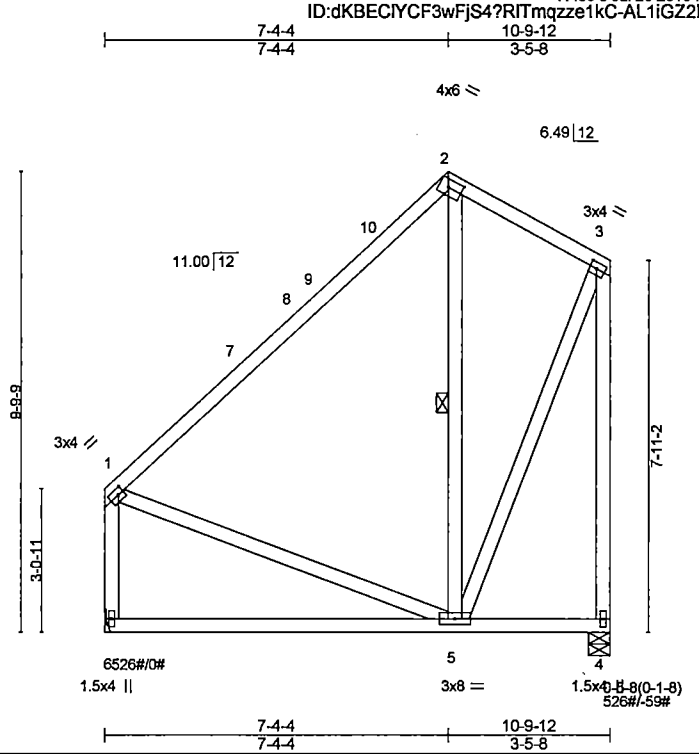


Plate Offsets (X,Y): [1:0-1-8,0-1-8], [2:0-3-9,0-2-0], [3:0-1-12,0-1-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.73	Vert(LL)	-0.10	5-6	>999	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.43	Vert(TL)	-0.25	5-6	>502	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.15	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.00	5	>999	240	Weight: 68 lb	FT = 10%

LUMBER
TOP CHORD 2x4 SPF 2400F 2.0E *Except*
T2: 2x4 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 2-5

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=526/Mechanical, 4=526/0-5-8
Max Horz 6=299(LC 5)
Max Uplift4=-59(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-7=-341/54, 1-6=-466/68, 3-4=-525/59
BOT CHORD 5-6=-272/224
WEBS 3-5=-90/347

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 4-4-4, Exterior(2) 4-4-4 to 7-4-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Plates checked for a plus or minus 5 degree rotation about its center.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
8) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	D6	Roof Special	1	1	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:42 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-7j9ShE3S3EXN4YTkamjvbp44UcQGlg5SVN?DB4zzRnB

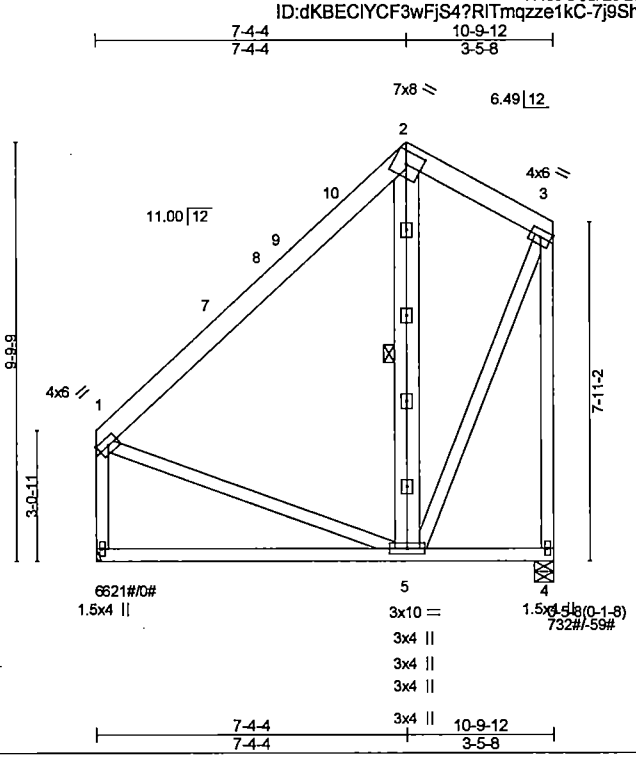


Plate Offsets (X,Y): [2:0-4-5,0-3-8]

LOADING (psf)		SPACING 2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.85	Vert(LL)	-0.09	5-6	>999	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.45	Vert(TL)	-0.24	5-6	>536	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.23	Horz(TL)	-0.00	4	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.00	5	>999	240	Weight: 87 lb	FT = 10%

LUMBER
TOP CHORD 2x6 SPF No.2
BOT CHORD 2x4 SPF No.2
WEBS 2x4 SPF Stud

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 2-5

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 6=621/Mechanical, 4=732/0-5-8
Max Horz 6=296(LC 5)
Max Uplift4=-59(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-7=-441/59, 7-8=-285/64, 8-9=-252/69, 2-3=-288/144, 1-6=-561/74, 3-4=-726/61
BOT CHORD 5-6=-270/222
WEBS 2-5=-414/106, 3-5=-91/523

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 4-4-4, Exterior(2) 4-4-4 to 7-4-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Plates checked for a plus or minus 5 degree rotation about its center.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.
8) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
9) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 300 lb down and 21 lb up at 7-4-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.
11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-2=-80, 2-3=-80, 4-6=-20
Concentrated Loads (lb)
Vert: 2=-300(F)

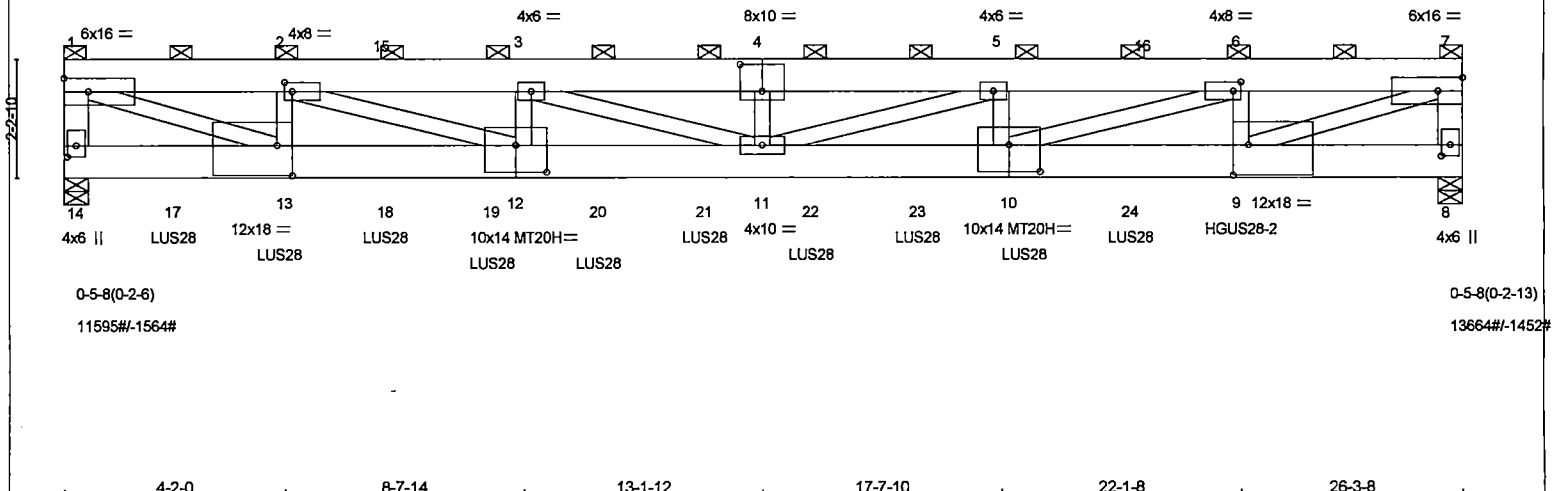
Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	EG	FLAT GIRDER	1	4	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:44 2014 Page 1
ID:dKBECfYCF3wFjS4?RITmqzze1kC-36HC5w5ibm4Jrd6hBlNgE9UkP28mP0lzhUKFzzzRn9

4-2-0	8-7-14	13-1-12	17-7-10	22-1-8	26-3-8
4-2-0	4-5-14	4-5-14	4-5-14	4-5-14	4-2-0

Scale = 1:43.2

MARK "UP" ON TRUSSES IN SHOP!!



4-2-0	8-7-14	13-1-12	17-7-10	22-1-8	26-3-8
4-2-0	4-5-14	4-5-14	4-5-14	4-5-14	4-2-0

Plate Offsets (X,Y): [2:0-1-12,0-2-0], [4:0-5-0,0-6-0], [6:0-1-12,0-2-0], [8:0-2-8,0-2-0], [9:0-3-8,0-6-12], [10:0-7-0,0-6-0], [12:0-7-0,0-6-0], [13:0-3-8,0-6-12], [14:0-2-8,0-2-0]

LOADING (psf)		SPACING 8-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.61	Vert(LL)	-0.58	11	>536	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.75	Vert(TL)	-0.93	11	>332	180	MT20H	148/108
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.90	Horz(TL)	0.10	8	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.25	11	>999	240	Weight: 798 lb	FT = 10%

LUMBER
TOP CHORD 2x8 SP 2400F 2.0E
BOT CHORD 2x8 SP 2400F 2.0E
WEBS 2x4 SPF Stud *Except*
W1: 2x6 SPF No.2, W2: 2x4 SPF 2100F 1.8E, W4: 2x4 SPF 1650F 1.5E

BRACING
TOP CHORD 2-0-0 oc purlins (6-0-0 max.), except end verticals (Switched from sheeted: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 14=11595/0-5-8, 8=13664/0-5-8
Max Horz 14=-186(LC 3)
Max Uplift14=-1564(LC 3), 8=-1452(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-14=-10448/1474, 1-2=-24362/3338, 2-15=-40899/5487, 3-15=-40899/5487, 3-4=-48131/6290, 4-5=-48131/6290, 5-16=-44088/5485, 6-16=-44088/5485, 6-7=-29335/3292, 7-8=-12644/1458
BOT CHORD 14-17=-372/1767, 13-17=-372/1767, 13-18=-3419/24362, 18-19=-3419/24362, 12-19=-3419/24362, 12-20=-5622/41245, 20-21=-5622/41245, 11-21=-5622/41245, 11-22=-5584/44346, 22-23=-5584/44346, 10-23=-5584/44346, 10-24=-3308/29335, 9-24=-3308/29335, 8-9=-259/2101
WEBS 1-13=-3326/24417, 2-13=-7115/1088, 2-12=-2324/17640, 3-12=-3837/607, 3-11=-823/7317, 4-11=-1253/316, 5-11=-837/4021, 5-10=-2871/605, 6-10=-2370/15737, 6-9=-5987/1112, 7-9=-3295/29431

NOTES
1) Special connection required to distribute web loads equally between all plies.
2) 4-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x8 - 2 rows staggered at 0-9-0 oc.
Bottom chords connected as follows: 2x8 - 4 rows staggered at 0-4-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc, Except member 6-9 2x4 - 2 rows staggered at 0-4-0 oc.
Attach TC w/ 1/2" diam. bolts (ASTM A-307) in the center of the member w/washers at 4-0-0 oc.
Attach BC w/ 1/2" diam. bolts (ASTM A-307) in the center of the member w/washers at 4-0-0 oc.
3) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
4) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
5) Provide adequate drainage to prevent water ponding.
6) All plates are MT20 plates unless otherwise indicated.
7) Plates checked for a plus or minus 5 degree rotation about its center.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 14=1564, 8=1452.
11) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
12) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
13) Use Simpson Strong-Tie LUS28 (6-10d Girder, 4-10d Truss) or equivalent spaced at 2-0-0 oc max. starting at 2-0-12 from the left end to 20-0-12 to connect truss(es) A2 (1 ply 2x4 SPF), A3 (1 ply 2x4 SPF) to back face of bottom chord.
14) Use Simpson Strong-Tie HGUS28-2 (36-10d Girder, 12-10d Truss) or equivalent at 22-1-8 from the left end to connect truss(es) A4G (2 ply 2x6 SPF) to back face of bottom chord.
15) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced) + Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-7=-320, 8-14=-80
Concentrated Loads (lb)
Vert: 13=-968(B) 10=-968(B) 9=-5238(B) 17=-968(B) 18=-968(B) 19=-968(B) 20=-968(B) 21=-968(B) 22=-968(B) 23=-968(B) 24=-974(B)

$94.00 Receipt # 48419

VILLAGE OF FOX POINT
MILWAUKEE COUNTY, WISCONSIN

No. 7999

APPLICATION AND RECORD FOR HEATING PERMIT

TO THE BUILDING INSPECTION DEPARTMENT:

The undersigned hereby applies for a permit to install, in accordance with the information tabulated hereafter,

Winter Air Conditioner ☐ Type ____________
Forced Air, Radiant, Baseboard, Etc.

Fuel: ☑ Gas ☐ Oil ☐ Coal ☐ Elect. ☐ Other ____________

Desc. of Heating Plant Bryant furnace 100,000 BTU

Vented to Outside

Fuel Tank ☐: ____________ Size ____________ Location

Summer Air Conditioner ☐ Size 5 ton (Ton, H.P.)

Coolant R410 a refrigerant.

Compressor Coolant: Air ☒; Water ☐;
If Water Cooled:
Source of Water ____________
Discharged to ____________

Location of unit on premises including distances to lot lines required for approval of exterior apparatus.

Incinerator ☐ Manufacturer's Name ____________

Model No. ____________ Capacity ____________ Bushels

Has installation permit been issued by Milwaukee Co. Dept. of Air Pollution Control? ____________

Remarks ____________

The undersigned acknowledges that he is familiar with Ordinance No. 261, and all amendments thereto and that the work described herewith shall conform in all respects to said ordinances and all other ordinances of the Village of Fox Point, and laws of the State of Wisconsin. Any variations of the above may cause immediate revocation of the permit, if granted.

Owner Wired Construction

Address of Work 1015 E Quarles Pl.

OFFICE USE ONLY
Application Approved:
Installation Approved:

Signed [signature]

Date 10/1/14

~~AC unit comply with Village~~ [illegible] 10/14

Contractor Lake Country Heating & Cooling
Address N6333 Hwy F Phone 262-593-8300
City Oconomowoc State WI Zip 53066
State Credential Number 666679 & Expiration Date 6/30/15

VILLAGE OF FOX POINT
MILWAUKEE COUNTY
WISCONSIN

VILLAGE HALL
7200 N. SANTA MONICA BLVD.
FOX POINT 53217-3505
414-351-8900
FAX 414-351-8909

January 1, 2012

Dear Heating Contractor:

I am writing to inform you that on January 1, 2012 the permit fee schedule will be as follows:

Heating	$35.00/unit up to and including 150,000 input BTU units. Additional fee of $12.00 each 50,000 BTU or fraction thereof.
Heating & Air Conditioning Distribution Systems	$2.00/100 square foot of conditioned air
	35+24= 59
Air Conditioning	$35.00/unit up to 3 tons or 36,000 BTUs. Additional fee of $12.00 each ton or 12,000 BTUs or fraction thereof.
Minimum fee of all permits	$60.00

If you have any questions please feel free to contact me.

Sincerely,

Scott Miller
Inspector

Wisconsin Department of Safety and Professional Services

Credential/Licensing Search

Credential/License Search

DSPS Home

Credential/License Search DSPS Home

Trade Search Results

Credential/License ID	Name	City,State,Zip	Profession	Expiration
662679	LAKE COUNTRY HEATING & COOLING INC,	OCONOMOWOC WI 53066	HVAC Contractor	6/30/2017

Return to Search

1W1R1015

Plumber Alpine Plumbing
Address 1458 W. Greenfield Ave
City, State, Zip Brookfield WI 53005
Tel. No. 262-797-4130

No. 13550

Application and Record

Village of Fox Point
7200 N. Santa Monica Blvd.
Fox Point, WI 53217
(414) 351-8900

Owner Wred Construction
Address 4526 N. Oakland Whitefish Bay
Date 4-24, 2014

TO THE VILLAGE OF FOX POINT, PLUMBING AND WATER INSPECTION DEPARTMENT:
The undersigned hereby make application to do the work of plumbing consisting of
laying a _____ inch _____ builder sewer from Main to Lot line to Building
laying a _____ inch _____ water service from Main to Lot line to Building
at

1015 E Quarles Place, Fox Point, WI
Address at which work is to be done

PERMITS USED	
Kind	No.
Sewer and Plumbing	13003
Water	
Street	
Meter	
Water Usage	

Subdivision	Lot	Block

In the performance of this work the undersigned Plumber hereby agrees to bounden by and submit to all statutes, Village ordinances and rules and regulations prescribed by the Village Board for Plumbers.

State Master Lic.# 220705 Expir. 3/15 Building Contractor Reg# 120256 Expir. _____

HVAC Contractor Reg# _____ Expir. _____ Signature _____ Applicant

FIXTURE WITH DRAIN OR WATER CONNECTIONS

	No.		No.
Hose Bibs	3	Dishwashers	1
Bath Tubs	1	Wash Basins	8
Sump Pumps	1	Water Closets	7
Laundry Trays	1	Showers	3
Drinking Fountains		Floor Drains	1
Sinks	1	Food Waste Grinders	1
Water Heaters	1	Sprinkling System	
Wash. Mach. Wastes	1	Urinals	
Bidets			
Catch Basins			

FEES

Water Usage		$
Building Sewer		
Water Service		
Building Drain		50.00
Fixtures	30	240.00
Water Meter		
Total		290.00

Deposit to cover street repairs Receipt #46826

(A current certificate of insurance must be provided when doing work in road right of way (ROW)).

_____ Permit Clerk

A _____ inch _____ water service pipes laid in _____

Curb box is located _____ feet _____ of _____ feet _____ of _____

_____ inch _____ Water Meter No. _____ Date Installed _____

A _____ inch _____ sanitary sewer connection was made in _____

_____ feet _____ of manhole _____

A _____ inch _____ storm sewer connection was made in _____

_____ feet _____ of manhole _____

Building Sewer	Report	Building Drain	Report	Rough In Plumbing Inspection	Report	Final Inspection	Report

Installation Approved _____ Application Approved _____, 20_____

As Built _____

_____
Water and Plumbing Inspector

Search for Individual or Company by Credential ID here:

Specific Credential ID 220705

Search

1 record(s) were returned by your search.

ID	Name	City,State,Zip	Credential Type	Expiration
220705	HOLLE, CHRISTOPHER F	MENOMONEE FALLS WI 53051-6327	Master Plumber	03/31/15

Receipt No: 1.046826 May 08, 2014

1015 E QUARLES

LICENSES & PERMITS-PLUMBING PERMIT	290.00
24-44470 PLUMBING PERMIT	
Total:	290.00

CHECK	Chk No: 027651	290.00
Total Applied:		290.00
Change Tendered:		.00

05/08/14 11:46am

VILLAGE OF FOX POINT
7200 N. SANTA MONICA BLVD
FOX POINT, WI 53217 414-351-8900

CONTRACTOR USE
Date
State Master Electrician Lic. No. 170072
State Elect. Contractor Cert. No. 1098009
Village Elect. Contractor Cert. No.

VILLAGE OF FOX POINT

7200 N. Santa Monica Blvd.
Fox Point 53217
414-351-8900

APPLICATION FOR ELECTRICAL PERMIT

PLEASE TYPE OR PRINT WITH BALL POINT PEN

OFFICE USE ONLY	
Permit No.	20836
Received	5/15/14
Service	8/1/14
Rough-In	
Final	

Builder Wired Construction | Owner | Occupant

Job Address 1015 E Quarles Place

ESTIMATED COST OF JOB $15,000

Buildings
- [X] Residential
- [ ] Commercial
- [ ] Industrial
- [ ] Institutional

[X] New Construction
- [ ] Additional Rooms
- [ ] Remodeling
- [ ] New Occupancy

Where on Premises?

Describe _____

List Name of Installing Contractor

HEATING _____

AIR CONDITIONING _____

PLUMBING _____

Date of Inspection

Rough _____ Will Call ☐

Final _____ Will Call ☐

Service Approval Sent _____ ☐

REMARKS: 7/25/14

1) Explore both Gas Pipes

2) Raise Chase to Fill-Line on Permit

No.	Description	Qty.	Rate of Fees	Dollars	Cents
1	Light, switch, and convenience outlets	100	.70 ea	70	00
2	Lighting Fixtures	45	.70 ea	31	50
3	Fluorescent Fixtures - per tube		.70 ea		
4	Range, Electric	1	8.00 ea	8	00
5	Garbage Grinding and Disposal Unit	1	8.00 ea	8	00
6	Dishwasher	1	8.00 ea	8	00
7	Clothes Dryer	1	8.00 ea	8	00
8	Water Heaters, Electric		8.00 ea		
9	Gas Burner, Oil Burner, or Stoker		8.00 ea		
10	Refrigerating, Air Cooling, or similar machine - .25 per HP	1	8.00 min	8	00
11	Feeders - No. 6 A.W.G. or Larger		10.00 ea		
12	Temporary Service Permit for: How Long?		30.00 ea		
13	Services: Service Switches, ea.	1	5.00 ea	5	00
	Service 1. 0 through 100 amps.		25.00 ea		
	2. 101 through 400 amps.	1	40.00 ea	40	00
	3. 401 through 600 amps.		40.00 ea		
	4. 601 through 1000 amps.		60.00 ea		
	5. Thereafter, ea. additional 1000 amps.		5.00 ea		
14	Motors over 1/4 HP		.70 per HP or frac.		
15	Fuel Dispensing Pumps		6.00 ea		
16	Transformers, Rectifiers, and Generators		2.00 per KW		
17	Space Heating Systems, per circuit		4.00 ea		
18	Power receptacles - 120 Volts or over 1.Through 30 amps.		3.00 ea		
19	2.Over 30 amps.		5.00 ea		
20	Wireways, busways, underfloor raceways, or auxiliary gutters		.50 per ft.		
21	Strip Lighting, Plug-in Strip, Trol-E Duct, or similar system		.50 per ft.		
22	Signs, Electric-.75 ea. socket, plus 2.00 ea. add'tl transformer		8.00 min		
23	Swimming Pool Wiring: A. Inground pools		40.00		
24	B. Above ground pools		40.00		
25	Spas, Hot-Tubs, Hydromassage Bathtubs		6.00		
26	**MINIMUM CHARGE FOR ANY ONE PERMIT**		60.00		
27	**FAILURE TO CALL FOR FINAL INSPECTION**		15.00		
28	**DOUBLE FEES will be charged for any work started before obtaining permit.**	Receipt #		46	758
			TOTAL FEES	186	50

The undersigned hereby makes application for a permit for the execution of electrical installation for light, heat or power as prescribed and agrees to comply with all applicable State and Local Codes and Ordinances regulating the installation of electrical wiring and equipment in the Village of Fox Point.

MAKE CHECKS PAYABLE TO:
Treasurer, Village of Fox Point
MAIL TO: Electrical Inspector

Contractor Current Electric Co | Supervising Electrician (Signature) | Date 4/16/2014

Address 17625 W Burleigh Road | Telephone 262 786 5885 | 262 883 9415

City Brookfield | State WI | Zip Code 53005

This Permit is void if work is not started within 2 months, or if started, no work is done for 2 months.

Receipt No: 1.046758 May 05, 2014

1015 E QUARLES PLACE

LICENSES & PERMITS-ELECTRICAL PERMIT	186.50
24-44430 ELECTRICAL PERMIT	
Total:	186.50

CHECK	Chk No: 47737	186.50
Total Applied:		186.50
Change Tendered:		.00

05/05/14 09:21am

VILLAGE OF FOX POINT
7200 N. SANTA MONICA BLVD
FOX POINT, WI 53217

414-351-8900

VILLAGE OF FOX POINT
7200 N. Santa Monica Blvd.
Fox Point, WI 53217
Phone 414-351-8900
Fax 414-351-8909

Fax

To:	WE ENERGIES-CENTRALIZED GROUP	**From:**	Scott Miller
Fax:	262-574-6401	**Pages:**	2
Phone:		**Date:**	
Re:		**cc:**	

☐ **Urgent** X **For Review** ☐ **Please Comment** ☐ **Please Reply** ☐ **Please Recycle**

ELECTRICAL INSPECTION APPROVED

FAX TO (262)574-6401 PHONE (866)423-0364

COUNTY Milwaukee C T (V) MUNICIPALITY Fox Point INSPECTION NUMBER _____

INSPECTOR Scott Miller

CUSTOMER NAME Frederick Stratton PHONE _____

ADDRESS 1015 E Quarles Place SUBDIVISION NAME _____ LOT _____

ELECTRICAL CONTRACTOR Current Electric

NEW SERVICE

OVERHEAD ☐

UNDERGROUND ☒

REWIRED SERVICE

OVERHEAD TO UNDERGROUD ☐

OVERHEAD TO OVERHEAD ☐

UNDERGROUND TO UNDERGROUND ☐

PERMANENT SERVICE ☐

SIZE (AMPS) 200 (NUMBER OF METERS: CHANGE FROM ____ TO ____)

1 PHASE ☒

3 PHASE ☐

VOLTAGE 120/240

TEMPORARY SERVICE ☐

SIZE AMPS _____

1 PHASE ☐

3 PHASE ☐

VOLTAGE _____

OVERHEAD RESIDENTIAL REWIRE INFORMATION	YES	NO
PERMANENT CONNECTIONS HAVE BEEN MADE	☐	☐
PERMANENT CONNECTIONS REQUIRED	☐	☐
SERVICE DROP RELOCATION OR REPLACEMENT REQUIRED	☐	☐

RESIDENTIAL / FARM ☐ COMMERCIAL ☐

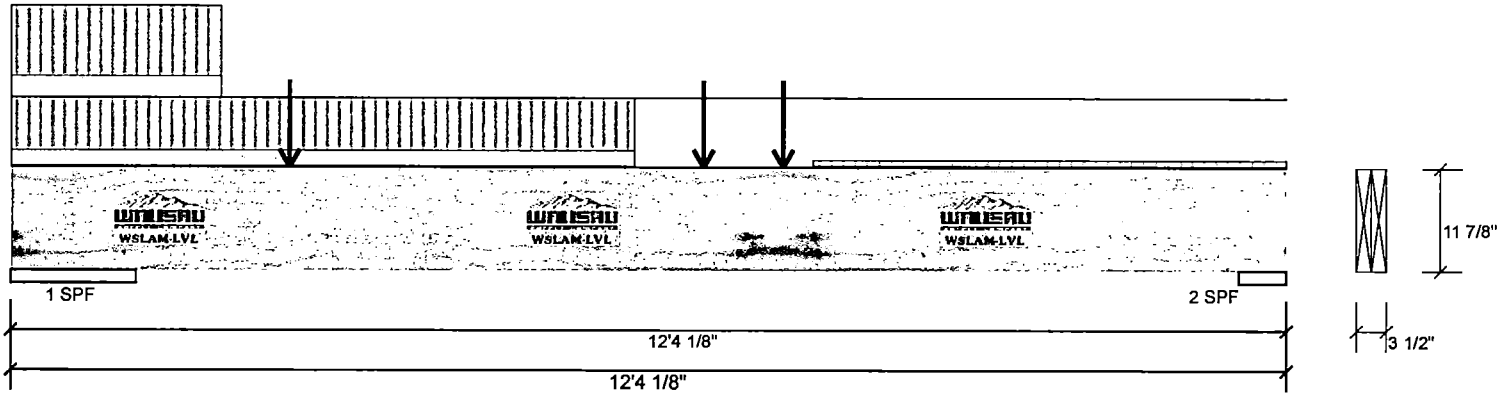
INSTALL ONLY ☐ NUMBERS OF METERS _____

	Client Butler chase		Shipping
Project Name: Butler Chase stratton	Job#: Stratton	Quantity 1 (2pcs.)	Description: second Floor

G2-A 2.0E WS-LAM LVL 1.750" X 11.875" 2-Ply - PASSED

10/16/2014 1:45 PM

Designer:



Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC 2012 / IRC
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	3/4" OSB Nailed and Glued
Importance:	Normal	Vibration:	Not Checked
Temperature:	Temp <= 100°F		

Reactions

Brg	Live	Dead	Snow	Wind	Const
1	4166	1372	0	0	0
2	2128	786	0	0	0

Bearings

Bearing	Input Length	In Analysis	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	14.563"	3.750"	99%	1372 / 4166	5538	L	D+L
2 - SPF	5.500"	2.000"	98%	786 / 2128	2914	L	D+L

Analysis	Actual	Location	Allowed	Capacity	Load Comb.	Ld. Case
Moment	12323 ft-lb	7'5 3/4"	21295 ft-lb	0.579 (58%)	D+L	L
Shear	3184 lb	2' 9/16"	7897 lb	0.403 (40%)	D+L	L
LL Defl inch	0.168 (L/781)	6'6 5/16"	0.364 (L/360)	0.460 (46%)	L	L
TL Defl inch	0.227 (L/576)	6'6 7/16"	0.546 (L/240)	0.420 (42%)	D+L	L

Design OK.

Design Notes

1 Girders are designed to be supported on the bottom edge only.
2 Multiple plies must be fastened together as per manufacturer's details.
3 Top loads must be supported equally by all plies.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Const.	Comments
1	Tie-In	0-0-0 to 1-4-9	(Span)0-1-12	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 1-4-9	(Span)0-1-12	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
3	Part. Uniform	0-0-0 to 6-0-9		Far Face	112 PLF	373 PLF	0 PLF	0 PLF	0 PLF	
4	Part. Uniform	0-0-0 to 2-0-9		Near Face	151 PLF	502 PLF	0 PLF	0 PLF	0 PLF	
5	Tie-In	1-4-9 to 2-8-9	(Span)0-1-12	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
6	Tie-In	1-4-9 to 12-4-2	(Span)0-1-12	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
7	Tie-In	2-8-9 to 4-0-9	(Span)0-1-12	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
8	Point	2-8-9		Near Face	68 lb	227 lb	0 lb	0 lb	0 lb	J5
9	Tie-In	4-0-9 to 5-4-9	(Span)0-1-12	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
10	Tie-In	5-4-9 to 7-9-4	(Span)0-1-12	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
11	Point	6-8-9		Far Face	107 lb	355 lb	0 lb	0 lb	0 lb	J5
12	Point	7-5-12		Far Face	808 lb	2194 lb	0 lb	0 lb	0 lb	G7

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

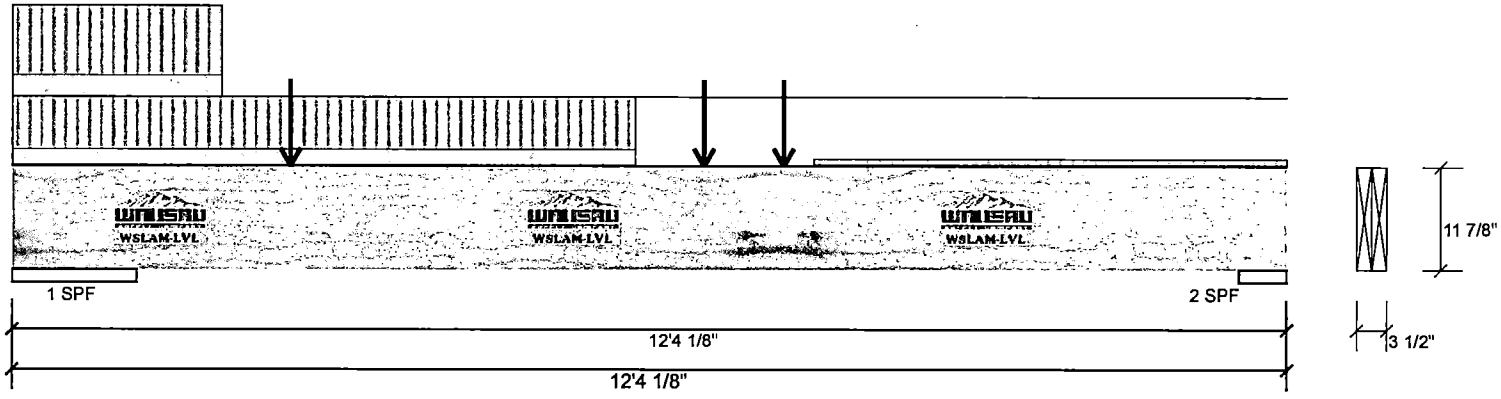
1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation
6. For flat roofs provide proper drainage to prevent ponding

Wisconsin Building Supply
W232 N 5700 HWY 74, Wi
US
53089

	Client Butler chase		Shipping
Project Name: Butler Chase stratton	Job#: Stratton	Quantity 1 (2pcs.)	Description: second Floor
G2-A 2.0E WS-LAM LVL 1.750" X 11.875" 2-Ply - PASSED			10/16/2014 1:45 PM Page 2 of 2 Designer:



...Continued from page 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Const.	Comments
13	Tie-In	7-9-4 to 12-4-2	(Span)1-11-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
	Self Weight				11 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

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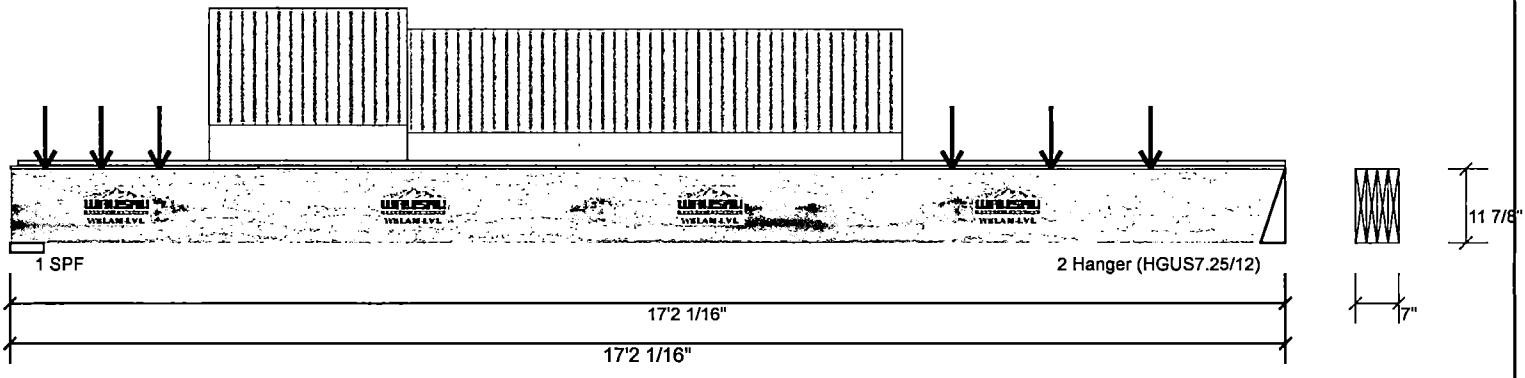
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation
6. For flat roofs provide proper drainage to prevent ponding

Wisconsin Building Supply
W232 N 5700 HWY 74, Wi
US
53089

Wisconsin BUILDING SUPPLY
La Ann Price
W232 N 5700 HWY 74
Sussex,WI 53089
262-246-0611

	Client Butler chase		Shipping
Project Name: Butler Chase stratton	Job#: Stratton	Quantity 1 (4pcs.)	Description: second Floor
G7-A 2.0E WS-LAM LVL 1.750" X 11.875" 4-Ply - PASSED			10/16/2014 1:46 PM Page 1 of 2 Designer:



Type:	Girder	Application:	Floor
Plies:	4	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC 2012 / IRC
Deflection LL:	360	Load Sharing:	Yes
Deflection TL:	240	Deck:	3/4" OSB Nailed and Glued
Importance:	Normal	Vibration:	Not Checked
Temperature:	Temp <= 100°F		

Reactions

Brg	Live	Dead	Snow	Wind	Const
1	3196	1193	0	0	0
2	2194	808	0	0	0

Bearings

Bearing	Input Length	In Analysis	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	1.500"	98%	1193 / 3196	4389	L	D+L
2 - Hanger	4.000"	1.500"	34%	808 / 2194	3002	L	D+L

Analysis	Actual	Location	Allowed	Capacity	Load Comb.	Ld. Case
Moment	16059 ft-lb	8'3 5/8"	44293 ft-lb	0.363 (36%)	D+L	L
Shear	3906 lb	1'4 5/8"	15794 lb	0.247 (25%)	D+L	L
LL Defl inch	0.284 (L/698)	8'6 1/2"	0.550 (L/360)	0.520 (52%)	L	L
TL Defl inch	0.381 (L/520)	8'6 1/2"	0.825 (L/240)	0.460 (46%)	D+L	L

Design OK.

Design Notes

1 Girders are designed to be supported on the bottom edge only.
2 Multiple plies must be fastened together as per manufacturer's details.
3 Top loads must be supported equally by all plies.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Const.	Comments
1	Tie-In	0-0-0 to 2-0-4	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
2	Tie-In	0-1-4 to 17-2-1	(Span)0-9-4	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
3	Point	0-5-12		Far Face	165 lb	235 lb	0 lb	0 lb	0 lb	G3
4	Point	1-2-14		Far Face	91 lb	303 lb	0 lb	0 lb	0 lb	J7
5	Tie-In	2-0-4 to 3-4-4	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
6	Point	2-0-4		Far Face	125 lb	415 lb	0 lb	0 lb	0 lb	J7
7	Part. Uniform	2-8-4 to 5-4-4		Far Face	118 PLF	392 PLF	0 PLF	0 PLF	0 PLF	
8	Tie-In	3-4-4 to 4-8-4	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
9	Tie-In	4-8-4 to 6-0-4	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
10	Part. Uniform	5-4-4 to 12-0-4		Far Face	93 PLF	348 PLF	0 PLF	0 PLF	0 PLF	
11	Tie-In	6-0-4 to 7-4-4	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
12	Tie-In	7-4-4 to 8-8-4	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

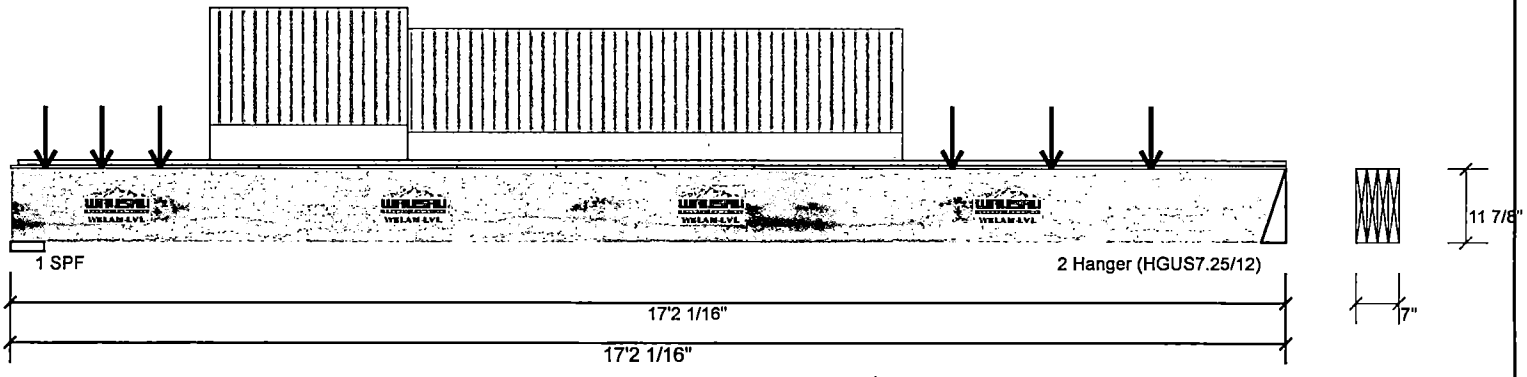
1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation
6. For flat roofs provide proper drainage to prevent ponding

Wisconsin Building Supply
W232 N 5700 HWY 74, Wi
US
53089

	Client Butler chase		Shipping
Project Name: Butler Chase stratton	Job#: Stratton	Quantity 1 (4pcs.)	Description: second Floor
G7-A 2.0E WS-LAM LVL 1.750" X 11.875" 4-Ply - PASSED			10/16/2014 1:46 PM Page 2 of 2 Designer:



...Continued from page 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Const.	Comments
13	Tie-In	8-8-4 to 10-0-4	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
14	Tie-In	10-0-4 to 11-4-4	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
15	Tie-In	11-4-4 to 12-8-4	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
16	Tie-In	12-8-4 to 17-2-1	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
17	Point	12-8-4		Far Face	122 lb	456 lb	0 lb	0 lb	0 lb	J9
18	Point	14-0-4		Far Face	34 lb	114 lb	0 lb	0 lb	0 lb	J2
19	Point	15-4-4		Far Face	64 lb	139 lb	0 lb	0 lb	0 lb	G8
	Self Weight				22 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation
6. For flat roofs provide proper drainage to prevent ponding

Wisconsin Building Supply
W232 N 5700 HWY 74, Wi
US
53089

La Ana Price
W232 N 5700 HWY 74
Sussex,WI 53089
262-246-8611

Wisconsin BUILDING SUPPLY

Plumber E&W Sewer & Water Const., Inc. No. 12897

Address P. O. Box 190

City, State, Zip Big Bend, WI 53103

Tel. No. (262) 662-3263

Application and Record

Village of Fox Point
7200 N. Santa Monica Blvd.
Fox Point, WI 53217
(414) 351-8900

Owner Wired Const

Address 2022 E. North Ave, SU 300, MKE, WI 53202

Date October, 14th, 20 13

TO THE VILLAGE OF FOX POINT, PLUMBING AND WATER INSPECTION DEPARTMENT:
The undersigned hereby make application to do the work of plumbing consisting of
laying a _____ inch _____ builder sewer from Main to Lot line to Building
laying a _____ inch _____ water service from Main to Lot line to Building

ABANDON/CAP 6" CLAY SANITARY SEWER LATERAL AT LOT LINE

1015 E. Quarles Place Fox Point, WI

Address at which work is to be done

Subdivision	Lot	Block

PERMITS USED	
Kind	No.
Sewer and Plumbing	12851
Water	
Street	
Meter	
Water Usage	

In the performance of this work the undersigned Plumber hereby agrees to bounden by and submit to all statutes, Village ordinances and rules and regulations prescribed by the Village Board for Plumbers.

State Master Lic.# 227764 Expir. 3/31/14 Building Contractor Reg# N/A Expir.

HVAC Contractor Reg# _____ Expir. _____ Signature _____ Applicant

FIXTURE WITH DRAIN OR WATER CONNECTIONS

	No.		No.
Hose Bibs		Dishwashers	
Bath Tubs		Wash Basins	
Sump Pumps		Water Closets	
Laundry Trays		Showers	
Drinking Fountains		Floor Drains	
Sinks		Food Waste Grinders	
Water Heaters		Sprinkling System	
Wash. Mach. Wastes		Urinals	
Bidets			
Catch Basins			

FEES

Water Usage		$
Building Sewer	CAP	$60.00
Water Service		
Building Drain		
Fixtures		
Water Meter		
Total		$60.00

Deposit to cover street repairs Receipt #45311

(A current certificate of insurance must be provided when doing work in road right of way (ROW)).

_____ Permit Clerk

A _____ inch _____ water service pipes laid in _____

Curb box is located _____ feet _____ of _____ feet _____ of _____

_____ inch _____ Water Meter No. _____ Date Installed _____

A _____ inch _____ sanitary sewer connection was made in _____

_____ feet _____ of manhole _____

A _____ inch _____ storm sewer connection was made in _____

_____ feet _____ of manhole _____

Building Sewer	Report	Building Drain	Report	Rough In Plumbing Inspection	Report	Final Inspection	Report
						10/23/13	

Installation Approved _____ Application Approved _____, 20____

As Built _____

_____
Water and Plumbing Inspector

Search for Individual or Company by Credential ID here:

Specific Credential ID 227764

Search

2 record(s) were returned by your search.

ID	Name	City,State,Zip	Credential Type	Expiration
227764	OVERLIEN, WAYNE A	BIG BEND WI 53103	Master Plumber-Restricted Service	03/31/15
227764	OVERLIEN, WAYNE A	BIG BEND WI 53103	Utility Contractor	03/31/16

Receipt No: 1.045311 Oct 21, 2013

1015 E QUARLES PLACE

LICENSES & PERMITS-PLUMBING PERMIT	60.00
24-44470 PLUMBING PERMIT	
Total:	60.00
CHECK Chk No: 9012	60.00
Total Applied:	60.00
Change Tendered:	.00

10/21/13 11:07am

VILLAGE OF FOX POINT
7200 N. SANTA MONICA BLVD
FOX POINT, WI 53217 414-351-8900

3009 N. Hackett Ave
Milwaukee, WI 53211

January 3, 2013

Mr. Scott Miller
Fox Point Building Inspector
Village of Fox Point
7200 N. Santa Monica Blvd.
Fox Point, WI 53217

Dear Mr. Miller,

On January 9, 2013 we will be purchasing the home on 1015 E. Quarles Place from Lolita Friedlen.

This letter is to inform you that, upon purchase of the property and dwelling, we will not occupy the house.

Our plan is to demolish the house and build a new structure.

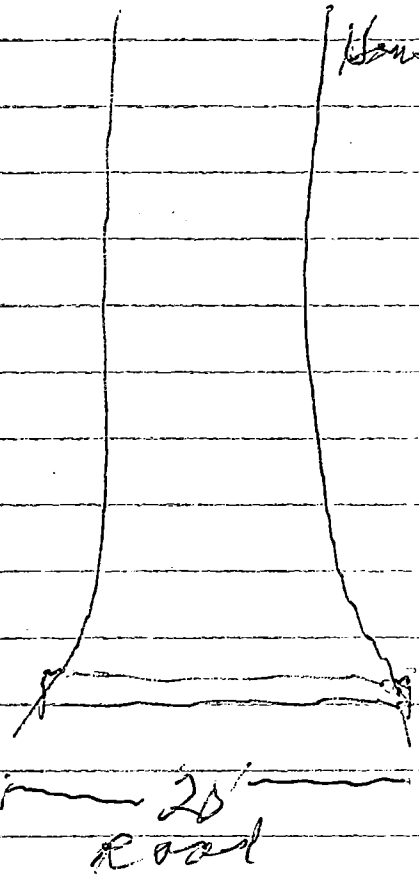
Thank you for your attention in this matter.

Sincerely,

Frederick Stratton III

Frank Armstrong Paving
8535 W. Keefe Ave
Milw 53225
ph 358-1613
(Paul cell 899-4124

Job site:
Lolita Friedler
1015 Bradley Place



Install arched culvert 20' x 12"

Receipt No: 1.003168 Jan 08, 2002

1015 E QUARLES PLACE

LICENSES & PERMITS-PLUMBING PERMIT	40.00
10-44470 PLUMBING PERMIT	
Total:	40.00
CHECK Chk No: 6589	40.00
Total Applied:	40.00
Change Tendered:	.00

01/08/02 11:20am

VILLAGE OF FOX POINT
7200 N. SANTA MONICA BLVD
FOX POINT, WI 53217

414-351-8900

Plumber Don's Inst. Service, Inc.
Address 1749 New Street, Union Grove, WI 53182
Tel. No. 878-2185

No. 795 C

Application and Record

Village of Fox Point
7200 N. Santa Monica Blvd.
Fox Point, WI 53217
351-8900

Owner HERBERT FRIEDLEN
Address 1015 E. QUARLES PL.
Date 12-75, 1975

TO the VILLAGE OF FOX POINT, PLUMBING AND WATER INSPECTION DEPARTMENT: The undersigned hereby make application to do the work of plumbing consisting of

laying a ..... inch ..... builder sewer from Main to Lot line to Building

laying a ..... inch ..... water service from Main to Lot line to Building

at

1015 E. QUARLES PL. ..... Fox Point, Wis.
Address at which work is to be done

PERMITS USED	
Kind	No.
Sewer and Plumbing	7889
Water	
Street	
Meter	
Water Usage	

Subdivision	Lot	Block

In the performance of this work the undersigned Plumber hereby agrees to bounden by and submit to all statutes, village ordinances, and rules and regulations prescribed by the Village Board for Plumbers.

License No. MPRA 3227 ..... Don Regnier ..... Plumber

Application must be signed by licensed plumber who has current insurance certificate on file in Fox Point.

FIXTURES WITH DRAIN OR WATER CONNECTIONS

	No.		No.
Hose Bibs		Dishwashers	
Bath Tubs		Wash Basins	
Sump Pumps		Water Closets	
Laundry Trays		Showers	
Drinking Fountains		Floor Drains	
Sinks		Food Waste Grinders	
Water Heaters	1	Sprinkling System	
Wash. Mach. Wastes		Urinals	
Bidets			
Catch Basins			

FEES

	$
Water Usage	
Building Sewer	
Water Service	
Building Drain	
Fixtures	
Water Meter	
Total	$40.00
Deposit to cover street repairs	Receipt #3435

..... Permit Clerk

A ..... inch ..... water service pipes laid in .....
Curb box is located ..... feet ..... of ..... feet ..... of .....
.....
..... inch ..... Water Meter No. ..... Date Installed .....

A ..... inch ..... sanitary sewer connection was made in .....
..... feet ..... of manhole .....
.....

A ..... inch ..... storm sewer connection was made in .....
..... feet ..... of manhole .....
.....

Building Sewer	Report	Building Drain	Report	Rough-in Plumbing Inspection	Report	Final Inspection	Report

Installation Approved ..... Application Approved ....., 19 .....

As Built ..... Water and Plumbing Inspector

REMARKS

.....
.....
.....

DISCREPANCY RECORD

.....
.....
.....

1 Story
762 Sq. Ft.

VILLAGE OF FOX POINT

MILWAUKEE COUNTY, WISCONSIN

No. 775

APPLICATION FOR BUILDING PERMIT

TO THE BUILDING INSPECTION DEPARTMENT:

The undersigned hereby applies for a permit to build, in accordance with the information tabulated hereafter,

Designation Residence and garage
(Duplex, Residence, Apartment, Store, Garage, Theatre)

House Number 1015 E. Quarles Place STREET

Lot 4 Block 1

Subdivision Fox Point Subd

District "A"

Does contemplated building violate the Village zoning ordinance? No

Height of Building 1 Story (stories or feet)

Width (parallel to highway) 77' 11" overall (feet)

Depth (perpendicular to highway) 75' 2" (feet)

Distance: Street Line to Front Line of Porch 30' (feet)

Type of Construction: Concrete block
(Frame, Brick-Tile)

Exterior finish Brick
(Stucco-Siding-Brick Veneer)

Height of front yard above street sidewalk grade 14"

Number of rooms 5 rooms, 2 baths

Estimated cost { Garage
Building $18,000.00

Is there a private garage? Yes

Does the contemplated garage violate the Village zoning ordinance? No

Size 16 X 27' 8" Number of stalls 1 stall

Where situated Attached to residence

General construction Concrete block and brick
(Frame—Brick—Stucco)

Have you applied to the Industrial Commission for a permit under the State Building Code?

Has the permit been granted?

Herewith are filed the following duplicate plans ____ in number, which I certify I will conform to in the work hereby applied for.

Remarks: Plans, plot plan, survey furnished.
Door between garage and residence to be metal clad and also frame

Herewith are filed the specifications that describe the work in question and as shown on plans above submitted.

In making the application the undersigned agrees to obey the Fox Point Building Code pertaining to the erection of buildings and also agrees to obey all other ordinances of the Village of Fox Point.

The undersigned, owner or being duly authorized so to do, hereby gives express authorization to the Village of Fox Point, its officers, agents and employees, to enter upon the premises herein described and fill up any excavation, or tear down, remove or enclose the unfinished structure for which a permit is herein requested in the event of cessation of the building, whenever the Building Inspector shall determine that such premises in the unfinished condition of the structure are dangerous to members of the public, including children, even though trespassers. The undersigned further hereby waives all statutory notices and consents to the determination by the Village Board and the levy and placing upon the tax roll of a special assessment in the amount of the cost to the Village, including customary Village overhead charges incurred in filling up any such excavation or tearing down, removing or enclosing any such unfinished structure.

Owner of land Royal Bethke Owner of building Royal Bethke

Permit fee $ 13.52 herewith tendered.
Water Area - 2600 sq. ft.

Signed R C Bethke

Dated, September 9, 19 47

Architect, Owner, Builder.

BUILDING PERMIT ISSUED 9-11-47

175

Rec # 29447
10/22/71

VILLAGE OF FOX POINT
MILWAUKEE COUNTY, WISCONSIN

No. 1923

APPLICATION FOR HEATING PERMIT

TO THE BUILDING INSPECTION DEPARTMENT:

The undersigned hereby applies for a permit to install, in accordance with the information tabulated hereafter,

Winter Air Conditioner ☐

Fuel: ☑ Gas ☐ Oil ☐ Coal ☐ Elect. : Other _____

Type: _____
Forced Air, Radiant, Baseboard, Etc.

Desc. of Heating Plant GAS FIRED HOT WATER BOILER

Vented to CHIMNEY

Fuel Tank ☐: _____ _____
Size Location

Summer Air Conditioner ☐

Size _____ (Ton, H.P.)

Coolant _____

Compressor Coolant: Air ☐; Water ☐;

If Water Cooled:

Source of Water _____

Discharged to _____

Remarks _____

The undersigned acknowledges that he is familiar with Ordinance No. 261, and all amendments thereto and that the work described herewith shall conform in all respects to said ordinances and all other ordinances of the Village of Fox Point, and laws of the State of Wisconsin. Any variations of the above may cause immediate revocation of the permit, if granted.

Owner HERB FRIEDLEN

Address of Work 1015 E QUARLES

Lot Block Subdivision

Contractor BACHMAN HEATING

Address 6931 W FOND DU LAC AVE. Phone 466-5070

Approved: James L Kalsh
Signed

10-22-71 10/22/71
Date

775

Rec # 29460
12/28/71

License No. 29 SHEET 2 — VILLAGE'S COPY Permit No. 7907

DEPARTMENT OF ELECTRICAL INSPECTION

Application for Permit for Electrical Installation

VILLAGE OF FOX POINT

TO THE ELECTRICAL INSPECTOR: Date October 27, 1971

The undersigned hereby applies for a permit for the execution of electrical installation for light, heat, or power, as hereafter prescribed:

Location 1015 East Quarles Place
(Give exact street and number. Do not give corner)

Elec. Contractor North Side Electric Co., Inc. Address 4020 West Kiehnau Avenue

Builder George Bachman Heating Address 4041 North 71st Street

Owner Mr. H. Friedlen Address 1015 East Quarles Place

What is occupancy of the building NEW ☐ OLD ☒ Residence

1.	Outlets		each	$ .20	
2.	Fixtures		each	.15	
3.	Fixtures — fluorescent, cold cathode, lumiline, mercury vapor		each lamp	.15	
4.	Audible or visual devices		per device	.20	
5.	Exhaust and ventilating fans and their control (below 1 H.P.)		each	.50	
6.	Built-in electric heaters; bathroom, nursery, etc.		each	2.00	
7.	Garbage Disposal		each	2.00	
8.	Dishwasher		each	2.00	
9.	Clothes dryer		each	2.00	
10.	Range or other receptacles over 150 volts		each	3.00	
11.	Water heater		each	3.00	
12.	Automatic heating equipment — gas, oil, coal	1	each	3.00	3.00
13.	Automatic water systems		each	2.00	
14.	Refrigerating, air conditioning, etc., machines		each	2.00	
15.	Strip lighting, plug in strip, trol-e-duct, etc.		per ft.	.05	
16.	Dimmers or Time Clocks		each	1.00	
17.	Vacuum and Inert-Gas tube sign		each transformer	1.00	
18.	Incandescent Signs, studded lights		per socket	.10	
19.	Arc and mercury lamps, spot and floodlights (mogul base)		each	.50	
20.	Motors, each horsepower or fraction thereof each motor		H.P.	.20	
21.	Generators, rectifiers, transformers, etc.		per K.W.	.15	
22.	Feeders or subfeeders No. 3 B & S gauge or larger		each	5.00	
23.	Raceways, wireways, busways, gutters		per ft.	.10	
24.	Electric heating devices (other than those listed above)		per K.W.	.50	
25.	Service equipment — 0-100 amps. new or overhauling		per disconnect	2.00	
	Service equipment — 100 amps. to 600 amps.		per disconnect	6.00	
	Service equipment — over 600 amps.		per disconnect	8.00	
26.	Temporary service, etc. (3 month period)			3.00	
27.	Motion picture, stereopticon and x-ray machines, etc.		each	3.00	
28.	Re-inspection after time limit on notice			2.00	
29.	Minimum fee for any permit requiring separate inspection		MINIMUM FEE	5.00	
30.	Double fee shall be charged for any work started before filing an application for a permit		FEES DOUBLE		
			TOTAL FEES		5.00

It is hereby agreed between the undersigned, as owner, his agent or servant and the Village of Fox Point that for and in consideration of the premises and of the permit for the execution of electrical installation, for light, heat or power as above described, to be issued and granted by the Electrical Inspector, that the work thereon will be done in accordance, with the description herein set forth in this statement, and it is further agreed to alter or install same in strict compliance with the Village of Fox Point Elec. Code and to obey any and all lawful orders of the Electrical Inspector of the Village of Fox Point, the Statutes of the State of Wisconsin and the rules and regulations issued by the Industrial Commission of Wisconsin under authority of the State Statutes.

REMARKS:

Date for Inspection	Date Approved
Roughing in	
Temp.	
Final November 1, 1971	

Signature John J Berst per sc.
(Supervising Electrician)

Address 4020 West Kiehnau Avenue

City Milwaukee

Zone 53209 Telephone 353 - 5580

Make check Payable to Treasurer, Village of Fox Point.

VILLAGE OF FOX POINT
MILWAUKEE COUNTY
WISCONSIN

VILLAGE HALL
7200 N. SANTA MONICA BLVD.
FOX POINT 53217
414-351-8900

June 19, 1991

Herbert Friedlen
1015 E. Quarles Place
Fox Point, Wisconsin 53217

Dear Mr. Friedlen:

Thank you for your letter of June 07. Our Department of Public Works last week patched an "embryonic" pot hole near your residence. We will repair the cracking edges when Quarles Place is repaved. That work is currently scheduled for 1994.

Sincerely,

Noreen R. Cook

Noreen R. Cook
Village Manager

HERBERT FRIEDLEN
1015 E. QUARLES PLACE
MILWAUKEE, WI 53217

June 7, 1991

Village Manager
Fox Point Village Hall
No. Santa Monica Ave.
Fox Point, WI 53217

Dear Sir:

I respectfully call to your attention that the asphalt on Quarles Pl. needs some attention. There is an embryo pot hole, and the edges are cracking off.

Very truly yours,

Herbert Friedlen

Plumber E&W Sewer & Water Const., Inc. No. 13099

Address P.O. Box 190

City, State, Zip Big Bend, WI 53103

Tel. No. (262) 662-3263

Application and Record

Village of Fox Point
7200 N. Santa Monica Blvd.
Fox Point, WI 53217
(414) 351-8900

Owner c/o Gregg Norman of Wired Const

Address 2022 E. North Ave, Su 300, MKE, WI 53202

Date July 8th, 20 14

TO THE VILLAGE OF FOX POINT, PLUMBING AND WATER INSPECTION DEPARTMENT:
The undersigned hereby make application to do the work of plumbing consisting of laying a 4 inch PVC builder sewer from Main to Lot line to Building

laying a 1.25 inch Poly water service from Main to Lot line to Building

at

1015 E. Quarels Place Fox Point, WI
Address at which work is to be done

PERMITS USED	
Kind	No.
Sewer and Plumbing	1752
Water	
Street	
Meter	
Water Usage	

Subdivision	Lot	Block
Fox Point Subdivision	4	1

In the performance of this work the undersigned Plumber hereby agrees to bounden by and submit to all statutes, Village ordinances and rules and regulations prescribed by the Village Board for Plumbers.

State Master Lic.# 227764 Expir. 3/31/15 Building Contractor Reg# _____ Expir. _____

HVAC Contractor Reg# _____ Expir. _____ Signature _____ Applicant

FIXTURE WITH DRAIN OR WATER CONNECTIONS

	No.		No.
Hose Bibs		Dishwashers	
Bath Tubs		Wash Basins	
Sump Pumps		Water Closets	
Laundry Trays		Showers	
Drinking Fountains		Floor Drains	
Sinks		Food Waste Grinders	
Water Heaters		Sprinkling System	
Wash. Mach. Wastes		Urinals	
Bidets			
Catch Basins			

FEES

Water Usage		$
Building Sewer	1	$50.00
Water Service	1 (2" & under)	$50.00
Building Drain		
Fixtures		
Water Meter		
Total		$100.00

Deposit to cover street repairs Receipt #47785

(A current certificate of insurance must be provided when doing work in road right of way (ROW)).

Permit Clerk

A _____ inch _____ water service pipes laid in _____

Curb box is located _____ feet _____ of _____ feet _____ of _____

_____ inch _____ Water Meter No. _____ Date Installed _____

A _____ inch _____ sanitary sewer connection was made in _____

_____ feet _____ of manhole _____

A _____ inch _____ storm sewer connection was made in _____

_____ feet _____ of manhole _____

Building Sewer	Report	Building Drain	Report	Rough In Plumbing Inspection	Report	Final Inspection	Report

Installation Approved _____ Application Approved _____, 20____

As Built _____

Water and Plumbing Inspector

Search for Individual or Company by Credential ID here:

Specific Credential ID 227764

Search

2 record(s) were returned by your search.

ID	Name	City,State,Zip	Credential Type	Expiration
227764	OVERLIEN, WAYNE A	BIG BEND WI 53103	Master Plumber-Restricted Service	03/31/15
227764	OVERLIEN, WAYNE A	BIG BEND WI 53103	Utility Contractor	03/31/16

Receipt No: 1.047785 Jul 17, 2014

1015 E QUARLES PLACE

LICENSES & PERMITS-PLUMBING PERMIT	100.00
24-44470 PLUMBING PERMIT	
Total:	100.00

CHECK	Chk No: 9513	100.00
Total Applied:		100.00
Change Tendered:		.00

07/17/14 09:31am

VILLAGE OF FOX POINT
7200 N. SANTA MONICA BLVD
FOX POINT, WI 53217 414-351-8900

Village of Fox Point
7200 N. Santa Monica Blvd.
Fox Point, WI 53217
(414) 351-8900

Date Submitted _____ No. 15575

APPLICATION FOR BUILDING

The undersigned hereby applies for a permit to build, in accordance with the information tabulated hereafter,

Type of Project RAZING OF RESIDENTIAL STRUCTURE Address 1015 E QUARLES PLACE
Residence, Garage, Store, Office, School, Fence, Shed, Sign, Swimming Pool, Underground Storage Tank, Etc.

Lot _____ Block _____ Subdivision _____ District _____
Does contemplated structure violate the Village zoning ordinance? _____
Height of Structure _____ (stories or feet)
Width (parallel to highway) _____ (feet) Depth (perpendicular to highway) _____ (feet)
Distance: Street Line to Front Line of Structure _____ (feet)
Distance: Side Lot Line to Structure _____
Type of Construction: _____ Exterior finish _____
Frame, Brick-tile, etc. Stucco, Siding, Brick Veneer, Etc.

Height of front yard above street grade _____
Number of rooms _____ Baths _____
Estimated cost Garage _____
Building _____
Structure _____
Is there a private garage? _____
Does the contemplated garage violate the Village zoning ordinance? _____
Size _____ Number of Stalls _____ Where Situated _____
Have plans been submitted to the Wisconsin Department of Industry, Labor and Human Relations for examination and approval? _____
Have plans been approved as being in compliance with all applicable sections of the Wisconsin Administrative code? _____
Herewith are filed the following duplicate plans _____ in number, which I certify I will conform to in the work hereby applied for:
Remarks: BUILDING SIZE: MAIN LIVING AREA + GARAGE = 18,528 CU FT
ROOF AREA = 10,093 CU FT
TOTAL AREA = 28,621 CU FT

Herewith are filed the specifications that describe the work in question and as shown on plans above submitted.

In making the application the undersigned agrees to obey the Fox Point Building and Zoning Codes pertaining to the erection of all structures and also agrees to obey all other ordinances of the Village of Fox Point.

The undersigned, owner or being duly authorized so to do, hereby gives express authorization to the Village of Fox Point, its officers, agents and employees, to enter upon the premises herein described and fill up any excavation, or tear down, remove or enclose the unfinished structure for which a permit is herein requested in the event of cessation of the building, whenever the Building Inspector shall determine that such premises in the unfinished condition of the structure are dangerous to members of the public, including children, even though trespassers. The undersigned further hereby waives all statutory notices and consents to the determination by the Village Board and the levy and placing upon the tax roll of a special assessment in the amount of the cost to the Village, including customary Village overhead charges incurred in filling up any such excavation or tearing down, removing or enclosing any such unfinished structure.

We hereby agree to provide a house number plate or sign readily observable from the public highway which will be installed not less than 15 days after the structure is occupied.

Owner of Structure RICK STRATTON & KORI SAMARTIN
Address 3009 N HACKETT AVE
City MILWAUKEE State WI Zip 53211
Phone 414-534-6695

Arch. or Contr. WIRED CONSTRUCTION MANAGEMENT
Address 2022 E NORTH AVE, SUITE 300
City MILWAUKEE State WI Zip 53202
Phone 414-915-7493 (GREG NORMAN)

Size of Structure _____ (sq. ft.) Permit Fee $1130 Receipt 45327

Dwelling Contractor Certification No. 1269359 Expires 9/25/14
Dwelling Contractor Qualifier Certification No. 1003500 Expires 3/14/15
Building Contractor Certification No. _____ Applicant Signature _____
Architect, Owner, Builder

Date of Approved 10/22/13
Builder Inspector

08/14/2013

we energies

Central Group
W240 N2989 Pewaukee Rd.
Pewaukee, WI 53072
Phone: 262-574-6400
Toll-free: 866-423-0364

Rick Stratton
3009 Hackett Ave
Milwaukee, WI 53211

UTILITY DISCONNECTION VERIFICATION

Re: 1015 E Quarles Pl
Fox Point, WI

Electric Service

I hereby certify that the electric utilities which are the responsibility of We Energies located at the above address were disconnected on: 08/13/2013

Signature: Annette Bennett

Expediting Clerk
Phone 262-574-6452; Fax 262-574-6401

Note: Gas utility disconnections are handled separately; contact your gas utility supplier to arrange for disconnection. For verification of We Energies gas service disconnection, please contact me at the phone number listed above.

09/26/2013

we energies

W140 N9100 Lilly Rd.
Menomonee Falls, WI 53051
www.we-energies.com

RICK STRATTON
3009 HACKETT AVE
MILWAUKEE, WI 53211

UTILITY DISCONNECTION VERIFICATION

Re: 1015 E QUARLES PL
FOX POINT, WI 53217

Natural Gas Service

I hereby certify that the natural gas utilities which are the responsibility of We Energies located at the above address were disconnected on: 09/18/2013

Signature: _____

Residential Energy Service Consultant
Phone 262-502-6856; Fax 262-502-6875

Note: Electric utility disconnections are handled separately; contact your electric utility supplier to arrange for disconnection. For verification of We Energies electric service disconnection, please contact me at the phone number listed above.

Plumber Gene Tess Watertight Plumbing Company, Inc.
Address 24707 W. Loomis Rd. Unit 5 Wind Lake, Wisconsin 53185
Tel. No. (262) 895-4573

No. 9966

Application and Record

Village of Fox Point
7200 N. Santa Monica Blvd.
Fox Point, WI 53217
351-8900

Owner L. Friedlen 362-6222
Address 1015 E. Quarles PL.
Date 12/21/01 19 ....

TO the VILLAGE OF FOX POINT, PLUMBING AND WATER INSPECTION DEPARTMENT: The undersigned hereby make application to do the work of plumbing consisting of

laying a .....inch ..... builder sewer from Main to Lot line to Building

laying a .....inch ..... water service from Main to Lot line to Building

at

.....Fox Point, Wis.
Address at which work is to be done

PERMITS USED	
Kind	No.
Sewer and Plumbing	9966
Water	
Street	
Meter	
Water Usage	

Subdivision	Lot	Block

In the performance of this work the undersigned Plumber hereby agrees to bounden by and submit to all statutes, village ordinances, and rules and regulations prescribed by the Village Board for Plumbers.

License No. 222395 Gene Tess .....Plumber

Application must be signed by licensed plumber who has current insurance certificate on file in Fox Point.

FIXTURES WITH DRAIN OR WATER CONNECTIONS

	No.		No.
Hose Bibs		Dishwashers	
Bath Tubs		Wash Basins	
Sump Pumps		Water Closets	
Laundry Trays		Showers	
Drinking Fountains		Floor Drains	
Sinks		Food Waste Grinders	
Water Heaters	1	Sprinkling System	
Wash. Mach. Wastes		Urinals	
Bidets			
Catch Basins			

FEES

	$
Water Usage	
Building Sewer	
Water Service	
Building Drain	
Fixtures	
Water Meter	
Total	#40.00
Deposit to cover street repairs	Receipt #3168

40.00

.....Permit Clerk

A .....inch .....water service pipes laid in .....

Curb box is located .....feet .....of .....feet .....of .....

.....

.....inch .....Water Meter No. .....Date Installed .....

A .....inch.....sanitary sewer connection was made in.....

.....feet .....of manhole .....

.....

A .....inch.....storm sewer connection was made in .....

.....feet.....of manhole .....

.....

Building Sewer	Report	Building Drain	Report	Rough in Plumbing Inspection	Report	Final Inspection	Report

Installation Approved .....Application Approved....., 19 .....

As Built ....
Water and Plumbing Inspector

REMARKS

.....

.....

.....

DISCREPANCY RECORD

.....

.....

.....

STRUCTURAL CALCULATIONS

FEB. 27, 2013
PROJECT NO. 13-129

PROJECT:

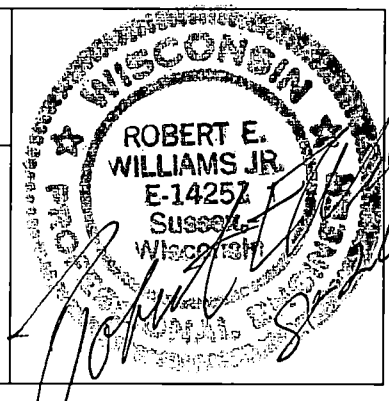
The Stratton Residence
A CUSTOM DESIGNED HOME
1015 EAST QUARLES PLACE
FOX POINT, WI

ARCHITECT:

JOY PEOT-SHIELDS

ENGINEER:

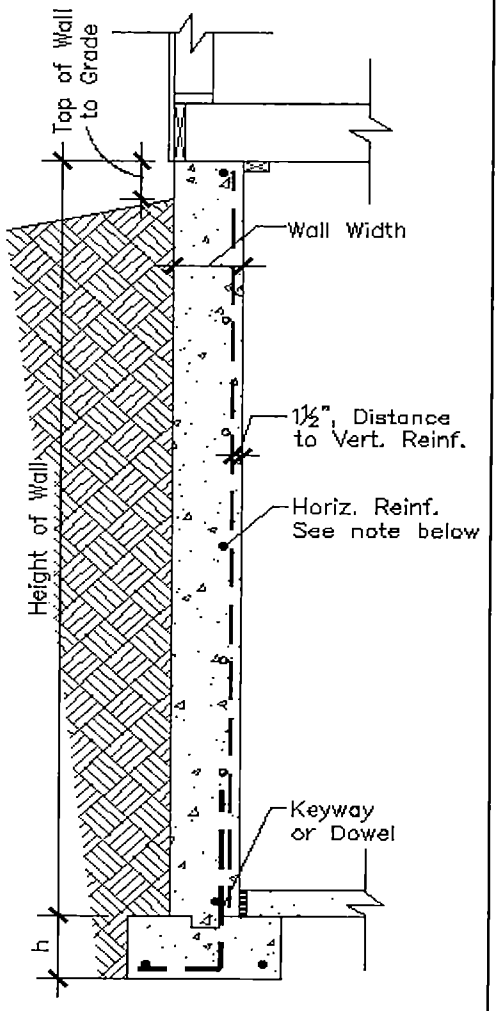
TDI ASSOCIATES, INC.
ARCHITECTS, ENGINEERS, PLANNERS
NB W22350 JOHNSON DRIVE
SUITE B4
WAUKESHA, WISCONSIN 53186
(262) 409-2530 OFFICE
(262) 409-2531 FAX



Concrete Foundation Wall- Resist Soil Pressure

If Unreinforced Wall works for Loading, Reinforcement may be added without satisfying Temperature & Shrinkage

Thickness of Foundation Wall (Width)	W (in)	10
Height of Wall	L (ft)	9.0
Top of Wall to Grade	l (inches)	8
Thickness of Basement Slab	t_{slab} (inches)	3



Equivalent Lateral Fluid Pressure		
Horizontal Pressure from Soil	psf	60
Hydrostatic Pressure	psf	15
Total Equiv. Pressure (unfactored)	psf	75
Unbraced Length	ft	8.21

Horizontal Force and Moment Produced from Equiv. Pressure		
Compounded Press. At Wall Bot.	psf	615.625
Horizontal Force (1/3) from Bot.	Pound-force	2526.6
Unfactored Moment	M (#-ft)	2654.64
Factored Moment (1.2DL+1.6LL)	M_u (#-ft)	**3397.94**

Check Wall Design- Unreinforced		
Section Modulus of Wall Section	S (in^3)	200
Concrete Compressive Strength	Fc (psi)	3,000
$\varphi Mn = \varphi\ 5\ (F_c)^{1/2}\ S\ (1'/12")$	φMn (#*ft)	4107.9
Check $\varphi Mn > M_u$	**Unreinforced Wall Works**	

Vertical Reinforcement Required (for Bending)		
Factored Moment (1.2DL+1.6LL)	M_u (#-ft)	3397.94
Depth to Comp. Line (minus cover)	**d (in)**	8.19
Bar Designation	__/8 = inches	**5**
Allowed Steel Stress	f_y (psi)	60,000
Bar Development at Critical Bending	# bar vs. 1/3L	**O.K.**
As- required (bending vs. Asmin, if Req'd)	in^2	0.0933
Spacing of Bars- Maximum	inches	39.89
Spacing of Bars- Chosen (less than Max)	inches	**36**
$\varphi Mn = \varphi$ Asfy/12 * [d- (Asfy/(1.7*b*f'c)]	φMn (#*ft)	3760.08
Check $\varphi Mn > M_u$		**O.K.**

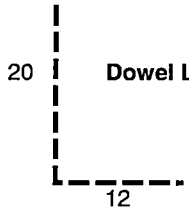
Check Beam Shear at Bottom Support		
Force at Bottom of wall (at support)	P (lbs)	1684.4
V_u =[(1.2DL+1.6LL)/DL+LL]*P	V_u (lbs)	2156.1
$\varphi V_c = \varphi$*2*(f'c)^½*l*d	φV_c (lbs)	9148.3
Check $\varphi V_c > V_u$		**O.K.**

Design width of Keyway or Dowel required		
Keyway width (minimum) = (V_u * d)/ φV_c	w (inches)	1.930
Bar Designation	__/8 = inches	**5**
Spacing of Bars- Chosen	inches	**36**
Depth of Footing	h (inches)	12.0
Strength of Rebar (see lengths, right)	φV_n (lbs)	2241.3
Check $\varphi V_n > V_u$		**O.K.**

Horizontal Reinforcement

Reinforcing Per ACI 7.12.2 is Not Req'd

Use # 5 bar at Top, Middle, & Bottom

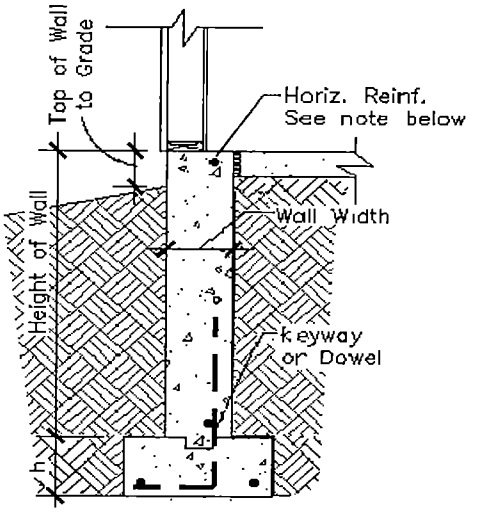


TDI Associates, Inc. Job: Stratton Residence-Fox Point
Architects • Engineers • Planners Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

Concrete Foundation Wall- Resist Soil Pressure

If Unreinforced Wall works for Loading, Reinforcement may be added without satisfying Temperature & Shrinkage

Thickness of Foundation Wall (Width)	W (in)	8
Height of Wall	L (ft)	4.0
Top of Wall to Grade	l (inches)	6
Thickness of Floor Slab	t_{slab} (inches)	4



Equivalent Lateral Fluid Pressure		
Net Horizontal Pressure from Soil	psf	1
Net Hydrostatic Pressure	psf	175
Total Equiv. Pressure (unfactored)	psf	176
Unbraced Length	ft	3.33

Horizontal Force and Moment Produced from Equiv. Pressure		
Compounded Press. At Wall Bot.	psf	586.6666667
Horizontal Force (1/3) from Bot.	Pound-force	977.8
Unfactored Moment	M (#-ft)	417.19
Factored Moment (1.2DL+1.6LL)	M_u (#-ft)	**666.55**

Check Wall Design- Unreinforced		
Section Modulus of Wall Section	S (in^3)	128
Concrete Compressive Strength	Fc (psi)	3,000
$\varphi Mn = \varphi\ 5\ (F_c)^{1/2}\ S\ (1'/12")$	φMn (#*ft)	2629.1
Check $\varphi Mn > M_u$	**Unreinforced Wall Works**	

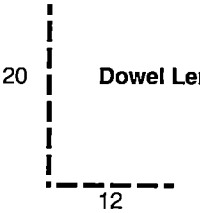
Vertical Reinforcement Required (for Bending)		
Factored Moment (1.2DL+1.6LL)	M_u (#-ft)	666.55
Depth to Comp. Line (minus cover)	**d (in)**	6.25
Bar Designation	__/8 = inches	**4**
Allowed Steel Stress	f_y (psi)	60,000
As- required (bending vs. Asmin, if Req'd)	in^2	0.0238
Spacing of Bars- Maximum	inches	100.89
Spacing of Bars- Chosen (less than Max)	inches	**96**
$\varphi Mn = \varphi$ Asfy/12 * [d- (Asfy/(1.7*b*f'c)]	φMn (#*ft)	700.37
Check $\varphi Mn > M_u$		**O.K.**

Check Beam Shear at Bottom Support		
Force at Bottom of wall (at support)	P (lbs)	651.9
V_u =[(1.2DL+1.6LL)/DL+LL]*P	V_u (lbs)	1041.5
$\varphi V_c = \varphi$*2*(f'c)^1/2*l*d	φV_c (lbs)	6983.5
Check $\varphi V_c > V_u$		**O.K.**

Design width of Keyway or Dowel required		
Keyway width (minimum) = $(V_u * d)/ \varphi V_c$	w (inches)	0.932
Bar Designation	__/8 = inches	**4**
Spacing of Bars- Chosen	inches	**48**
Depth of Footing	h (inches)	12.0
Strength of Rebar (see lengths, right)	φV_n (lbs)	1355.6
Check $\varphi V_n > V_u$		**O.K.**

Horizontal Reinforcement

Reinforcing Per ACI 7.12.2 is Not Req'd
Use # 5 bar at Top & Bottom



TDI Associates, Inc.
Architects • Engineers • Planners

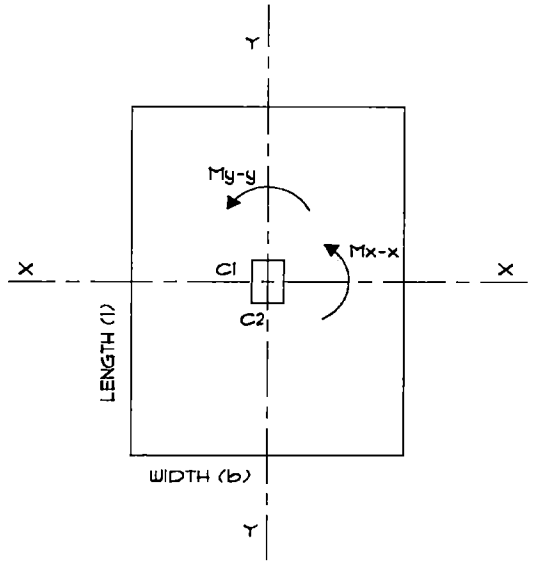
Job: Stratton Residence-Fox Point
Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

F-1 Footing Design- Point Load & Moment

Knowns	worst case	
Vertical Load- combined	P (k)	30
Moment (x-x) dead	M_{xd} (k-ft)	0
Moment (x-x) live	M_{xl} (k-ft)	0
Moment (y-y) dead	M_{yd} (k-ft)	0
Moment (y-y) live	M_{yl} (k-ft)	0
Column dim- l	c1 (in)	4
Column dim- b	c2 (in)	4
Conc.C stress	f'_c (ksi)	3
Allowed Steel stress	f_y (ksi)	60
Soil Pressure	q_a (ksf)	3

Factors (1.2DL + 1.6LL), use P*1.49		
Vertical Load- Ult.	P_u (k)	44.7
Moment (x-x) -Ult.	M_{ux} (k-ft)	0
Moment (y-y) -Ult.	M_{uy} (k-ft)	0

Dimensions			
Estimate A	~A (ft^2)	12.00	
($1.2* P/q_a$)			
Length	l (ft)	4.5	
Width	b (ft)	2.5	
Area	A (ft^2)	11.25	
Section Mod. (x-x)	S_x (ft^3)	8.44	
Section Mod. (y-y)	S_y (ft^3)	4.69	
Soil P (max)	q_{max} (ksi)	2.67	
Soil P (min)	q_{min} (ksi)	2.67	**O.K.**
Factored Soil P (max)	q_{umax} (ksi)	3.97	
Factored Soil P (min)	qu_{min} (ksi)	3.97	
Depth	h (in)	12	
Comp. Line	**d (in)**	8.0625	
Conc. Volume	ft^3	11.25	



Loads applied at center of footing- Using ACI-318-02

Beam Shear- length			
b (base) = width	inches	30	
Edge Dist = 1/2l-.5c1-d	inches	16.94	
V_u = qu*(area to edge)	kips	14.02	
$V_n = V_u/.85$	kips	16.49	
$V_c = 2*(f'_c)^{1/2}*b*d$	kips	26.50	**O.K.**

Beam Shear- width			
b (base) = length	inches	54	
Edge Dist = 1/2b-.5c1-d	inches	4.94	
V_u = qu*(area to edge)	kips	7.36	
$V_n = V_u/.85$	kips	8.66	
$V_c = 2*(f'_c)^{1/2}*l*d$	kips	47.69	**O.K.**

Punching Shear			
b_o	inches	48.25	
A_o	sq. in.	145.50	
$V_u = qu*((l*b)-A_o)$	kips	40.69	
$V_n = V_u/.85$	kips	47.86	
$V_c = 4*(f'_c)^{1/2}b_o*d$	kips	85.23	**O.K.**

Bending- A_s Req-d			
Edge Dist Length= 1/2l-.5c1	feet	2.08	
Edge Dist Width= 1/2b-.5c2	feet	1.08	
M_{ux} =qu*area*1/2 dist.***1.33**	k-ft	28.67	
As- Long direction (bending)	in^2	0.82	
Min As (temp. & shrinkage)	in^2	0.65	
As Required- long direction	in^2	**0.82**	
M_{uy} =qu*area*1/2 dist.***1.33**	k-ft	13.95	
As -short direction (bending)	in^2	0.39	
Min As (temp. & shrinkage)	in^2	1.17	
As Required- short direction	in^2	**1.17**	

Choose Bars and Spacing- Long Direction			
Bar designation		5	**O.K.**
# of bar layers (top & bottom?)		1	
Development Length (inches)	in	27.39	<Edge Dist.
# of bars required		3.0	
Total area of bars	in^2	0.93	
Spacing Distance (inches)	in	13.50	**O.K.**
φMn of the design (φ = .9)	k-ft	**32.22**	

Choose Bars and Spacing- Short Direction			
Bar designation		5	**O.K.**
# of bar layers (top & bottom?)		1	
Development Length	in.	27.39	<Edge Dist.
# of bars required		4.0	
Total area of bars	in^2	1.24	
Spacing Distance	in	17.00	**O.K.**
φMn of the design (φ = .9)	k-ft	**43.48**	

TDI Associates, Inc.
Architects • Engineers • Planners

Job: Stratton Residence-Fox Point
Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

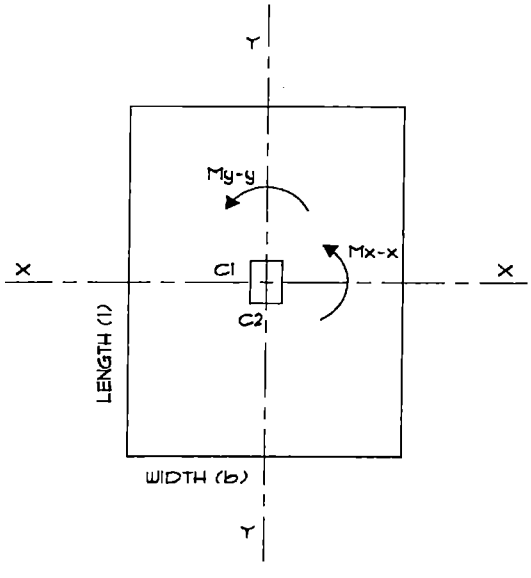
F-2 Footing Design- Point Load & Moment

Loads applied at center of footing- Using ACI-318-02

Knowns		worst case
Vertical Load- combined	P (k)	25
Moment (x-x) dead	M_{xd} (k-ft)	0
Moment (x-x) live	M_{xl} (k-ft)	0
Moment (y-y) dead	M_{yd} (k-ft)	0
Moment (y-y) live	M_{yl} (k-ft)	0
Column dim- l	c1 (in)	4
Column dim- b	c2 (in)	4
Conc.C stress	f'_c (ksi)	3
Allowed Steel stress	f_y (ksi)	60
Soil Pressure	q_a (ksf)	3

Factors (1.2DL + 1.6LL), use P*1.49		
Vertical Load- Ult.	P_u (k)	37.25
Moment (x-x) -Ult.	M_{ux} (k-ft)	0
Moment (y-y) -Ult.	M_{uy} (k-ft)	0

Dimensions			
Estimate A	~A (ft^2)	10.00	
(1.2* P/q_a)			
Length	l (ft)	3	
Width	b (ft)	3	
Area	A (ft^2)	9	
Section Mod. (x-x)	S_x (ft^3)	4.50	
Section Mod. (y-y)	S_y (ft^3)	4.50	
Soil P (max)	q_{max} (ksi)	2.78	
Soil P (min)	q_{min} (ksi)	2.78	O.K.
Factored Soil P (max)	q_{umax} (ksi)	4.14	
Factored Soil P (min)	qu_{min} (ksi)	4.14	
Depth	h (in)	12	
Comp. Line	**d (in)**	8.0625	
Conc. Volume	ft^3	9	



Beam Shear- length			
b (base) = width	inches	36	
Edge Dist = 1/2l-.5c1-d	inches	7.94	
V_u = qu*(area to edge)	kips	8.21	
$V_n = V_u/.85$	kips	9.66	
$V_c = 2*(f'_c)^{1/2}*b*d$	kips	31.80	O.K.

Beam Shear- width			
b (base) = length	inches	36	
Edge Dist = 1/2b-.5c1-d	inches	7.94	
V_u = qu*(area to edge)	kips	8.21	
$V_n = V_u/.85$	kips	9.66	
$V_c = 2*(f'_c)^{1/2}*l*d$	kips	31.80	O.K.

Punching Shear			
b_o	inches	48.25	
A_o	sq. in.	145.50	
$V_u = qu*((l*b)-A_o)$	kips	33.07	
$V_n = V_u/.85$	kips	38.90	
$V_c = 4*(f'_c)^{1/2}b_o*d$	kips	85.23	O.K.

Bending- A_s Req-d			
Edge Dist Length= 1/2l-.5c1	feet	1.33	
Edge Dist Width= 1/2b-.5c2	feet	1.33	
M_{ux} =qu*area*1/2 dist.***1.33**	k-ft	14.68	
As- Long direction (bending)	in^2	0.41	
Min As (temp. & shrinkage)	in^2	0.78	
As Required- long direction	in^2	**0.78**	
M_{uy} =qu*area*1/2 dist.***1.33**	k-ft	14.68	
As -short direction (bending)	in^2	0.41	
Min As (temp. & shrinkage)	in^2	0.78	
As Required- short direction	in^2	**0.78**	

Choose Bars and Spacing- Long Direction			
Bar designation		5	O.K.
# of bar layers (top & bottom?)		1	
Development Length (inches)	in	27.39	<Edge Dist.
# of bars required		3.0	
Total area of bars	in^2	0.93	
Spacing Distance (inches)	in	16.50	O.K.
φMn of the design (φ = .9)	k-ft	**32.47**	

Choose Bars and Spacing- Short Direction			
Bar designation		5	O.K.
# of bar layers (top & bottom?)		1	
Development Length	in.	27.39	<Edge Dist.
# of bars required		3.0	
Total area of bars	in^2	0.93	
Spacing Distance	in	16.50	O.K.
φMn of the design (φ = .9)	k-ft	**32.47**	

TDI Associates, Inc.
Architects • Engineers • Planners

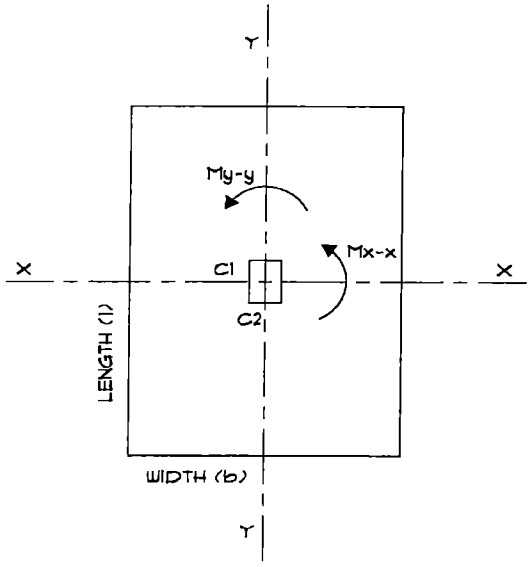
Job: Stratton Residence-Fox Point
Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

F-3 Footing Design- Point Load & Moment

Knowns	worst case	
Vertical Load- combined	P (k)	31
Moment (x-x) dead	M_{xd} (k-ft)	0
Moment (x-x) live	M_{xl} (k-ft)	0
Moment (y-y) dead	M_{yd} (k-ft)	0
Moment (y-y) live	M_{yl} (k-ft)	0
Column dim- l	c1 (in)	4
Column dim- b	c2 (in)	4
Conc.C stress	f'_c (ksi)	3
Allowed Steel stress	f_y (ksi)	60
Soil Pressure	q_a (ksf)	3

Factors (1.2DL + 1.6LL), use P*1.49		
Vertical Load- Ult.	P_u (k)	46.19
Moment (x-x) -Ult.	M_{ux} (k-ft)	0
Moment (y-y) -Ult.	M_{uy} (k-ft)	0

Dimensions			
Estimate A	~A (ft^2)	12.40	
(1.2* P/q_a)			
Length	l (ft)	3.5	
Width	b (ft)	3.5	
Area	A (ft^2)	12.25	
Section Mod. (x-x)	S_x (ft^3)	7.15	
Section Mod. (y-y)	S_y (ft^3)	7.15	
Soil P (max)	q_{max} (ksi)	2.53	
Soil P (min)	q_{min} (ksi)	2.53	O.K.
Factored Soil P (max)	q_{umax} (ksi)	3.77	
Factored Soil P (min)	qu_{min} (ksi)	3.77	
Depth	h (in)	12	
Comp. Line	**d (in)**	8.0625	
Conc. Volume	ft^3	12.25	



Loads applied at center of footing- Using ACI-318-02

Beam Shear- length			
b (base) = width	inches	42	
Edge Dist = 1/2l-.5c1-d	inches	10.94	
V_u = qu*(area to edge)	kips	12.03	
$V_n = V_u/.85$	kips	14.15	
$V_c = 2*(f'_c)^{1/2}*b*d$	kips	37.09	O.K.

Beam Shear- width			
b (base) = length	inches	42	
Edge Dist = 1/2b-.5c1-d	inches	10.94	
V_u = qu*(area to edge)	kips	12.03	
$V_n = V_u/.85$	kips	14.15	
$V_c = 2*(f'_c)^{1/2}*l*d$	kips	37.09	O.K.

Punching Shear			
b_o	inches	48.25	
A_o	sq. in.	145.50	
$V_u = qu*((l*b)-A_o)$	kips	42.38	
$V_n = V_u/.85$	kips	49.86	
$V_c = 4*(f'_c)^{1/2}b_o*d$	kips	85.23	O.K.

Bending- A_s Req-d			
Edge Dist Length= 1/2l-.5c1	feet	1.58	
Edge Dist Width= 1/2b-.5c2	feet	1.58	
M_{ux} =qu*area*1/2 dist.***1.33**	k-ft	22.00	
As- Long direction (bending)	in^2	0.62	
Min As (temp. & shrinkage)	in^2	0.91	
As Required- long direction	in^2	**0.91**	
M_{uy} =qu*area*1/2 dist.***1.33**	k-ft	22.00	
As -short direction (bending)	in^2	0.62	
Min As (temp. & shrinkage)	in^2	0.91	
As Required- short direction	in^2	**0.91**	

Choose Bars and Spacing- Long Direction			
Bar designation		5	O.K.
# of bar layers (top & bottom?)		1	
Development Length (inches)	in	27.39	<Edge Dist.
# of bars required		3.0	
Total area of bars	in^2	0.93	
Spacing Distance (inches)	in	18.00	O.K.
φMn of the design (φ = .9)	k-ft	**32.65**	

Choose Bars and Spacing- Short Direction			
Bar designation		5	O.K.
# of bar layers (top & bottom?)		1	
Development Length	in.	27.39	<Edge Dist.
# of bars required		3.0	
Total area of bars	in^2	0.93	
Spacing Distance	in	18.00	O.K.
φMn of the design (φ = .9)	k-ft	**32.65**	

III. Support Structure

C. Spread Footings at house

1. Worse case exterior wall load

P_{max} = 2000.0 #, Wall, Floor & Roof Loads
P_{wall} = 1125 #, wall weight
P_{total} = 3125.0 # on Footing

Wall Properties

Height: 9.00 ft., Concrete Wall
Width: 10 in., Concrete Wall

pf_{allow} = 3000 psf, Allowable Soil Pressure
F'c = 3000 psi, Conc. Compr. Strength
As_{req} = 0.346 in2, (0.0018*b*h) for T & S

Footing Properties

$Width_{req}$: 12.5 in., Concrete Footing
$Width_{chosen}$: 24 in. O.K.
$Height_{req}$: 4.0 in., Concrete Footing
$Height_{chosen}$: 8 in. O.K.

(2) #5 bar =.62 in2 > Asreq O.K.

Bars at bottom have 3" of Cover Req'd

Use 24'' wide x 8'' tall footing, with (2) #5 bar continuous

III. Support Structure

C. Spread Footings at garage

1. Worse case exterior wall load

P_{max} = 1400.0 #, Wall, Floor & Roof Loads
P_{wall} = 400 #, wall weight
P_{total} = 1800.0 # on Footing

Wall Properties

Height: 4.00 ft., Concrete Wall
Width: 8 in., Concrete Wall

pf_{allow} = 3000 psf, Allowable Soil Pressure
F'c = 3000 psi, Conc. Compr. Strength
As_{req}= 0.288 in2, (0.0018*b*h) for T & S

Footing Properties

$Width_{req}$: 7.2 in., Concrete Footing
$Width_{chosen}$: 20 in. O.K.
$Height_{req}$: 3.4 in., Concrete Footing
$Height_{chosen}$: 8 in. O.K.

(2) #5 bar =.62 in2 > Asreq O.K.

Bars at bottom have 3" of Cover Req'd

Use 20'' wide x 8'' tall footing, with (2) #5 bar continuous

III. Support Structure

B. Steel Pipe Columns

1. Column C-1

Try: 3.00 " Dia. std Steel Pipe

Column Height (kL):	9.00	feet	Fy :	35	ksi
Unfactored Load P:	13.40	kips	r :	1.16	inches
Factored Load Pu:	13.00	kips	Ag:	2.23	in^2

$\lambda c = kL/r*(F_y/\pi^2*E)^{(1/2)}$ $\quad \lambda c = 1.03$

If $\lambda c<1.5$, $Fcr = (0.658^{\lambda c^2}) *Fy$ $\quad Fcr = 22.46$

If $\lambda c>1.5$, $Fcr = (0.877/\lambda c^2) *Fy$

$\varphi Pcr = 0.85*Fcr*As$ $\quad \varphi Pcr = 42.57$ kips < 13.00 kips O.K.

At Column C-1	, Use	3	" Dia.	7.58	Lbs/ft Pipe

III. Support Structure

B. Steel Pipe Columns

1. Column C-2

Try: 3 " Dia. std Steel Pipe

Column Height (kL):	9.00	feet	Fy :	35	ksi
Unfactored Load P:	12.50	kips	r :	1.16	inches
Factored Load Pu:	26.00	kips	Ag:	2.23	in^2

$\lambda c = kL/r*(F_y/\pi^2*E)^{(1/2)}$ $\quad \lambda c = 1.03$

If $\lambda c<1.5$, $Fcr = (0.658^{\lambda c^2}) * Fy$ $\quad Fcr = 22.46$

If $\lambda c>1.5$, $Fcr = (0.877/\lambda c^2) * Fy$

$\varphi Pcr = 0.85*Fcr*As$ $\quad \varphi Pcr = 42.57$ kips < 26.00 kips O.K.

At Column C-2 , Use 3 " Dia. 7.58 Lbs/ft Pipe

III. Support Structure

B. Steel Pipe Columns

1. Column C-3

Try: 3 " Dia. std Steel Pipe

Column Height (kL):	9.00	feet	Fy :	35	ksi
Unfactored Load P:	24.60	kips	r :	1.16	inches
Factored Load Pu:	36.65	kips	Ag:	2.23	in^2

$\lambda c = kL/r*(F_y/\pi^2*E)^{(1/2)}$ $\quad \lambda c = 1.03$

If $\lambda c<1.5$, $Fcr = (0.658^{\lambda c^2}) * Fy$ $\quad Fcr = 22.46$

If $\lambda c>1.5$, $Fcr = (0.877/\lambda c^2) * Fy$

$\varphi Pcr = 0.85*Fcr*As$ $\quad \varphi Pcr = 42.57$ kips < 36.65 kips O.K.

At Column C-3 , Use 3 " Dia. 7.58 Lbs/ft Pipe

III. Support Structure

B. Steel Pipe Columns

1. Basement Beam C-4

Try: 3.50 " Dia. std Steel Pipe

Column Height (kL):	9.00	feet	Fy :	35	ksi
Unfactored Load P:	30.22	kips	r :	1.34	inches
Factored Load Pu:	45.03	kips	Ag:	2.68	in^2

$\lambda c = kL/r*(F_y/\pi^2*E)^{(1/2)}$ $\quad \lambda c = 0.891$

If $\lambda c<1.5$, $Fcr = (0.658^{\lambda c^2}) * Fy$ $\quad Fcr = 25.1$

If $\lambda c>1.5$, $Fcr = (0.877/\lambda c^2) * Fy$

$\varphi Pcr = 0.85*Fcr*As$ $\quad \varphi Pcr = 57.18$ kips < 45.03 kips O.K.

At Basement Beam C-4 , Use 3.50 " Dia. 9.12 Lbs/ft Pipe

TDI Associates, Inc.
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Job: Stratton Residence-Fox Point
Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

W 8 x 21 -Wide Flange Section Designation B-1

50,000 psi, Elastic Yield Stress
LRFD, 3rd Edition

Beam/Column Length (strong)	**12'6"**		
$(k)_x$ length factor adjustment	**1.0**		
Unbraced Length of Comp. Flange	**1.34**	feet	
Beam/Column Length (weak)	**12'6"**		
$(k)_y$ length factor adjustment	**1.0**		
Axial Load 1, and type (kips)	**0.1**	**DL**	
Axial Load 2, and type (kips)	**0.1**	**LL**	
Transverse Load 1, strong (#/LF)	**720**	**DL**	
Transverse Load 2, strong (#/LF)	**1440**	**LL**	
Transverse Load 1, weak (#/LF)	**0**	**WL**	
Transverse Load 2, weak (#/LF)	**0**	**-**	
Torsion Load 1, and type (#/LF)	**500**	**DL**	
Torsion Load 2, and type (#/LF)	**500**	**LL**	

Properties from Section Designated, include LTB

$\lambda c_x = kL_x/r_x*(F_y/\pi^{\wedge}2*E)$	λc_x	0.568
If $\lambda c_x<1.5$, $Fcr_x = (0.658^{\wedge}\lambda c_x^{\wedge}2) *Fy$	Fcr_x	43.68
If $\lambda c_x>1.5$, $Fcr_x = (0.877/\lambda c_x^{\wedge}2) *Fy$	Fcr_x	0.00
$\lambda c_y = kL_y/r_y*(F_y/\pi^{\wedge}2*E)$	λc_y	1.573
If $\lambda c_y<1.5$, $Fcr_y = (0.658^{\wedge}\lambda c_y^{\wedge}2) *Fy$	Fcr_y	0.00
If $\lambda c_y>1.5$, $Fcr_y = (0.877/\lambda c_y^{\wedge}2) *Fy$	Fcr_y	17.71

Capacity for Axial Compression	φPn (kips)	92.74	**O.K.**
Capacity for Moment (strong axis)	φMn_x (k-ft)	76.50	**O.K.**
Capacity for Moment (weak axis)	φMn_y (k-ft)	21.34	**O.K.**
Capacity for Shear	φVn (k-ft)	55.89	**O.K.**
Capacity for Torsion	φTn (k-ft)	342.00	**O.K.**

Calculate Factored Load Case 1: 1.2DL + 1.6LL + 0.8WL

Factored Axial Load	P_u (kips)	0.28
Factored Transverse Load (strong)	w_u (k/ft)	3.168
Factored Transverse Load (weak)	w_u (k/ft)	0
Factored Torsion Load	T_u (k/ft)	1.4
Factored Shear (strong axis)	Vu_x (kips)	19.80
Factored Shear (weak axis)	Vu_y (kips)	0.00
Factored Moment (strong axis)	Mu_x (k-ft)	61.88
Additional Moment (strong axis)	$+Mu_x$ (k-ft)	0.00
Factored Moment (weak axis)	Mu_y (k-ft)	0.00
Additional Moment (weak axis)	$+Mu_y$ (k-ft)	0.00
Pe_x (max. Euler value for P-delta)	Pe_x (kips)	957.88
Pe_y (max. Euler value for P-delta)	Pe_y (kips)	124.28
Moment Magnification (strong)	$B1_x$	1.000
Moment Magnification (weak)	$B1_y$	1.002

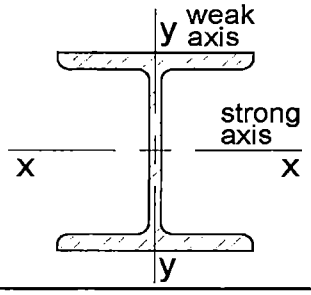
%Axial+%Moment+(%Shear+%Torsion)$^2 \leq 1.0$ **0.939** **O.K.**

Calculate Factored Load Case 2: 1.2DL + 1.0LL + 1.6WL

Factored Axial Load	P_u (kips)	0.22
Factored Transverse Load (strong)	w_u (k/ft)	2.304
Factored Transverse Load (weak)	w_u (k/ft)	0
Factored Torsion Load	T_u (k/ft)	1.1
Factored Shear (strong axis)	Vu_x (kips)	14.40
Factored Shear (weak axis)	Vu_y (kips)	0.00
Factored Moment (strong axis)	Mu_x (k-ft)	45.00
Additional Moment (strong axis)	$+Mu_x$ (k-ft)	0.00
Factored Moment (weak axis)	Mu_y (k-ft)	0.00
Additional Moment (weak axis)	$+Mu_y$ (k-ft)	0.00
Pe_x (max. Euler value for P-delta)	Pe_x (kips)	957.88
Pe_y (max. Euler value for P-delta)	Pe_y (kips)	124.28
Moment Magnification (strong)	$B1_x$	1.000
Moment Magnification (weak)	$B1_y$	1.002

%Axial+%Moment+(%Shear+%Torsion)$^2 \leq 1.0$ **0.718** **O.K.**

Interaction Equations:
If $Pu/\varphi Pn \geq 0.2$, then $Pu/\varphi Pn + 8/9(Mu_x/\varphi Mn_x + Mu_y/\varphi Mn_y) \leq 1.0$
If $Pu/\varphi Pn < 0.2$, then $Pu/(2\varphi Pn) + Mu_x/\varphi Mn_x + Mu_y/\varphi Mn_y \leq 1.0$
* If Torsion is >0, then add: $(Vu/\varphi Vn + Tu/\varphi Tn)^2$



Wide Flange Member designed for Elastic and Inelastic LTB

Deflection

Deflection Bending L/240	**L/240**	**0.625**	
vs. calculated Defl. (total load)	$5wL^4/384EI$	0.543	**O.K.**
Deflection Bending L/360	**L/360**	**0.417**	
vs. calculated Defl. (live load)	$5wL^4/384EI$	0.362	**O.K.**

** If wind is causing bending, mult. w by 0.7, IBC 1604.3

TDI Associates, Inc.
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Job: Stratton Residence-Fox Point
Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

W 8 x 21 -Wide Flange Section Designation B-2

50,000 psi, Elastic Yield Stress
LRFD, 3rd Edition

Beam/Column Length (strong)	**11'9"**	
$(k)_x$ length factor adjustment	**1.0**	
Unbraced Length of Comp. Flange	**1.34**	feet
Beam/Column Length (weak)	**11'9"**	
$(k)_y$ length factor adjustment	**1.0**	
Axial Load 1, and type (kips)	**0.1**	**DL**
Axial Load 2, and type (kips)	**0.1**	**LL**
Transverse Load 1, strong (#/LF)	**720**	**DL**
Transverse Load 2, strong (#/LF)	**1440**	**LL**
Transverse Load 1, weak (#/LF)	**0**	**WL**
Transverse Load 2, weak (#/LF)	**0**	**-**
Torsion Load 1, and type (#/LF)	**500**	**DL**
Torsion Load 2, and type (#/LF)	**500**	**LL**

Properties from Section Designated, include LTB

$\lambda c_x = kL_x/r_x*(F_y/\pi^{2}*E)$	λc_x	0.534
If $\lambda c_x<1.5$, $Fcr_x = (0.658^{\lambda c_x^2}) *Fy$	Fcr_x	44.38
If $\lambda c_x>1.5$, $Fcr_x = (0.877/\lambda c^2) *Fy$	Fcr_x	0.00
$\lambda c_y = kL_y/r_y*(F_y/\pi^{2}*E)$	λc_y	1.479
If $\lambda c_y<1.5$, $Fcr_y = (0.658^{\lambda c_y^2}) *Fy$	Fcr_y	20.01
If $\lambda c_y>1.5$, $Fcr_y = (0.877/\lambda c^2) *Fy$	Fcr_y	0.00

Capacity for Axial Compression	φPn (kips)	104.79	**O.K.**
Capacity for Moment (strong axis)	φMn_x (k-ft)	76.50	**O.K.**
Capacity for Moment (weak axis)	φMn_y (k-ft)	21.34	**O.K.**
Capacity for Shear	φVn (k-ft)	55.89	**O.K.**
Capacity for Torsion	φTn (k-ft)	342.00	**O.K.**

Calculate Factored Load Case 1: 1.2DL + 1.6LL + 0.8WL

Factored Axial Load	P_u (kips)	0.28
Factored Transverse Load (strong)	w_u (k/ft)	3.168
Factored Transverse Load (weak)	w_u (k/ft)	0
Factored Torsion Load	T_u (k/ft)	1.4
Factored Shear (strong axis)	Vu_x (kips)	18.61
Factored Shear (weak axis)	Vu_y (kips)	0.00
Factored Moment (strong axis)	Mu_x (k-ft)	54.67
Additional Moment (strong axis)	$+Mu_x$ (k-ft)	0.00
Factored Moment (weak axis)	Mu_y (k-ft)	0.00
Additional Moment (weak axis)	$+Mu_y$ (k-ft)	0.00
Pe_x (max. Euler value for P-delta)	Pe_x (kips)	1084.06
Pe_y (max. Euler value for P-delta)	Pe_y (kips)	140.65
Moment Magnification (strong)	$B1_x$	1.000
Moment Magnification (weak)	$B1_y$	1.002

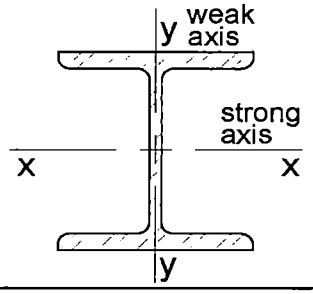
%Axial+%Moment+(%Shear+%Torsion)$^2 \leq 1.0$ **0.830** **O.K.**

Calculate Factored Load Case 2: 1.2DL + 1.0LL + 1.6WL

Factored Axial Load	P_u (kips)	0.22
Factored Transverse Load (strong)	w_u (k/ft)	2.304
Factored Transverse Load (weak)	w_u (k/ft)	0
Factored Torsion Load	T_u (k/ft)	1.1
Factored Shear (strong axis)	Vu_x (kips)	13.54
Factored Shear (weak axis)	Vu_y (kips)	0.00
Factored Moment (strong axis)	Mu_x (k-ft)	39.76
Additional Moment (strong axis)	$+Mu_x$ (k-ft)	0.00
Factored Moment (weak axis)	Mu_y (k-ft)	0.00
Additional Moment (weak axis)	$+Mu_y$ (k-ft)	0.00
Pe_x (max. Euler value for P-delta)	Pe_x (kips)	1084.06
Pe_y (max. Euler value for P-delta)	Pe_y (kips)	140.65
Moment Magnification (strong)	$B1_x$	1.000
Moment Magnification (weak)	$B1_y$	1.002

%Axial+%Moment+(%Shear+%Torsion)$^2 \leq 1.0$ **0.635** **O.K.**

Interaction Equations:
If $Pu/\varphi Pn \geq 0.2$, then $Pu/\varphi Pn + 8/9(Mu_x/\varphi Mn_x + Mu_y/\varphi Mn_y) \leq 1.0$
If $Pu/\varphi Pn < 0.2$, then $Pu/(2\varphi Pn) + Mu_x/\varphi Mn_x + Mu_y/\varphi Mn_y \leq 1.0$
* If Torsion is >0, then add: $(Vu/\varphi Vn + Tu/\varphi Tn)^2$



Wide Flange Member designed for Elastic and Inelastic LTB

Deflection

Deflection Bending L/240	**L/240**	**0.588**	
vs. calculated Defl. (total load)	$5wL^4/384EI$	0.424	**O.K.**
Deflection Bending L/360	**L/360**	**0.392**	
vs. calculated Defl. (live load)	$5wL^4/384EI$	0.283	**O.K.**

** If wind is causing bending, mult. w by 0.7, IBC 1604.3

TDI Associates, Inc.
Architects • Engineers • Planners

Job: Stratton Residence-Fox Point
Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

W 8 x 21 -Wide Flange Section Designation B-3

50,000 psi, Elastic Yield Stress
LRFD, 3rd Edition

Beam/Column Length (strong)	**12'0"**		
$(k)_x$ length factor adjustment	**1.0**		
Unbraced Length of Comp. Flange	**1.34**	feet	
Beam/Column Length (weak)	**12'0"**		
$(k)_y$ length factor adjustment	**1.0**		
Axial Load 1, and type (kips)	**0.1**	**DL**	
Axial Load 2, and type (kips)	**0.1**	**LL**	
Transverse Load 1, strong (#/LF)	**720**	**DL**	
Transverse Load 2, strong (#/LF)	**1440**	**LL**	
Transverse Load 1, weak (#/LF)	**0**	**WL**	
Transverse Load 2, weak (#/LF)	**0**	**-**	
Torsion Load 1, and type (#/LF)	**500**	**DL**	
Torsion Load 2, and type (#/LF)	**500**	**LL**	

Properties from Section Designated, include LTB

$\lambda c_x = kL_x/r_x*(F_y/\pi^{c}*E)$	λc_x	0.545
If $\lambda c_x<1.5$, $Fcr_x = (0.658^{\lambda c_x^2}) *Fy$	Fcr_x	44.15
If $\lambda c_x>1.5$, $Fcr_x = (0.877/\lambda c^{c}) *Fy$	Fcr_x	0.00
$\lambda c_y = kL_y/r_y*(F_y/\pi^{c}*E)$	λc_y	1.511
If $\lambda c_y<1.5$, $Fcr_y = (0.658^{\lambda c_y^2}) *Fy$	Fcr_y	0.00
If $\lambda c_y>1.5$, $Fcr_y = (0.877/\lambda c^{c}) *Fy$	Fcr_y	19.22

Capacity for Axial Compression	φPn (kips)	100.63	**O.K.**
Capacity for Moment (strong axis)	φMn_x (k-ft)	76.50	**O.K.**
Capacity for Moment (weak axis)	φMn_y (k-ft)	21.34	**O.K.**
Capacity for Shear	φVn (k-ft)	55.89	**O.K.**
Capacity for Torsion	φTn (k-ft)	342.00	**O.K.**

Calculate Factored Load Case 1: 1.2DL + 1.6LL + 0.8WL

Factored Axial Load	P_u (kips)	0.28
Factored Transverse Load (strong)	w_u (k/ft)	3.168
Factored Transverse Load (weak)	w_u (k/ft)	0
Factored Torsion Load	T_u (k/ft)	1.4
Factored Shear (strong axis)	Vu_x (kips)	19.01
Factored Shear (weak axis)	Vu_y (kips)	0.00
Factored Moment (strong axis)	Mu_x (k-ft)	57.02
Additional Moment (strong axis)	$+Mu_x$ (k-ft)	0.00
Factored Moment (weak axis)	Mu_y (k-ft)	0.00
Additional Moment (weak axis)	$+Mu_y$ (k-ft)	0.00
Pe_x (max. Euler value for P-delta)	Pe_x (kips)	1039.36
Pe_y (max. Euler value for P-delta)	Pe_y (kips)	134.86
Moment Magnification (strong)	$B1_x$	1.000
Moment Magnification (weak)	$B1_y$	1.002

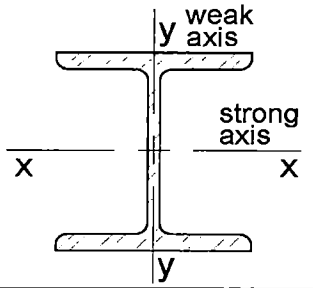
%Axial+%Moment+(%Shear+%Torsion)$^2 \leq 1.0$ **0.865** **O.K.**

Calculate Factored Load Case 2: 1.2DL + 1.0LL + 1.6WL

Factored Axial Load	P_u (kips)	0.22
Factored Transverse Load (strong)	w_u (k/ft)	2.304
Factored Transverse Load (weak)	w_u (k/ft)	0
Factored Torsion Load	T_u (k/ft)	1.1
Factored Shear (strong axis)	Vu_x (kips)	13.82
Factored Shear (weak axis)	Vu_y (kips)	0.00
Factored Moment (strong axis)	Mu_x (k-ft)	41.47
Additional Moment (strong axis)	$+Mu_x$ (k-ft)	0.00
Factored Moment (weak axis)	Mu_y (k-ft)	0.00
Additional Moment (weak axis)	$+Mu_y$ (k-ft)	0.00
Pe_x (max. Euler value for P-delta)	Pe_x (kips)	1039.36
Pe_y (max. Euler value for P-delta)	Pe_y (kips)	134.86
Moment Magnification (strong)	$B1_x$	1.000
Moment Magnification (weak)	$B1_y$	1.002

%Axial+%Moment+(%Shear+%Torsion)$^2 \leq 1.0$ **0.662** **O.K.**

Interaction Equations:
If $Pu/\varphi Pn \geq 0.2$, then $Pu/\varphi Pn + 8/9(Mu_x/\varphi Mn_x + Mu_y/\varphi Mn_y) \leq 1.0$
If $Pu/\varphi Pn < 0.2$, then $Pu/(2\varphi Pn) + Mu_x/\varphi Mn_x + Mu_y/\varphi Mn_y \leq 1.0$
* If Torsion is >0, then add: $(Vu/\varphi Vn + Tu/\varphi Tn)^2$



Wide Flange Member designed for Elastic and Inelastic LTB

Deflection

Deflection Bending L/240	**L/240**	**0.600**	
vs. calculated Defl. (total load)	$5wL^4/384EI$	0.461	**O.K.**
Deflection Bending L/360	**L/360**	**0.400**	
vs. calculated Defl. (live load)	$5wL^4/384EI$	0.308	**O.K.**

** If wind is causing bending, mult. w by 0.7, IBC 1604.3

TDI Associates, Inc.
Architects • Engineers • Planners

Job: Stratton Residence-Fox Point
Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

W 8 x 24 -Wide Flange Section Designation B-4

50,000 psi, Elastic Yield Stress
LRFD, 3rd Edition

Beam/Column Length (strong)	**16'4"**		
$(k)_x$ length factor adjustment	**1.0**		
Unbraced Length of Comp. Flange	**1.34**	feet	
Beam/Column Length (weak)	**16'4"**		
$(k)_y$ length factor adjustment	**1.0**		
Axial Load 1, and type (kips)	**0.1**	**DL**	
Axial Load 2, and type (kips)	**0.1**	**LL**	
Transverse Load 1, strong (#/LF)	**200**	**DL**	
Transverse Load 2, strong (#/LF)	**400**	**LL**	
Transverse Load 1, weak (#/LF)	**0**	**WL**	
Transverse Load 2, weak (#/LF)	**0**	**-**	
Torsion Load 1, and type (#/LF)	**250**	**DL**	
Torsion Load 2, and type (#/LF)	**250**	**LL**	

Properties from Section Designated, include LTB

$\lambda c_x = kL_x/r_x*(F_y/\pi^{\wedge}2*E)$	λc_x	0.757	
If $\lambda c_x<1.5$, $Fcr_x = (0.658^{\wedge}\lambda c^{\wedge}2)*Fy$	Fcr_x	39.33	
If $\lambda c_x>1.5$, $Fcr_x = (0.877/\lambda c^{\wedge}2)*Fy$	Fcr_x	0.00	
$\lambda c_y = kL_y/r_y*(F_y/\pi^{\wedge}2*E)$	λc_y	1.609	
If $\lambda c_y<1.5$, $Fcr_y = (0.658^{\wedge}\lambda c^{\wedge}2)*Fy$	Fcr_y	0.00	
If $\lambda c_y>1.5$, $Fcr_y = (0.877/\lambda c^{\wedge}2)*Fy$	Fcr_y	16.94	
Capacity for Axial Compression	φPn (kips)	101.93	**O.K.**
Capacity for Moment (strong axis)	φMn_x (k-ft)	86.63	**O.K.**
Capacity for Moment (weak axis)	φMn_y (k-ft)	32.14	**O.K.**
Capacity for Shear	φVn (k-ft)	52.46	**O.K.**
Capacity for Torsion	φTn (k-ft)	582.75	**O.K.**

Calculate Factored Load Case 1: 1.2DL + 1.6LL + 0.8WL

Factored Axial Load	P_u (kips)	0.28
Factored Transverse Load (strong)	w_u (k/ft)	0.88
Factored Transverse Load (weak)	w_u (k/ft)	0
Factored Torsion Load	T_u (k/ft)	0.7
Factored Shear (strong axis)	Vu_x (kips)	7.19
Factored Shear (weak axis)	Vu_y (kips)	0.00
Factored Moment (strong axis)	Mu_x (k-ft)	29.35
Additional Moment (strong axis)	$+Mu_x$ (k-ft)	0.00
Factored Moment (weak axis)	Mu_y (k-ft)	0.00
Additional Moment (weak axis)	$+Mu_y$ (k-ft)	0.00
Pe_x (max. Euler value for P-delta)	Pe_x (kips)	616.16
Pe_y (max. Euler value for P-delta)	Pe_y (kips)	136.34
Moment Magnification (strong)	$B1_x$	1.000
Moment Magnification (weak)	$B1_y$	1.002

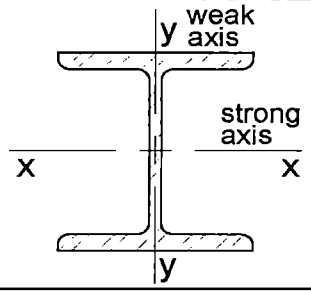
%Axial+%Moment+(%Shear+%Torsion)$^2 \le 1.0$ **0.359** **O.K.**

Calculate Factored Load Case 2: 1.2DL + 1.0LL + 1.6WL

Factored Axial Load	P_u (kips)	0.22
Factored Transverse Load (strong)	w_u (k/ft)	0.64
Factored Transverse Load (weak)	w_u (k/ft)	0
Factored Torsion Load	T_u (k/ft)	0.55
Factored Shear (strong axis)	Vu_x (kips)	5.23
Factored Shear (weak axis)	Vu_y (kips)	0.00
Factored Moment (strong axis)	Mu_x (k-ft)	21.34
Additional Moment (strong axis)	$+Mu_x$ (k-ft)	0.00
Factored Moment (weak axis)	Mu_y (k-ft)	0.00
Additional Moment (weak axis)	$+Mu_y$ (k-ft)	0.00
Pe_x (max. Euler value for P-delta)	Pe_x (kips)	616.16
Pe_y (max. Euler value for P-delta)	Pe_y (kips)	136.34
Moment Magnification (strong)	$B1_x$	1.000
Moment Magnification (weak)	$B1_y$	1.002

%Axial+%Moment+(%Shear+%Torsion)$^2 \le 1.0$ **0.267** **O.K.**

Interaction Equations:
If $Pu/\varphi Pn \ge 0.2$, then $Pu/\varphi Pn + 8/9(Mu_x/\varphi Mn_x + Mu_y/\varphi Mn_y) \le 1.0$
If $Pu/\varphi Pn < 0.2$, then $Pu/(2\varphi Pn) + Mu_x/\varphi Mn_x + Mu_y/\varphi Mn_y \le 1.0$
* If Torsion is >0, then add: $(Vu/\varphi Vn + Tu/\varphi Tn)^2$



Wide Flange Member designed for Elastic and Inelastic LTB

Deflection

Deflection Bending L/240	**L/240**	**0.817**	
vs. calculated Defl. (total load)	$5wL^4/384EI$	0.401	**O.K.**
Deflection Bending L/360	**L/360**	**0.544**	
vs. calculated Defl. (live load)	$5wL^4/384EI$	0.267	**O.K.**

** If wind is causing bending, mult. w by 0.7, IBC 1604.3

I. LOADS CONTINUED

G. Adjustment Factors for Environmental Loads

5. Wind Analysis

Tabular Results from Wind Speed of 90 MPH and Roof Slope (degrees): 39.00
Exposure Class B (No reduction or magification factor)
Mean Roof Height (ft): 25.0

- Main Force Resisting System: Low Rise Building Provisions

	Horizontal Loads	
	End Zone	Interior Zone
Wall	14.4	11.5
Roof	9.9	7.9

	Vertical Loads	
	End Zone	Interior Zone
Leeward	-8.8	-7.5
Windward	-5.6	4.8
Overhang	-5.1	-5.8

Height and Exposure Adjustment Coefficie: 1.00

	Horizontal Loads * Coeff.	
	End Zone	Interior Zone
Wall	14.4	11.5
Roof	9.9	7.9

	Vertical Loads * Coeff.	
	End Zone	Interior Zone
Leeward	-8.8	-7.5
Windward	-5.6	4.8
Overhang	-5.1	-5.8

- Components and Cladding: Low Rise Building Provisions

	Wall	
Area	Dir. Pres.	Uplift
10	14.6	-15.8
20	13.9	-15.2
50	13.0	-14.3
100	12.4	-13.5

	Roof	
Area	Dir. Pres.	Uplift
10	13.3	-14.6
20	13.0	-13.8
50	12.5	-12.8
100	12.1	-12.1

Height and Exposure Adjustment Coefficie: 1.00

	Wall * Coeff.	
Area	Dir. Pres.	Uplift
10	14.6	-15.8
20	13.9	-15.2
50	13.0	-14.3
100	12.4	-13.5

	Roof * Coeff.	
Area	Dir. Pres.	Uplift
10	13.3	-14.6
20	13.0	-13.8
50	12.5	-12.8
100	12.1	-12.1

III. Lateral wind Force Design Table

shear wall type	floor	grid	wind force	trib. span	height	no. of stories	shear force	shear force on wall	total shear force on wall
A	1ST	END	14.4	19.25	9.50	1	2,633.40		
A	1ST	END	11.9	19.25	0.00	1	0.00	2,633.40	6,209.96
B	1ST	INTERIOR	11.5	19.25	9.50	1	2,103.06		
B	1ST	INTERIOR	7.9	19.25	0.00	1	0.00	2,103.06	4,626.83
C	1ST	END	14.4	19.25	9.50	1	2,633.40		
C	1ST	END	11.9	19.25	0.00	1	0.00	2,633.40	6,209.96
D	1ST	END	14.4	19.75	9.50	1	2,701.80		
D	1ST	END	11.9	19.75	0.00	1	0.00	2,701.80	6,307.16
E	1ST	INTERIOR	11.5	19.75	9.50	1	2,157.69		
E	1ST	INTERIOR	7.9	19.75	0.00	1	0.00	2,157.69	4,704.45
F	1ST	END	14.4	19.75	9.50	1	2,701.80		
F	1ST	END	11.9	19.75	0.00	1	0.00	2,701.80	6,307.16
G	2ND	END	14.4	19.25	4.00	1	1,108.80		
G	2ND	END	11.9	19.25	10.50	1	2,467.76	3,576.56	
H	2ND	INTERIOR	11.5	19.25	4.00	1	885.50		
H	2ND	INTERIOR	7.9	19.25	10.50	1	1,638.26	2,523.76	
I	2ND	END	14.4	19.25	4.00	1	1,108.80		
I	2ND	END	11.9	19.25	10.50	1	2,467.76	3,576.56	
J	2ND	END	14.4	19.75	4.00	1	1,137.60		
J	2ND	END	11.9	19.75	10.50	1	2,467.76	3,605.36	
K	2ND	INTERIOR	11.5	19.75	4.00	1	908.50		
K	2ND	INTERIOR	7.9	19.75	10.50	1	1,638.26	2,546.76	
L	2ND	END	14.4	19.75	4.00	1	1,137.60		
L	2ND	END	11.9	19.75	10.50	1	2,467.76	3,605.36	

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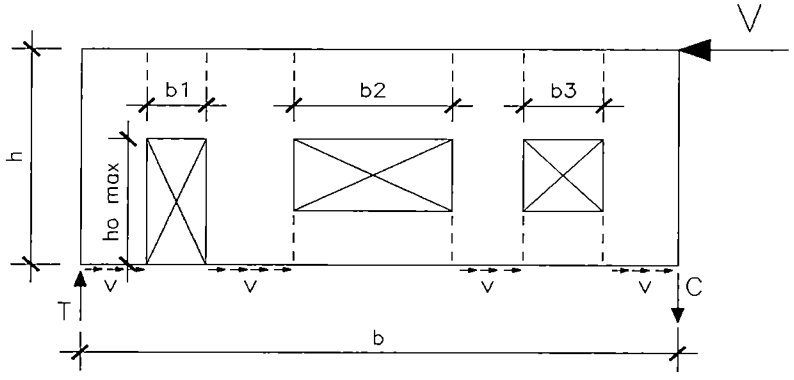
Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

Perforated Shear Wall Worksheet SW-#A 1ST Floor



$b_o = b1 + b2 + b3$
$b_{fh} = b - b_o$
ho = height, maximum, for openings

V = Shear applied to Wall
v = Unit Shear (from Piers * Perforation Factor)

T = Tension Force for Holddown to System Below
C = Compression Force to System Below

** Diagram is for illustrative purposes only. See Elevation Sheets for shear wall being examined.

C_o Table

b_{fh} / b	h_0 / h 0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
0.0	1.000	1.000	1.000	0.802	0.670	0.602	0.480	0.420	0.358	0.330
0.1	1.000	1.000	1.000	0.814	0.690	0.626	0.510	0.450	0.388	0.360
0.2	1.000	1.000	1.000	0.826	0.710	0.650	0.538	0.472	0.408	0.380
0.3	1.000	1.000	1.000	0.844	0.740	0.680	0.570	0.510	0.448	0.420
0.4	1.000	1.000	1.000	0.862	0.770	0.714	0.610	0.550	0.482	0.450
0.5	1.000	1.000	1.000	0.880	0.800	0.748	0.650	**0.590**	0.528	0.500
0.6	1.000	1.000	1.000	0.898	0.830	0.782	0.694	0.646	0.588	0.560
0.7	1.000	1.000	1.000	0.922	0.870	0.830	0.754	0.706	0.654	0.630
0.8	1.000	1.000	1.000	0.946	0.910	0.878	0.818	0.782	0.734	0.710
0.9	1.000	1.000	1.000	0.970	0.950	0.934	0.902	0.878	0.846	0.830
1.0	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000

Wall Input Values		
Height of Wall	h (ft)	9
Maximum Opening Height	h_o (ft)	7
Width of Wall (total)	b (ft)	59.25
Sum of Wall Opening Width	b_o (ft)	AAA

Calculate Perforation Reduction		
Max Opening Hgt / Wall Hgt	h / h_0	0.78
Shear Wall Remaining (piers)	b_{fh} (ft)	#VALUE!
Pier length / wall length	b_{fh} / b	#VALUE!
Perforation factor	**C_o (ft)**	**#VALUE!**

*Perforation Factor is from Double Intertoplation of **C_o Table**

Hold-down Force Required		
Length between Hold-downs	L (ft)	59
Tension Force	**T (lbs)**	**947.29**

IBC Code: Can sum the sheathing values per side, but the summation of the two must be less that twice the lowest unit shear of the two materials.

Force Input Value		
Shear Force applied to Wall	V (lbs)	6210

Unit Shear Calculation		
Remaining Pier Length * C_o	b_{fh} * C_o (ft)	#VALUE!
75 Unit Shear, V / Pier length	**v (lb/ft)**	**#VALUE!**

Side 1: Sheathing: 1/2" OSB			
Length	L (ft)	#VALUE!	
Unit Shear Capacity	v (lb/ft)	260	**8d Nail @ 6/12 Pattern**

Side 2: Sheating 1/2" GYP			
Length	L (ft)	#VALUE!	
Unit Shear Capacity	v (lb/ft)	80	**#6Screw @ 8/12 pattern**

Total Force Capacity	**V (lbs)**	**#VALUE!**	#VALUE!	**#VALUE!**
Equivalent Unit Shear	**v (lb/ft)**	**#VALUE!**	#VALUE!	**#VALUE!**

Gypsum is screwed at 16" OC at edge joint along horizontal butt joints

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Architects • Engineers • Planners

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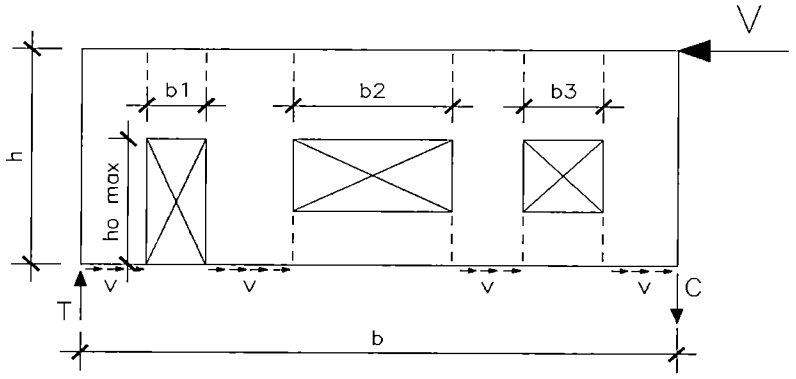
Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

Perforated Shear Wall Worksheet SW-#B-1ST FLR



$b_o = b1 + b2 + b3$
$b_{fh} = b - b_o$
ho = height, maximum, for openings

V = Shear applied to Wall
v = Unit Shear (from Piers * Perforation Factor)

T = Tension Force for Holddown to System Below
C = Compression Force to System Below

** Diagram is for illustrative purposes only. See Elevation Sheets for shear wall being examined.

C_o Table

h_o / h

b_{fh} / b	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
0.0	1.000	1.000	1.000	0.802	0.670	0.602	0.480	0.420	0.358	0.330
0.1	1.000	1.000	1.000	0.814	0.690	0.626	0.510	0.450	0.388	0.360
0.2	1.000	1.000	1.000	0.826	0.710	0.650	0.538	0.472	0.408	0.380
0.3	1.000	1.000	1.000	0.844	0.740	0.680	0.570	0.510	0.448	0.420
0.4	1.000	1.000	1.000	0.862	0.770	0.714	0.610	0.550	0.482	0.450
0.5	1.000	1.000	1.000	0.880	0.800	0.748	0.650	0.590	0.528	0.500
0.6	1.000	1.000	1.000	0.898	0.830	0.782	0.694	0.646	0.588	0.560
0.7	1.000	1.000	1.000	0.922	0.870	0.830	0.754	0.706	0.654	0.630
0.8	1.000	1.000	1.000	0.946	0.910	0.878	0.818	0.782	0.734	0.710
0.9	1.000	1.000	1.000	0.970	0.950	0.934	0.902	0.878	0.846	0.830
1.0	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000

Wall Input Values		
Height of Wall	h (ft)	9
Maximum Opening Height	h_o (ft)	7
Width of Wall (total)	b (ft)	59
Sum of Wall Opening Width	b_o (ft)	12

Calculate Perforation Reduction		
Max Opening Hgt / Wall Hgt	h / h_o	0.78
Shear Wall Remaining (piers)	b_{fh} (ft)	47
Pier length / wall length	b_{fh} / b	0.797
Perforation factor	**C_o (ft)**	**0.816**

*Perforation Factor is from Double Intertoplation of **C_o Table**

Hold-down Force Required		
Length between Hold-downs	L (ft)	58.75
Tension Force	**T (lbs)**	**708.82**

IBC Code: Can sum the sheathing values per side, but the summation of the two must be less that twice the lowest unit shear of the two materials.

Force Input Value		
Shear Force applied to Wall	V (lbs)	4627

Unit Shear Calculation		
Remaining Pier Length * C_o	b_{fh} * C_o (ft)	38.34
Unit Shear, V / Pier length	**v (lb/ft)**	**120.67**
8		

Side 1: Sheathing: 1/2" Gyp. Board			
Length	L (ft)	38.34	
Unit Shear Capacity	v (lb/ft)	80	**#6 Screw @ 8/12 Pattern**

Side 2: Sheathing: 1/2" Gyp. Board			
Length	L (ft)	38.34	
Unit Shear Capacity	v (lb/ft)	80	**#6 Screw @ 8/12 Pattern**

Total Force Capacity	**V (lbs)**	**6135.0**	> 4627	**O.K.**
Equivalent Unit Shear	**v (lb/ft)**	**160**	> 120.672	**O.K.**

Gypsum is screwed at 16" OC at edge joint along horizontal butt joints

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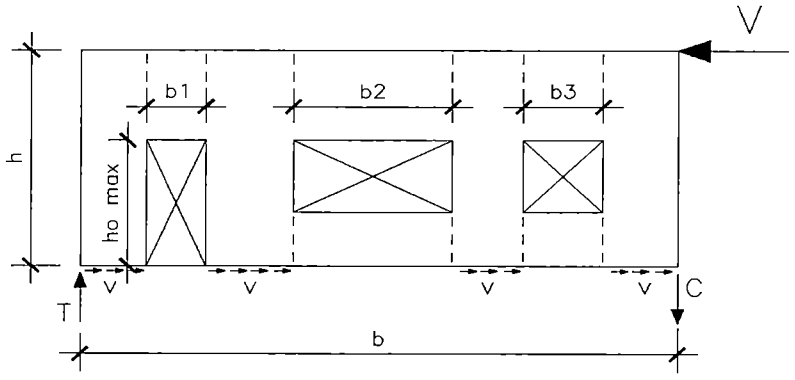
Job: Stratton Residence-Fox Point
Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

Perforated Shear Wall Worksheet SW-#C 1ST Floor



$b_o = b1 + b2 + b3$
$b_{fh} = b - b_o$
ho = height, maximum, for openings

V = Shear applied to Wall
v = Unit Shear (from Piers * Perforation Factor)

T = Tension Force for Holddown to System Below
C = Compression Force to System Below

** Diagram is for illustrative purposes only. See Elevation Sheets for shear wall being examined.

C_o Table

h_0 / h

b_{fh} / b	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
0.0	1.000	1.000	1.000	0.802	0.670	0.602	0.480	0.420	0.358	0.330
0.1	1.000	1.000	1.000	0.814	0.690	0.626	0.510	0.450	0.388	0.360
0.2	1.000	1.000	1.000	0.826	0.710	0.650	0.538	0.472	0.408	0.380
0.3	1.000	1.000	1.000	0.844	0.740	0.680	0.570	0.510	0.448	0.420
0.4	1.000	1.000	1.000	0.862	0.770	0.714	0.610	0.550	0.482	0.450
0.5	1.000	1.000	1.000	0.880	0.800	0.748	0.650	**0.590**	0.528	0.500
0.6	1.000	1.000	1.000	0.898	0.830	0.782	0.694	0.646	0.588	0.560
0.7	1.000	1.000	1.000	0.922	0.870	0.830	0.754	0.706	0.654	0.630
0.8	1.000	1.000	1.000	0.946	0.910	0.878	0.818	0.782	0.734	0.710
0.9	1.000	1.000	1.000	0.970	0.950	0.934	0.902	0.878	0.846	0.830
1.0	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000

Wall Input Values		
Height of Wall	h (ft)	9
Maximum Opening Height	h_o (ft)	7
Width of Wall (total)	b (ft)	59.25
Sum of Wall Opening Width	b_o (ft)	33

Calculate Perforation Reduction		
Max Opening Hgt / Wall Hgt	h / h_o	0.78
Shear Wall Remaining (piers)	b_{fh} (ft)	26.25
Pier length / wall length	b_{fh} / b	0.443
Perforation factor	**C_o (ft)**	**0.627**

*Perforation Factor is from Double Intertoplation of **C_o Table**

Hold-down Force Required		
Length between Hold-downs	L (ft)	59
Tension Force	**T (lbs)**	**947.29**

IBC Code: Can sum the sheathing values per side, but the summation of the two must be less that twice the lowest unit shear of the two materials.

Force Input Value		
Shear Force applied to Wall	V (lbs)	6210

Unit Shear Calculation		
Remaining Pier Length * C_o	b_{fh} * C_o (ft)	16.46
Unit Shear, V / Pier length	**v (lb/ft)**	**377.18**

Side 1: Sheathing: 1/2" OSB			
Length	L (ft)	16.46	
Unit Shear Capacity	v (lb/ft)	380	**8d Nail @ 4/12 Pattern**

Side 2: Sheathing: 1/2" Gyp. Board			
Length	L (ft)	16.46	
Unit Shear Capacity	v (lb/ft)	80	**#6Screw @ 8/12 pattern**

Total Force Capacity	**V (lbs)**	**7573.6**	> 6210	**O.K.**
Equivalent Unit Shear	**v (lb/ft)**	**460**	< 377.179	**O.K.**

Gypsum is screwed at 16" OC at edge joint along horizontal butt joints

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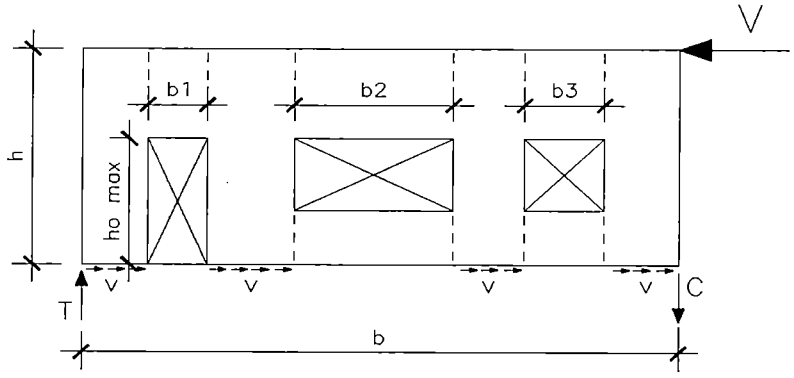
Job: Stratton Residence-Fox Point
Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

Perforated Shear Wall Worksheet SW-#D-1ST FLR



$b_o = b1 + b2 + b3$
$b_{fh} = b - b_o$
ho = height, maximum, for openings

V = Shear applied to Wall
v = Unit Shear (from Piers * Perforation Factor)

T = Tension Force for Holddown to System Below
C = Compression Force to System Below

** Diagram is for illustrative purposes only. See Elevation Sheets for shear wall being examined.

C_o Table

b_{fh} / b	h_0 / h 0.1	65.5	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
0.0	1.000	1.000	1.000	0.802	0.670	0.602	0.480	0.420	0.358	0.330
0.1	1.000	1.000	1.000	0.814	0.690	0.626	0.510	0.450	0.388	0.360
0.2	1.000	1.000	1.000	0.826	0.710	0.650	0.538	0.472	0.408	0.380
0.3	1.000	1.000	1.000	0.844	0.740	0.680	0.570	0.510	0.448	0.420
0.4	1.000	1.000	1.000	0.862	0.770	0.714	0.610	0.550	0.482	0.450
0.5	1.000	1.000	1.000	0.880	0.800	0.748	0.650	**0.590**	0.528	0.500
0.6	1.000	1.000	1.000	0.898	0.830	0.782	0.694	0.646	0.588	0.560
0.7	1.000	1.000	1.000	0.922	0.870	0.830	0.754	0.706	0.654	0.630
0.8	1.000	1.000	1.000	0.946	0.910	0.878	0.818	0.782	0.734	0.710
0.9	1.000	1.000	1.000	0.970	0.950	0.934	0.902	0.878	0.846	0.830
1.0	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000

Wall Input Values

Height of Wall	h (ft)	9
Maximum Opening Height	h_o (ft)	7
Width of Wall (total)	b (ft)	57.75
Sum of Wall Opening Width	b_o (ft)	29

Calculate Perforation Reduction

Max Opening Hgt / Wall Hgt	h / h_0	0.78
Shear Wall Remaining (piers)	b_{fh} (ft)	28.75
Pier length / wall length	b_{fh} / b	0.498
Perforation factor	**C_o (ft)**	**0.649**

*Perforation Factor is from Double Intertoplation of **C_o Table**

Hold-down Force Required

Length between Hold-downs	L (ft)	57.5
Tension Force	**T (lbs)**	**987.34**

IBC Code: Can sum the sheathing values per side, but the summation of the two must be less that twice the lowest unit shear of the two materials.

Force Input Value

Shear Force applied to Wall	V (lbs)	6308
Force applied by wind		

Unit Shear Calculation

Remaining Pier Length * C_o	b_{fh} * C_o (ft)	18.66
Unit Shear, V / Pier length	**v (lb/ft)**	**338.00**

Side 1: Sheathing: 1/2" OSB

Length		L (ft)	18.66	
Unit Shear Ca	50.42	v (lb/ft)	640	**8d Nail @ 2/12 Pattern**

Side 2: Sheathing: 1/2" Gyp. Board

Length		L (ft)	18.66	
Unit Shear Capacity		v (lb/ft)	80	**#6Screw @ 8/12 pattern**

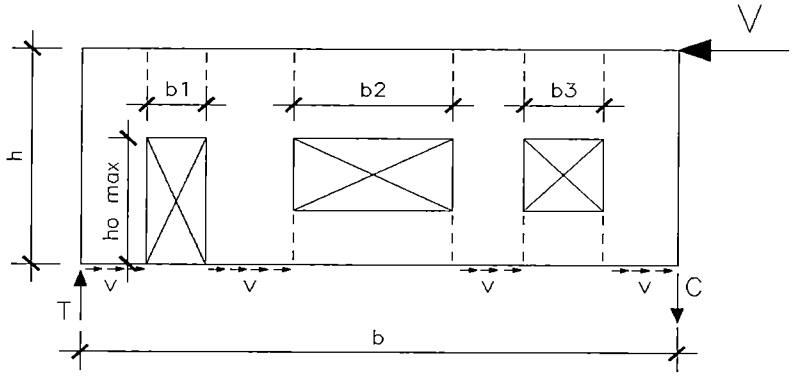
Total Force Capacity	**V (lbs)**	**13437.0**	> 6308	**O.K.**
Equivalent Unit Shear	**v (lb/ft)**	**720**	> 338.005	**O.K.**

Gypsum is screwed at 16" OC at edge joint along horizontal butt joints

TDI Associates, Inc.
Architects • Engineers • Planners

Job: Stratton Residence-Fox Point
Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

Perforated Shear Wall Worksheet SW-#E-1ST FLR



$b_o = b1 + b2 + b3$
$b_{fh} = b - b_o$
ho = height, maximum, for openings

V = Shear applied to Wall
v = Unit Shear (from Piers * Perforation Factor)

T = Tension Force for Holddown to System Below
C = Compression Force to System Below

** Diagram is for illustrative purposes only. See Elevation Sheets for shear wall being examined.

C_o Table

h_o / h

b_{fh} / b	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
0.0	1.000	1.000	1.000	0.802	0.670	0.602	0.480	0.420	0.358	0.330
0.1	1.000	1.000	1.000	0.814	0.690	0.626	0.510	0.450	0.388	0.360
0.2	1.000	1.000	1.000	0.826	0.710	0.650	0.538	0.472	0.408	0.380
0.3	1.000	1.000	1.000	0.844	0.740	0.680	0.570	0.510	0.448	0.420
0.4	1.000	1.000	1.000	0.862	0.770	0.714	0.610	0.550	0.482	0.450
0.5	1.000	1.000	1.000	0.880	0.800	0.748	0.650	0.590	0.528	0.500
0.6	1.000	1.000	1.000	0.898	0.830	0.782	0.694	0.646	0.588	0.560
0.7	1.000	1.000	1.000	0.922	0.870	0.830	0.754	0.706	0.654	0.630
0.8	1.000	1.000	1.000	0.946	0.910	0.878	0.818	0.782	0.734	0.710
0.9	1.000	1.000	1.000	0.970	0.950	0.934	0.902	0.878	0.846	0.830
1.0	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000

Wall Input Values		
Height of Wall	h (ft)	9
Maximum Opening Height	h_o (ft)	7
Width of Wall (total)	b (ft)	89.75
Sum of Wall Opening Width	b_o (ft)	33

Calculate Perforation Reduction		
Max Opening Hgt / Wall Hgt	h / h_o	0.78
Shear Wall Remaining (piers)	b_{fh} (ft)	56.75
Pier length / wall length	b_{fh} / b	0.632
Perforation factor	**C_o (ft)**	**0.713**

*Perforation Factor is from Double Intertoplation of **C_o Table**

Hold-down Force Required		
Length between Hold-downs	L (ft)	89.5
Tension Force	**T (lbs)**	**473.13**

IBC Code: Can sum the sheathing values per side, but the summation of the two must be less that twice the lowest unit shear of the two materials.

Force Input Value		
Shear Force applied to Wall	V (lbs)	4705
Force applied by wind		

Unit Shear Calculation		
Remaining Pier Length * C_o	b_{fh} * C_o (ft)	40.48
Unit Shear, V / Pier length	**v (lb/ft)**	**116.22**
8		

Side 1: Sheathing: 1/2" Gyp. Board			
Length	L (ft)	40.48	
Unit Shear Capacity	v (lb/ft)	80	**#6 Screw @ 8/12 Pattern**

Side 2: Sheathing: 1/2" Gyp. Board			
Length	L (ft)	40.48	
Unit Shear Capacity	v (lb/ft)	80	**#6 Screw @ 8/12 Pattern**

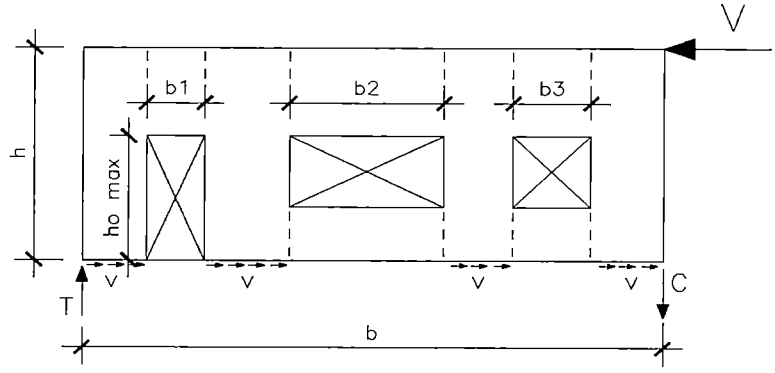
Total Force Capacity	**V (lbs)**	**6477.5**	> 4705	**O.K.**
Equivalent Unit Shear	**v (lb/ft)**	**160**	> 116.217	**O.K.**

Gypsum is screwed at 16" OC at edge joint along horizontal butt joints

TDI Associates, Inc.
Architects • Engineers • Planners

Job: Stratton Residence-Fox Point
Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

Perforated Shear Wall Worksheet SW-#F-1ST FLR



b_o = b1 + b2 +b3
b_{th} = b - b_o
ho = height, maximum, for openings

V = Shear applied to Wall
v = Unit Shear (from Piers * Perforation Factor)

T = Tension Force for Holddown to System Below
C = Compression Force to System Below

** Diagram is for illustrative purposes only. See Elevation Sheets for shear wall being examined.

C_o Table

h_o / h

b_{th} / b	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
0.0	1.000	1.000	1.000	0.802	0.670	0.602	0.480	0.420	0.358	0.330
0.1	1.000	1.000	1.000	0.814	0.690	0.626	0.510	0.450	0.388	0.360
0.2	1.000	1.000	1.000	0.826	0.710	0.650	0.538	0.472	0.408	0.380
0.3	1.000	1.000	1.000	0.844	0.740	0.680	0.570	0.510	0.448	0.420
0.4	1.000	1.000	1.000	0.862	0.770	0.714	0.610	0.550	0.482	0.450
0.5	1.000	1.000	1.000	0.880	0.800	0.748	0.650	0.590	0.528	0.500
0.6	1.000	1.000	1.000	0.898	0.830	0.782	0.694	0.646	0.588	0.560
0.7	1.000	1.000	1.000	0.922	0.870	0.830	0.754	0.706	0.654	0.630
0.8	1.000	1.000	1.000	AAAA	0.910	0.878	0.818	0.782	0.734	0.710
0.9	1.000	1.000	1.000	0.970	0.950	0.934	0.902	0.878	0.846	0.830
1.0	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000

Wall Input Values		
Height of Wall	h (ft)	9
Maximum Opening Height	h_o (ft)	7
Width of Wall (total)	b (ft)	57.75
Sum of Wall Opening Width	b_o (ft)	40

Calculate Perforation Reduction		
Max Opening Hgt / Wall Hgt	h / h_o	0.78
Shear Wall Remaining (piers)	b_{th} (ft)	17.75
Pier length / wall length	b_{th} / b	0.307
Perforation factor	**C_o (ft)**	**0.573**

*Perforation Factor is from Double Intertoplation of **C_o Table**

Hold-down Force Required		
Length between Hold-downs	L (ft)	57.5
Tension Force	**T (lbs)**	**987.34**

IBC Code: Can sum the sheathing values per side, but the summation of the two must be less that twice the lowest unit shear of the two materials.

Force Input Value		
Shear Force applied to Wall	V (lbs)	6308

Unit Shear Calculation		
Remaining Pier Length * C_o	b_{th} * C_o (ft)	10.17
Unit Shear, V / Pier length	**v (lb/ft)**	**620.27**

Side 1: Sheathing: 1/2" OSB			
Length	L (ft)	10.17	
Unit Shear Capacity	v (lb/ft)	640	**8d Nail @ 2/12 Pattern**

Side 2: Sheathing: 1/2" Gyp. Board			
Length	L (ft)	10.17	
Unit Shear Capacity	v (lb/ft)	80	**#6 Screw @ 8/12 Pattern**

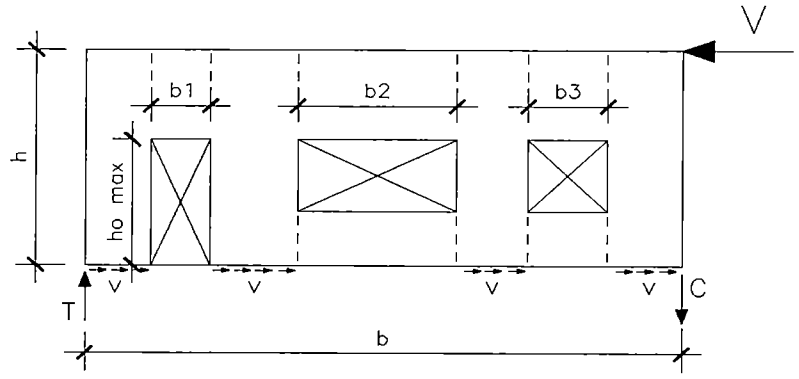
Total Force Capacity	**V (lbs)**	**7322.2**	> 6308	**O.K.**
Equivalent Unit Shear	**v (lb/ft)**	**720**	> 620.272	**O.K.**

Gypsum is screwed at 16" OC at edge joint along horizontal butt joints

TDI Associates, Inc.
Architects • Engineers • Planners

Job: Stratton Residence-Fox Point
Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

Perforated Shear Wall Worksheet SW-#G-2ND FLR



$b_o = b1 + b2 + b3$
$b_{fh} = b - b_o$
ho = height, maximum, for openings

V = Shear applied to Wall
v = Unit Shear (from Piers * Perforation Factor)

T = Tension Force for Holddown to System Below
C = Compression Force to System Below

** Diagram is for illustrative purposes only. See Elevation Sheets for shear wall being examined.

C_o Table

b_{fh} / b	h_o / h 0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
0.0	1.000	1.000	1.000	0.802	0.670	0.602	0.480	0.420	0.358	0.330
0.1	1.000	1.000	1.000	0.814	0.690	0.626	0.510	0.450	0.388	0.360
0.2	1.000	1.000	1.000	0.826	0.710	0.650	0.538	0.472	0.408	0.380
0.3	1.000	1.000	1.000	0.844	0.740	0.680	0.570	0.510	0.448	0.420
0.4	1.000	1.000	1.000	0.862	0.770	0.714	0.610	0.550	0.482	0.450
0.5	1.000	1.000	1.000	0.880	0.800	0.748	0.650	**0.590**	0.528	0.500
0.6	1.000	1.000	1.000	0.898	0.830	0.782	0.694	0.646	0.588	0.560
0.7	1.000	1.000	1.000	0.922	0.870	0.830	0.754	0.706	0.654	0.630
0.8	1.000	1.000	1.000	0.946	0.910	0.878	0.818	0.782	0.734	0.710
0.9	1.000	1.000	1.000	0.970	0.950	0.934	0.902	0.878	0.846	0.830
1.0	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000

Wall Input Values		
Height of Wall	h (ft)	8
Maximum Opening Height	h_o (ft)	5
Width of Wall (total)	b (ft)	59.25
Sum of Wall Opening Width	b_o (ft)	22

Calculate Perforation Reduction		
Max Opening Hgt / Wall Hgt	h / h_o	0.63
Shear Wall Remaining (piers)	b_{fh} (ft)	37.25
Pier length / wall length	b_{fh} / b	0.629
Perforation factor	**C_o (ft)**	**0.775**

*Perforation Factor is from Double Intertoplation of **C_o Table**

Hold-down Force Required		
Length between Hold-downs	L (ft)	59
Tension Force	**T (lbs)**	**485.29**

IBC Code: Can sum the sheathing values per side, but the summation of the two must be less that twice the lowest unit shear of the two materials.

Force Input Value		
Shear Force applied to Wall	V (lbs)	3579

Unit Shear Calculation		
Remaining Pier Length * C_o	b_{fh} * C_o (ft)	28.86
75 Unit Shear, V / Pier length	**v (lb/ft)**	**124.03**

Side 1: Sheathing: 1/2" OSB			
Length	L (ft)	28.86	
Unit Shear Capacity	v (lb/ft)	260	**8d Nail @ 6/12 Pattern**

Side 2: Sheating 1/2" GYP			
Length	L (ft)	28.86	
Unit Shear Capacity	v (lb/ft)	80	**#6Screw @ 8/12 pattern**

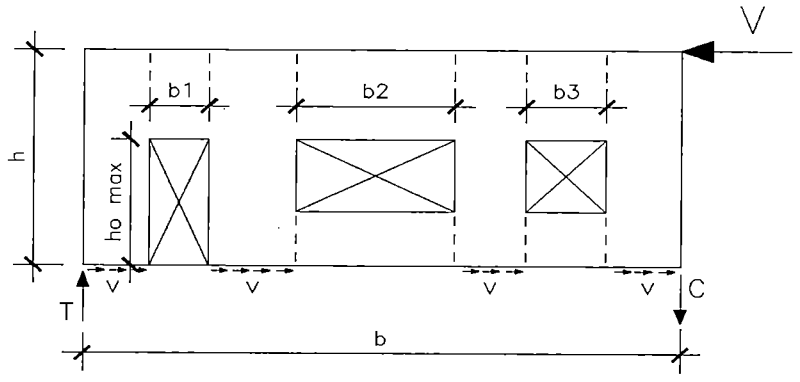
Total Force Capacity	**V (lbs)**	**9810.7**	> 3579	**O.K.**
Equivalent Unit Shear	**v (lb/ft)**	**340**	> 124.034	**O.K.**

Gypsum is screwed at 16" OC at edge joint along horizontal butt joints

TDI Associates, Inc.
Architects • Engineers • Planners

Job: Stratton Residence-Fox Point
Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

Perforated Shear Wall Worksheet SW-#H-2ND FLR



b_o = b1 + b2 +b3
b_{fh} = b - b_o
ho = height, maximum, for openings

V = Shear applied to Wall
v = Unit Shear (from Piers * Perforation Factor)

T = Tension Force for Holddown to System Below
C = Compression Force to System Below

** Diagram is for illustrative purposes only. See Elevation Sheets for shear wall being examined.

C_o Table

h_o / h

b_{fh} / b	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
0.0	1.000	1.000	1.000	0.802	0.670	0.602	0.480	0.420	0.358	0.330
0.1	1.000	1.000	1.000	0.814	0.690	0.626	0.510	0.450	0.388	0.360
0.2	1.000	1.000	1.000	0.826	0.710	0.650	0.538	0.472	0.408	0.380
0.3	1.000	1.000	1.000	0.844	0.740	0.680	0.570	0.510	0.448	0.420
0.4	1.000	1.000	1.000	0.862	0.770	0.714	0.610	0.550	0.482	0.450
0.5	1.000	1.000	1.000	0.880	0.800	0.748	0.650	0.590	0.528	0.500
0.6	1.000	1.000	1.000	0.898	0.830	0.782	0.694	0.646	0.588	0.560
0.7	1.000	1.000	1.000	0.922	0.870	0.830	0.754	0.706	0.654	0.630
0.8	1.000	1.000	1.000	0.946	0.910	0.878	0.818	0.782	0.734	0.710
0.9	1.000	1.000	1.000	0.970	0.950	0.934	0.902	0.878	0.846	0.830
1.0	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000

Wall Input Values		
Height of Wall	h (ft)	8
Maximum Opening Height	h_o (ft)	7
Width of Wall (total)	b (ft)	72.75
Sum of Wall Opening Width	b_o (ft)	15

Calculate Perforation Reduction		
Max Opening Hgt / Wall Hgt	h / h_o	0.88
Shear Wall Remaining (piers)	b_{fh} (ft)	57.75
Pier length / wall length	b_{fh} / b	0.794
Perforation factor	**C_o (ft)**	**0.741**

*Perforation Factor is from Double Intertoplation of **C_o Table**

Hold-down Force Required		
Length between Hold-downs	L (ft)	72.5
Tension Force	**T (lbs)**	**278.51**

IBC Code: Can sum the sheathing values per side, but the summation of the two must be less that twice the lowest unit shear of the two materials.

Force Input Value		
Shear Force applied to Wall	V (lbs)	2524

Unit Shear Calculation		
Remaining Pier Length * C_o	b_{fh} * C_o (ft)	42.80
Unit Shear, V / Pier length	**v (lb/ft)**	**58.97**

Side 1: Sheathing: 1/2" Gyp. Board			
Length	L (ft)	42.80	
Unit Shear Capacity	v (lb/ft)	80	**#6 Screw @ 8/12 Pattern**

Side 2: sheating 1/2" gyp board			
Length	L (ft)	42.80	
Unit Shear Capacity	v (lb/ft)	80	**#6 Screw @ 8/12 Pattern**

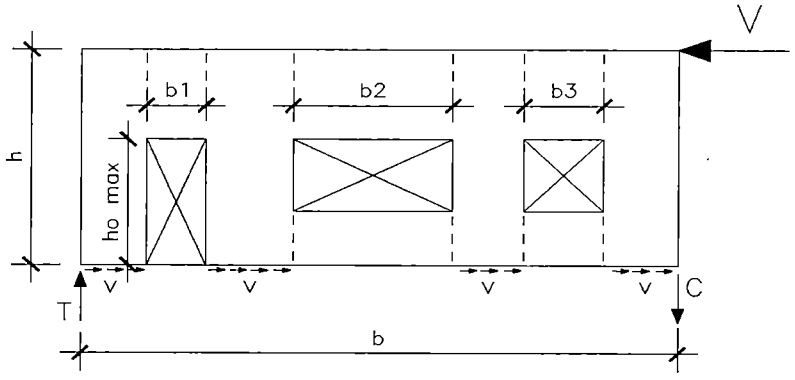
Total Force Capacity	**V (lbs)**	**6847.8**	> 2524	**O.K.**
Equivalent Unit Shear	**v (lb/ft)**	**160**	> 58.974	**O.K.**

Gypsum is screwed at 16" OC at edge joint along horizontal butt joints

TDI Associates, Inc.
Architects • Engineers • Planners

Job: Stratton Residence-Fox Point
Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

Perforated Shear Wall Worksheet SW-#I-2ND FLR



$b_o = b1 + b2 + b3$

$b_{fh} = b - b_o$

ho = height, maximum, for openings

V = Shear applied to Wall

v = Unit Shear (from Piers * Perforation Factor)

T = Tension Force for Holddown to System Below
C = Compression Force to System Below

** Diagram is for illustrative purposes only. See Elevation Sheets for shear wall being examined.

C_o Table

h_o / h

b_{fh} / b	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
0.0	1.000	1.000	1.000	0.802	0.670	0.602	0.480	0.420	0.358	0.330
0.1	1.000	1.000	1.000	0.814	0.690	0.626	0.510	0.450	0.388	0.360
0.2	1.000	1.000	1.000	0.826	0.710	0.650	0.538	0.472	0.408	0.380
0.3	1.000	1.000	1.000	0.844	0.740	0.680	0.570	0.510	0.448	0.420
0.4	1.000	1.000	1.000	0.862	0.770	0.714	0.610	0.550	0.482	0.450
0.5	1.000	1.000	1.000	0.880	0.800	0.748	0.650	**0.590**	0.528	0.500
0.6	1.000	1.000	1.000	0.898	0.830	0.782	0.694	0.646	0.588	0.560
0.7	1.000	1.000	1.000	0.922	0.870	0.830	0.754	0.706	0.654	0.630
0.8	1.000	1.000	1.000	0.946	0.910	0.878	0.818	0.782	0.734	0.710
0.9	1.000	1.000	1.000	0.970	0.950	0.934	0.902	0.878	0.846	0.830
1.0	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000

Wall Input Values

Height of Wall	h (ft)	8
Maximum Opening Height	h_o (ft)	5
Width of Wall (total)	b (ft)	59.25
Sum of Wall Opening Width		31

Calculate Perforation Reduction

Max Opening Hgt / Wall Hgt	h / h_o	0.63
Shear Wall Remaining (piers)	b_{fh} (ft)	28.25
Pier length / wall length	b_{fh} / b	0.477
Perforation factor	**C_o (ft)**	**0.715**

*Perforation Factor is from Double Intertoplation of **C_o Table**

Hold-down Force Required

Length between Hold-downs	L (ft)	59
Tension Force	**T (lbs)**	**0.00**

IBC Code: Can sum the sheathing values per side, but the summation of the two must be less that twice the lowest unit shear of the two materials.

Force Input Value

Shear Force applied to Wall	V (lbs)	3579

Unit Shear Calculation

Remaining Pier Length * C_o	b_{fh} * C_o (ft)	20.21
Unit Shear, V / Pier length	**v (lb/ft)**	**177.13**

Side 1: Sheathing: 1/2" OSB

Length	L (ft)	20.21	
Unit Shear Capacity	v (lb/ft)	260	**8d Nail @ 6/12 Pattern**

Side 2: Sheathing: 1/2" Gyp. Board

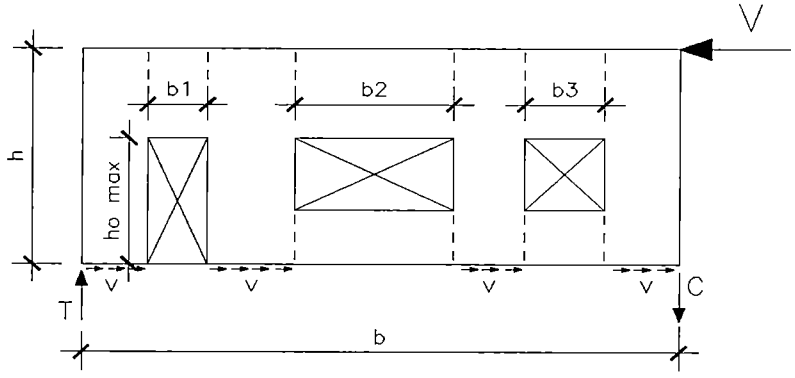
Length	L (ft)	20.21	
Unit Shear Capacity	v (lb/ft)	80	**#6Screw @ 8/12 pattern**
Total Force Capacity	**V (lbs)**	**6870.1**	> **O.K.**
Equivalent Unit Shear	**v (lb/ft)**	**340**	> 177.126 **O.K.**

Gypsum is screwed at 16" OC at edge joint along horizontal butt joints

TDI Associates, Inc.
Architects • Engineers • Planners

Job: Stratton Residence-Fox Point
Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

Perforated Shear Wall Worksheet SW-#J-2ND FLR



b_o = b1 + b2 +b3
b_{fh} = b - b_o
ho = height, maximum, for openings

V = Shear applied to Wall
v = Unit Shear (from Piers * Perforation Factor)

T = Tension Force for Holddown to System Below
C = Compression Force to System Below

** Diagram is for illustrative purposes only. See Elevation Sheets for shear wall being examined.

C_o Table

h_o / h

b_{fh} / b	0.1	65.5	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
0.0	1.000	1.000	1.000	0.802	0.670	0.602	0.480	0.420	0.358	0.330
0.1	1.000	1.000	1.000	0.814	0.690	0.626	0.510	0.450	0.388	0.360
0.2	1.000	1.000	1.000	0.826	0.710	0.650	0.538	0.472	0.408	0.380
0.3	1.000	1.000	1.000	0.844	0.740	0.680	0.570	0.510	0.448	0.420
0.4	1.000	1.000	1.000	0.862	0.770	0.714	0.610	0.550	0.482	0.450
0.5	1.000	1.000	1.000	0.880	0.800	0.748	0.650	**0.590**	0.528	0.500
0.6	1.000	1.000	1.000	0.898	0.830	0.782	0.694	0.646	0.588	0.560
0.7	1.000	1.000	1.000	0.922	0.870	0.830	0.754	0.706	0.654	0.630
0.8	1.000	1.000	1.000	0.946	0.910	0.878	0.818	0.782	0.734	0.710
0.9	1.000	1.000	1.000	0.970	0.950	0.934	0.902	0.878	0.846	0.830
1.0	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000

Wall Input Values		
Height of Wall	h (ft)	8
Maximum Opening Height	h_o (ft)	5
Width of Wall (total)	b (ft)	57.75
Sum of Wall Opening Width	b_o (ft)	5

Calculate Perforation Reduction		
Max Opening Hgt / Wall Hgt	h / h_0	0.63
Shear Wall Remaining (piers)	b_{fh} (ft)	52.75
Pier length / wall length	b_{fh} / b	0.913
Perforation factor	**C_o (ft)**	**0.936**

*Perforation Factor is from Double Intertoplation of **C_o Table**

Hold-down Force Required		
Length between Hold-downs	L (ft)	57.5
Tension Force	**T (lbs)**	**501.70**

IBC Code: Can sum the sheathing values per side, but the summation of the two must be less that twice the lowest unit shear of the two materials.

Force Input Value		
Shear Force applied to Wall	V (lbs)	3606

Unit Shear Calculation		
Remaining Pier Length * C_o	b_{fh} * C_o (ft)	49.37
Unit Shear, V / Pier length	**v (lb/ft)**	**73.04**

Side 1: Sheathing: 1/2" OSB			
Length	L (ft)	49.37	
Unit Shear Capacity	v (lb/ft)	260	**8d Nail @ 6/12 Pattern**

Side 2: Sheathing: 1/2" Gyp. Board			
Length	L (ft)	49.37	
Unit Shear Capacity	v (lb/ft)	80	**#6Screw @ 8/12 pattern**

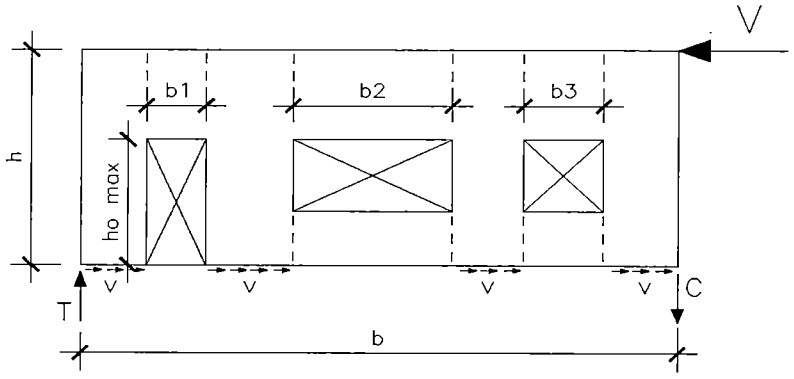
Total Force Capacity	**V (lbs)**	**16785.9**	> 3606	**O.K.**
Equivalent Unit Shear	**v (lb/ft)**	**340**	> 73.04	**O.K.**

Gypsum is screwed at 16" OC at edge joint along horizontal butt joints

TDI Associates, Inc.
Architects • Engineers • Planners

Job: Stratton Residence-Fox Point
Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

Perforated Shear Wall Worksheet SW-#K-2ND FLR



b_o = b1 + b2 +b3
b_{fh} = b - b_o
ho = height, maximum, for openings

V = Shear applied to Wall
v = Unit Shear (from Piers * Perforation Factor)

T = Tension Force for Holddown to System Below
C = Compression Force to System Below

** Diagram is for illustrative purposes only. See Elevation Sheets for shear wall being examined.

C_o Table

h_o / h

b_{fh} / b	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
0.0	1.000	1.000	1.000	0.802	0.670	0.602	0.480	0.420	0.358	0.330
0.1	1.000	1.000	1.000	0.814	0.690	0.626	0.510	0.450	0.388	0.360
0.2	1.000	1.000	1.000	0.826	0.710	0.650	0.538	0.472	0.408	0.380
0.3	1.000	1.000	1.000	0.844	0.740	0.680	0.570	0.510	0.448	0.420
0.4	1.000	1.000	1.000	0.862	0.770	0.714	0.610	0.550	0.482	0.450
0.5	1.000	1.000	1.000	0.880	0.800	0.748	0.650	0.590	0.528	0.500
0.6	1.000	1.000	1.000	0.898	0.830	0.782	0.694	0.646	0.588	0.560
0.7	1.000	1.000	1.000	0.922	0.870	0.830	0.754	0.706	0.654	0.630
0.8	1.000	1.000	1.000	0.946	0.910	0.878	0.818	0.782	0.734	0.710
0.9	1.000	1.000	1.000	0.970	0.950	0.934	0.902	0.878	0.846	0.830
1.0	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000

Wall Input Values		
Height of Wall	h (ft)	8
Maximum Opening Height	h_o (ft)	7
Width of Wall (total)	b (ft)	105
Sum of Wall Opening Width	b_o (ft)	24

Calculate Perforation Reduction		
Max Opening Hgt / Wall Hgt	h / h_o	0.88
Shear Wall Remaining (piers)	b_{fh} (ft)	81
Pier length / wall length	b_{fh} / b	0.771
Perforation factor	**C_o (ft)**	**0.723**

*Perforation Factor is from Double Intertoplation of **C_o Table**

Hold-down Force Required		
Length between Hold-downs	L (ft)	104.75
Tension Force	**T (lbs)**	**194.52**

IBC Code: Can sum the sheathing values per side, but the summation of the two must be less that twice the lowest unit shear of the two materials.

Force Input Value		
Shear Force applied to Wall	V (lbs)	2547
Force applied by wind		

Unit Shear Calculation		
Remaining Pier Length * C_o	b_{fh} * C_o (ft)	58.60
Unit Shear, V / Pier length 8	**v (lb/ft)**	**43.47**

Side 1: Sheathing: 1/2" Gyp. Board			
Length	L (ft)	58.60	
Unit Shear Capacity	v (lb/ft)	80	**#6 Screw @ 8/12 Pattern**

Side 2: Sheathing: 1/2" Gyp. Board			
Length	L (ft)	58.60	
Unit Shear Capacity	v (lb/ft)	80	**#6 Screw @ 8/12 Pattern**

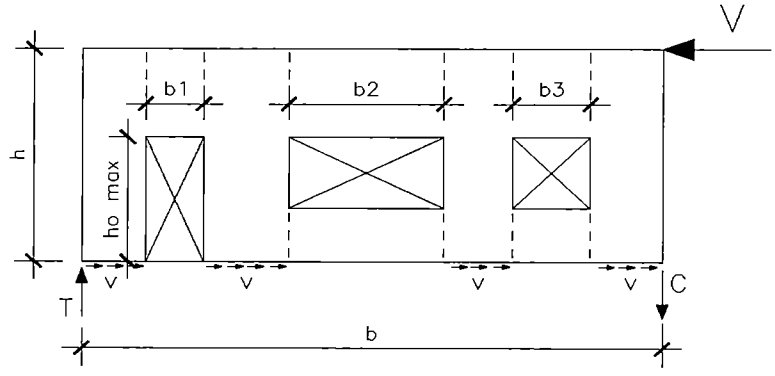
Total Force Capacity	**V (lbs)**	**9375.6**	> 2547	**O.K.**
Equivalent Unit Shear	**v (lb/ft)**	**160**	> 43.467	**O.K.**

Gypsum is screwed at 16" OC at edge joint along horizontal butt joints

TDI Associates, Inc.
Architects • Engineers • Planners

Job: Stratton Residence-Fox Point
Project No.: 13-159
Date: May 4, 2013 Sheet_____of_____

Perforated Shear Wall Worksheet SW-#L-2ND FLR



b_o = b1 + b2 +b3
b_{fh} = b - b_o
ho = height, maximum, for openings

V = Shear applied to Wall
v = Unit Shear (from Piers * Perforation Factor)

T = Tension Force for Holddown to System Below
C = Compression Force to System Below

** Diagram is for illustrative purposes only. See Elevation Sheets for shear wall being examined.

C_o Table

	h_o / h									
b_{fh} / b	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0
0.0	1.000	1.000	1.000	0.802	0.670	0.602	0.480	0.420	0.358	0.330
0.1	1.000	1.000	1.000	0.814	0.690	0.626	0.510	0.450	0.388	0.360
0.2	1.000	1.000	1.000	0.826	0.710	0.650	0.538	0.472	0.408	0.380
0.3	1.000	1.000	1.000	0.844	0.740	0.680	0.570	0.510	0.448	0.420
0.4	1.000	1.000	1.000	0.862	0.770	0.714	0.610	0.550	0.482	0.450
0.5	1.000	1.000	1.000	0.880	0.800	0.748	0.650	0.590	0.528	0.500
0.6	1.000	1.000	1.000	0.898	0.830	0.782	0.694	0.646	0.588	0.560
0.7	1.000	1.000	1.000	0.922	0.870	0.830	0.754	0.706	0.654	0.630
0.8	1.000	1.000	1.000	0.946	0.910	0.878	0.818	0.782	0.734	0.710
0.9	1.000	1.000	1.000	0.970	0.950	0.934	0.902	0.878	0.846	0.830
1.0	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000	1.000

Wall Input Values		
Height of Wall	h (ft)	8
Maximum Opening Height	h_o (ft)	5
Width of Wall (total)	b (ft)	57.75
Sum of Wall Opening Width	b_o (ft)	17

Calculate Perforation Reduction		
Max Opening Hgt / Wall Hgt	h / h_o	0.63
Shear Wall Remaining (piers)	b_{fh} (ft)	40.75
Pier length / wall length	b_{fh} / b	0.706
Perforation factor	**C_o (ft)**	**0.802**

*Perforation Factor is from Double Intertoplation of **C_o Table**

Hold-down Force Required		
Length between Hold-downs	L (ft)	57.5
Tension Force	**T (lbs)**	**501.70**

IBC Code: Can sum the sheathing values per side, but the summation of the two must be less that twice the lowest unit shear of the two materials.

Force Input Value		
Shear Force applied to Wall	V (lbs)	3606

Unit Shear Calculation		
Remaining Pier Length * C_o	b_{fh} * C_o (ft)	32.69
Unit Shear, V / Pier length	**v (lb/ft)**	**110.32**

Side 1: Sheathing: 1/2" OSB			
Length	L (ft)	32.69	
Unit Shear Capacity	v (lb/ft)	80	**8d Nail @ 6/12 Pattern**

Side 2: Sheathing: 1/2" Gyp. Board			
Length	L (ft)	32.69	
Unit Shear Capacity	v (lb/ft)	80	**#6 Screw @ 8/12 Pattern**

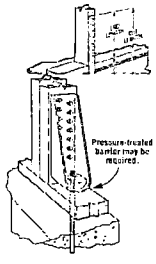
Total Force Capacity	**V (lbs)**	**5229.7**	> 3606	**O.K.**
Equivalent Unit Shear	**v (lb/ft)**	**160**	> 110.325	**O.K.**

Gypsum is screwed at 16" OC at edge joint along horizontal butt joints

III. Lateral Force Design

B. Roof Diaphragms and Continuous Load Path for Uplift

Height of wall-ft 1st	9						
Height of wall-ft 1st	8	Total Wind Shear force	10422.22				
Length pf wall-ft 1st	59.25						
Length pf wall-ft 1st	59.25	Design for Diaphragms					
Wind Force max-psf	11.50						
Height of roof	10.5	Worst Case Force = Shear F/ width	175.9 plf	less than	180	plf	Ok
Length of roof	59.25						
Wind force max	7.90	At Roof & Floor Diaphragms, Use 1/2" OSB Sheathing w/ 6/12 Nailing					
Bldg Length	59.25	Pattern with 8d Nails.					



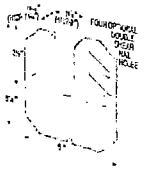
Hold-Downs at First Floor

Tension Force Worst Case=	788.0 lbs	From Shear Wall	
HLLT4-SDS3	Ft Allow=	3375 lbs	Ok

Use Simpson HLLT4-SDS with (10) SDS 1/4x3 screws & 5/8" Dia AB or Simpson 5/8" Dia.x8" Titen HD Anchor in 4000 psi NEW Concrete.

	Dead Load=psf	15
Continuous Load Path - Uplift Design, Truss	Bldg width-ft	36
	Spacing of trusses-ft	2

Area of Roof Truss =	spacing * 1/2 width	36.0	sq. ft.
By Components & Cladding:	Ft=	-19.5	psf, by interpolation
Uplifting force, Ft=	Ft-.6 DL=	-10.5	psf



At Hold-Down:	ıss span/2 x spacing	-378 lbs	
Simpson H5:	Ft Allow=	780 lbs	Ok
Check Shear:	Worst Case =	351.805 lbs	
Simpson H5:	Ft Allow=	455 lbs	Ok

Use Simpson H5 with (4) 8d Nails at Every Truss Bearing End and to plate.

Anchorage to Foundation

Walls except garage overhead door wall

Force Worst Case =	378	plf	Worst case From Shear Wall
Simpson Titen HD =	1950 lbs		
		5.16	ft. Spacing

Direct Wind Pressure= -14.4*4.5' 51.75 psf

Shear Force= SQRT (J13^2+J9^2) 381.526 plf

5.11 ft. Spacing

Use 1/2" dia AB or Simpson 1/2"Diax6" Titen HD Anchors @ 4'-0" O.C. at walls wiyhout OH drs

Garage overhead door wall

Force Worst Case =	621	plf	Worst case From Shear Wall
Simpson Titen HD =	1950 lbs		
		3.14	ft. Spacing

Direct Wind Pressure= -14.4*4.5' 51.75 psf

Shear Force= SQRT (J13^2+J9^2) 623.153 plf

3.13 ft. Spacing

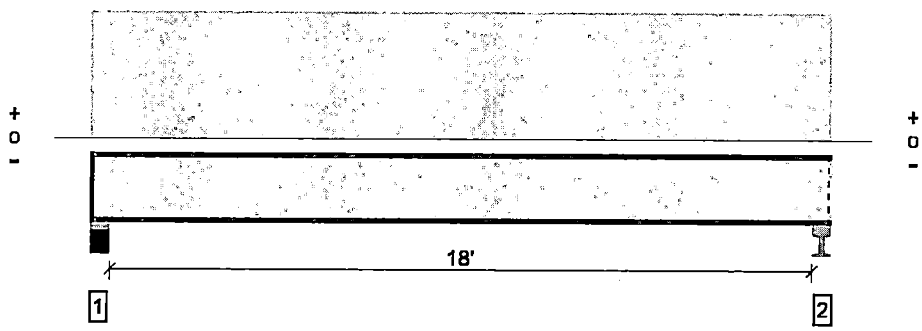
01: First Floor-Floor Framing			
Member Name	**Results**	**Current Solution**	**Comments**
Floor: Joist J-1	Passed	1 Piece(s) 11 7/8" TJI® 230 @ 16" OC	
Floor: Joist J-2	Passed	1 Piece(s) 11 7/8" TJI® 230 @ 16" OC	
Floor: Joist J-3	Passed	1 Piece(s) 11 7/8" TJI® 230 @ 16" OC	
Floor: Joist J-4	Passed	1 Piece(s) 11 7/8" TJI® 230 @ 16" OC	
Floor: Joist J-5	Passed	1 Piece(s) 11 7/8" TJI® 230 @ 16" OC	
Floor: Joist J-6	Passed	1 Piece(s) 11 7/8" TJI® 560 @ 16" OC	
Optional Floor: Joist J-6	Passed	1 Piece(s) 11 7/8" TJI® 230 @ 12" OC	
02: second Floor-Roof support headers			
Member Name	**Results**	**Current Solution**	**Comments**
Wall: Header H-201	Passed	2 Piece(s) 2 x 6 Spruce-Pine-Fir No. 1 / No. 2	
Wall: Header H-202	Passed	2 Piece(s) 2 x 6 Spruce-Pine-Fir No. 1 / No. 2	
Wall: Header H-203	Passed	2 Piece(s) 2 x 6 Spruce-Pine-Fir No. 1 / No. 2	
Wall: Header H-204	Passed	2 Piece(s) 2 x 12 Spruce-Pine-Fir No. 1 / No. 2	
Wall: Header H-205	Passed	2 Piece(s) 1 3/4" x 9 1/2" 2.0E Parallam® PSL	
Wall: Header H-206	Passed	2 Piece(s) 1 3/4" x 9 1/2" 2.0E Parallam® PSL	
Wall: Header H-207	Passed	2 Piece(s) 2 x 4 Spruce-Pine-Fir No. 1 / No. 2	
Wall: Header H-208	Passed	2 Piece(s) 2 x 4 Spruce-Pine-Fir No. 1 / No. 2	
Wall: Header H-209	Passed	2 Piece(s) 1 3/4" x 9 1/2" 2.0E Parallam® PSL	
Wall: Header H-210	Passed	2 Piece(s) 1 3/4" x 9 1/2" 2.0E Parallam® PSL	
Wall: Header H-211	Passed	2 Piece(s) 2 x 6 Spruce-Pine-Fir No. 1 / No. 2	
Roof: Flush Beam H-212	Passed	3 Piece(s) 1 3/4" x 11 7/8" 2.0E Parallam® PSL	
Roof: Flush Beam H-213	Passed	2 Piece(s) 1 3/4" x 11 7/8" 2.0E Parallam® PSL	
Roof: Flush Beam H-214	Passed	1 Piece(s) 1 3/4" x 11 7/8" 2.0E Parallam® PSL	
Wall: Header H-215	Passed	2 Piece(s) 2 x 10 Spruce-Pine-Fir No. 1 / No. 2	
03: second floor-Floor support			
Member Name	**Results**	**Current Solution**	**Comments**
Wall: Header H-1	Passed	2 Piece(s) 1 3/4" x 11 7/8" 2.0E Parallam® PSL	
Wall: Header H-2	Passed	2 Piece(s) 2 x 12 Spruce-Pine-Fir No. 1 / No. 2	
Wall: Header H-3	Passed	2 Piece(s) 2 x 12 Spruce-Pine-Fir No. 1 / No. 2	
Wall: Header H-4	Passed	2 Piece(s) 2 x 6 Spruce-Pine-Fir No. 1 / No. 2	
Wall: Header H-5	Passed	2 Piece(s) 2 x 6 Spruce-Pine-Fir No. 1 / No. 2	
Wall: Header H-6	Passed	2 Piece(s) 2 x 8 Spruce-Pine-Fir No. 1 / No. 2	
Wall: Header H-7	Passed	2 Piece(s) 2 x 8 Spruce-Pine-Fir No. 1 / No. 2	
Wall: Header H-8	Passed	2 Piece(s) 1 3/4" x 11 7/8" 2.0E Parallam® PSL	
Wall: Header H-9	Passed	2 Piece(s) 1 3/4" x 11 7/8" 2.0E Parallam® PSL	
Wall: Header H-10	Passed	2 Piece(s) 1 3/4" x 11 7/8" 2.0E Parallam® PSL	
Wall: Header H-11	Passed	2 Piece(s) 2 x 8 Spruce-Pine-Fir No. 1 / No. 2	
Wall: Header H-12	Passed	2 Piece(s) 2 x 12 Spruce-Pine-Fir No. 1 / No. 2	
Wall: Header H-13	Passed	2 Piece(s) 2 x 6 Spruce-Pine-Fir No. 1 / No. 2	
Wall: Header H-14	Passed	2 Piece(s) 1 3/4" x 11 7/8" 2.0E Parallam® PSL	
Wall: Header H-15	Passed	2 Piece(s) 1 3/4" x 9 1/2" 2.0E Parallam® PSL	
Wall: Header H-16	Passed	2 Piece(s) 1 3/4" x 11 7/8" 2.0E Parallam® PSL	
Wall: Header H-17	Passed	2 Piece(s) 2 x 8 Spruce-Pine-Fir No. 1 / No. 2	
Wall: Header H-18	Passed	2 Piece(s) 1 3/4" x 11 7/8" 2.0E Parallam® PSL	
Wall: Header H-19	Passed	2 Piece(s) 2 x 10 Spruce-Pine-Fir No. 1 / No. 2	
Wall: Header H-20	Passed	2 Piece(s) 2 x 10 Spruce-Pine-Fir No. 1 / No. 2	
Floor: Flush Beam H-21	Passed	2 Piece(s) 1 3/4" x 11 7/8" 2.0E Parallam® PSL	
Floor: Flush Beam H-22	Passed	2 Piece(s) 1 3/4" x 11 7/8" 2.0E Parallam® PSL	
Floor: Flush Beam H-23	Passed	2 Piece(s) 1 3/4" x 11 7/8" 2.0E Parallam® PSL	
Floor: Flush Beam H-24	Passed	2 Piece(s) 1 3/4" x 11 7/8" 2.0E Parallam® PSL	
Floor: Flush Beam H-25	Passed	2 Piece(s) 1 3/4" x 11 7/8" 2.0E Parallam® PSL	
Floor: Flush Beam H-26	Failed	4 Piece(s) 1 3/4" x 11 7/8" 2.0E Parallam® PSL	
Wall: Header H-27	Passed	2 Piece(s) 2 x 10 Spruce-Pine-Fir No. 1 / No. 2	
Wall: Header H-28	Passed	2 Piece(s) 1 3/4" x 11 7/8" 2.0E Parallam® PSL	
Wall: Header H-29	Passed	2 Piece(s) 2 x 12 Spruce-Pine-Fir No. 1 / No. 2	

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

04: First floor-Floor framing			
Member Name	**Results**	**Current Solution**	**Comments**
Floor: Flush Beam	Passed	3 Piece(s) 1 3/4" x 11 7/8" 2.0E Parallam® PSL	

1 piece(s) 11 7/8" TJI® 230 @ 16" OC

Overall Length: 18' 11"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	757 @ 18' 6 1/2"	1485 (3.50")	Passed (51%)	1.00	1.0 D + 1.0 L (All Spans)
Shear (lbs)	720 @ 5 1/2"	1655	Passed (44%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	3300 @ 9' 5 1/2"	4215	Passed (78%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.350 @ 9' 5 1/2"	0.606	Passed (L/623)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.525 @ 9' 5 1/2"	0.908	Passed (L/415)	--	1.0 D + 1.0 L (All Spans)
TJ-Pro™ Rating	43	Any	Passed	--	--

System : Floor
Member Type : Joist
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 4' 15/16" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- A structural analysis of the deck has not been performed.
- Deflection analysis is based on composite action with a single layer of 23/32" Weyerhaeuser Edge™ Panel (24" Span Rating) that is glued and nailed down.
- Additional considerations for the TJ-Pro™ Rating include: 1/2" Gypsum ceiling.

Supports	Bearing			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Plate on concrete - SPF	5.50"	4.25"	1.75"	252	504	756	1 1/4" Rim Board
2 - Plate on steel - SPF	5.50"	5.50"	1.75"	252	504	756	Blocking

- Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.
- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Loads	Location	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform(PSF)	0 to 18' 11"	16"	20.0	40.0	Residential - Living Areas

Weyerhaeuser Notes

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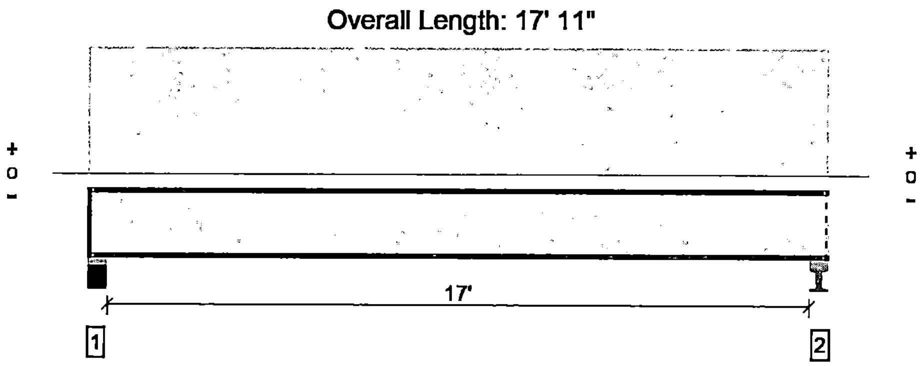
The product application, input design loads, dimensions and support information have been provided by Forte Software Operator

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	717 @ 17' 6 1/2"	1485 (3.50")	Passed (48%)	1.00	1.0 D + 1.0 L (All Spans)
Shear (lbs)	680 @ 17' 5 1/2"	1655	Passed (41%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	2947 @ 8' 11 1/2"	4215	Passed (70%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.283 @ 8' 11 1/2"	0.572	Passed (L/729)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.424 @ 8' 11 1/2"	0.858	Passed (L/486)	--	1.0 D + 1.0 L (All Spans)
TJ-Pro™ Rating	46	Any	Passed	--	--

System : Floor
Member Type : Joist
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 4' 3 13/16" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- A structural analysis of the deck has not been performed.
- Deflection analysis is based on composite action with a single layer of 23/32" Weyerhaeuser Edge™ Panel (24" Span Rating) that is glued and nailed down.
- Additional considerations for the TJ-Pro™ Rating include: 1/2" Gypsum ceiling.

Supports	Bearing			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Plate on concrete - SPF	5.50"	4.25"	1.75"	239	478	717	1 1/4" Rim Board
2 - Plate on steel - SPF	5.50"	5.50"	1.75"	239	478	717	Blocking

- Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.
- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Loads	Location	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform(PSF)	0 to 17' 11"	16"	20.0	40.0	Residential - Living Areas

Weyerhaeuser Notes

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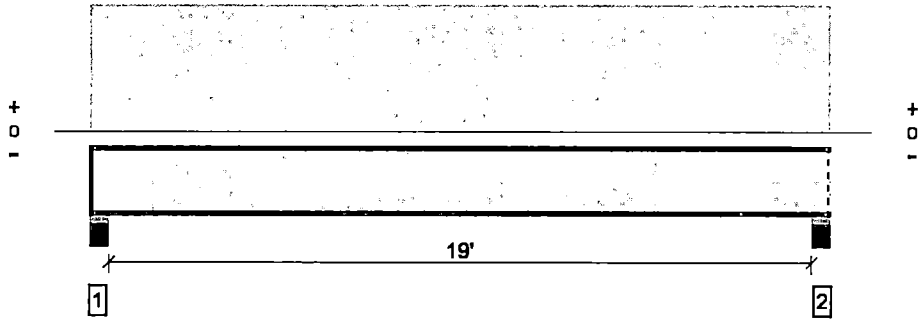
The product application, input design loads, dimensions and support information have been provided by Forte Software Operator

SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

Overall Length: 19' 11"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	797 @ 19' 6 1/2"	1485 (3.50")	Passed (54%)	1.00	1.0 D + 1.0 L (All Spans)
Shear (lbs)	760 @ 19' 5 1/2"	1655	Passed (46%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	3674 @ 9' 11 1/2"	4215	Passed (87%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.429 @ 9' 11 1/2"	0.639	Passed (L/537)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.643 @ 9' 11 1/2"	0.958	Passed (L/358)	--	1.0 D + 1.0 L (All Spans)
TJ-Pro™ Rating	41	Any	Passed	--	--

System : Floor
Member Type : Joist
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 3' 10 7/16" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- A structural analysis of the deck has not been performed.
- Deflection analysis is based on composite action with a single layer of 23/32" Weyerhaeuser Edge™ Panel (24" Span Rating) that is glued and nailed down.
- Additional considerations for the TJ-Pro™ Rating include: 1/2" Gypsum ceiling.

Supports	Bearing			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Plate on concrete - SPF	5.50"	4.25"	1.75"	266	531	797	1 1/4" Rim Board
2 - Plate on concrete - SPF	5.50"	5.50"	1.75"	266	531	797	Blocking

- Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.
- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Loads	Location	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform(PSF)	0 to 19' 11"	16"	20.0	40.0	Residential - Living Areas

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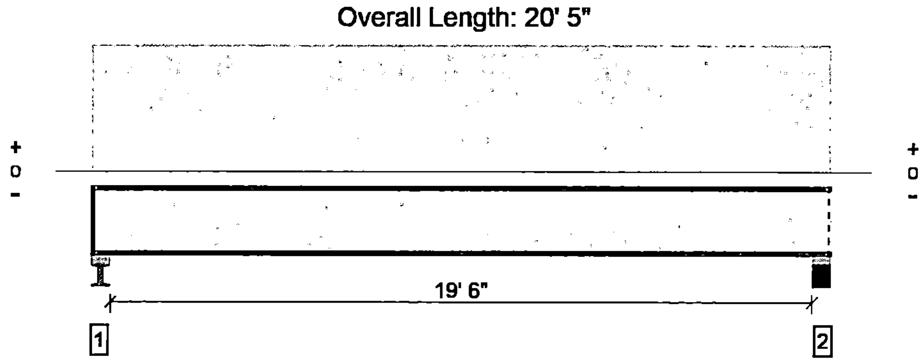
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

FORTE® **MEMBER REPORT** *First Floor-Floor Framing, Floor: Joist J-4* **PASSED**

1 piece(s) 11 7/8" TJI® 230 @ 16" OC



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	817 @ 20' 1/2"	1485 (3.50")	Passed (55%)	1.00	1.0 D + 1.0 L (All Spans)
Shear (lbs)	780 @ 19' 11 1/2"	1655	Passed (47%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	3868 @ 10' 2 1/2"	4215	Passed (92%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.473 @ 10' 2 1/2"	0.656	Passed (L/499)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.709 @ 10' 2 1/2"	0.983	Passed (L/333)	--	1.0 D + 1.0 L (All Spans)
TJ-Pro™ Rating	38	Any	Passed	--	--

System : Floor
Member Type : Joist
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 3' 9 1/4" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- A structural analysis of the deck has not been performed.
- Deflection analysis is based on composite action with a single layer of 23/32" Weyerhaeuser Edge™ Panel (24" Span Rating) that is glued and nailed down.
- Additional considerations for the TJ-Pro™ Rating include: 1/2" Gypsum ceiling.

Supports	Bearing			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Plate on steel - SPF	5.50"	4.25"	1.75"	272	544	816	1 1/4" Rim Board
2 - Plate on concrete - SPF	5.50"	5.50"	1.75"	272	544	816	Blocking

- Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.
- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Loads	Location	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform(PSF)	0 to 20' 5"	16"	20.0	40.0	Residential - Living Areas

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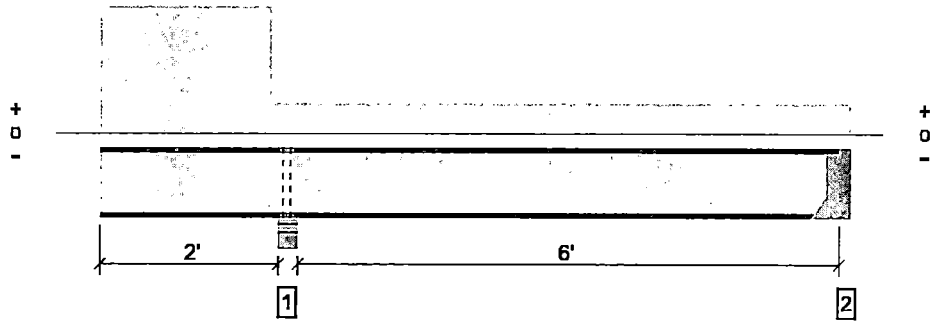
Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

5/3/2013 10:07:37 PM
Forte v4.0, Design Engine: V5.6.1.203
13-159 WOOD FRAMING.4te

FORTE® **MEMBER REPORT** *First Floor-Floor Framing, Floor: Joist J-5* **PASSED**

1 piece(s) 11 7/8" TJI® 230 @ 16" OC

Overall Length: 8' 9"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	819 @ 2' 2 3/4"	2790 (5.25")	Passed (29%)	1.00	1.0 D + 1.0 L (All Spans)
Shear (lbs)	460 @ 2'	1655	Passed (28%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	-568 @ 2' 2 3/4"	4215	Passed (13%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.009 @ 0	0.200	Passed (2L/999+)	--	1.0 D + 0.75 L + 0.75 S (Alt Spans)
Total Load Defl. (in)	0.025 @ 0	0.223	Passed (2L/999+)	--	1.0 D + 0.75 L + 0.75 S (Alt Spans)
TJ-Pro™ Rating	71	Any	Passed	--	--

System : Floor
Member Type : Joist
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Overhang deflection criteria: LL (0.2") and TL (2L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 8' 5 1/2" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- A structural analysis of the deck has not been performed.
- Deflection analysis is based on composite action with a single layer of 23/32" Weyerhaeuser Edge™ Panel (24" Span Rating) that is glued and nailed down.
- Additional considerations for the TJ-Pro™ Rating include: 1/2" Gypsum ceiling.

Supports	Bearing			Loads to Supports (lbs)					Accessories
	Total	Available	Required	Dead	Floor Live	Roof Live	Snow	Total	
1 - Stud wall - SPF	5.50"	5.50"	3.50"	512	306	96	144	1058	Blocking
2 - Hanger on 11 7/8" PSL beam	3.50"	Hanger[1]	1.75"	21	182/-6	-16	-24	203/-46	See note [1]

- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.
- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.

Connector: Simpson Strong-Tie Connectors						
Support	Model	Seat Length	Top Nails	Face Nails	Member Nails	Accessories
2 - Face Mount Hanger	IUS2.37/11.88	2.00"	N/A	10-10d x 1-1/2	N/A	

Loads	Location	Spacing	Dead (0.90)	Floor Live (1.00)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 8' 9"	16"	20.0	40.0	-	-	Residential - Living Areas
2 - Uniform(PLF)	0 to 2'	N/A	150.0	-	40.0	60.0	

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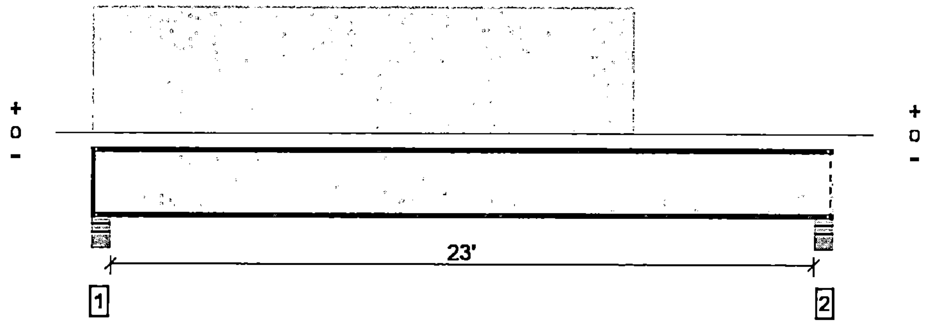
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

Overall Length: 23' 11"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	884 @ 4 1/2"	1725 (3.50")	Passed (51%)	1.00	1.0 D + 1.0 L (All Spans)
Shear (lbs)	856 @ 5 1/2"	2050	Passed (42%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	4652 @ 11' 1 15/16"	9500	Passed (49%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.451 @ 11' 8 5/16"	0.772	Passed (L/616)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.677 @ 11' 8 5/16"	1.158	Passed (L/411)	--	1.0 D + 1.0 L (All Spans)
TJ-Pro™ Rating	36	Any	Passed	--	--

System : Floor
Member Type : Joist
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 7' 1 9/16" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- A structural analysis of the deck has not been performed.
- Deflection analysis is based on composite action with a single layer of 23/32" Weyerhaeuser Edge™ Panel (24" Span Rating) that is glued and nailed down.
- Additional considerations for the TJ-Pro™ Rating include: 1/2" Gypsum ceiling.

Supports	Bearing			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Stud wall - SPF	5.50"	4.25"	1.75"	298	595	893	1 1/4" Rim Board
2 - Stud wall - SPF	5.50"	5.50"	1.75"	168	336	504	Blocking

- Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.
- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Loads	Location	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform(PSF)	0 to 17' 5 1/2"	16"	20.0	40.0	Residential - Living Areas

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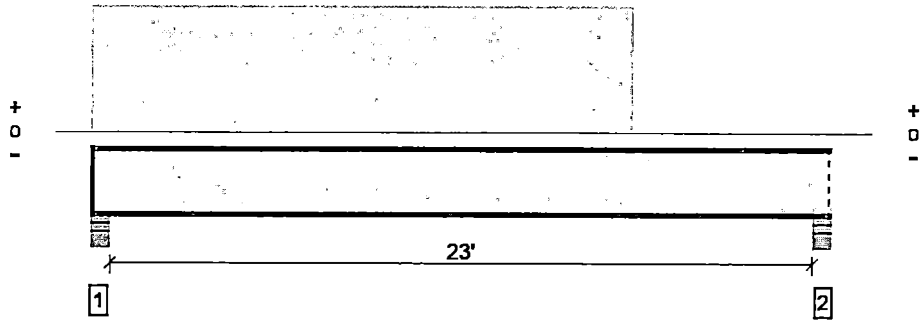
Overall Length: 23' 11"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	663 @ 4 1/2"	1485 (3.50")	Passed (45%)	1.00	1.0 D + 1.0 L (All Spans)
Shear (lbs)	642 @ 5 1/2"	1655	Passed (39%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	3489 @ 11' 1 15/16"	4215	Passed (83%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.578 @ 11' 8 5/16"	0.772	Passed (L/481)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.867 @ 11' 8 5/16"	1.158	Passed (L/321)	--	1.0 D + 1.0 L (All Spans)
TJ-Pro™ Rating	30	Any	Passed	--	--

System : Floor
Member Type : Joist
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 3' 11 5/8" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- A structural analysis of the deck has not been performed.
- Deflection analysis is based on composite action with a single layer of 23/32" Weyerhaeuser Edge™ Panel (24" Span Rating) that is glued and nailed down.
- Additional considerations for the TJ-Pro™ Rating include: 1/2" Gypsum ceiling.

Supports	Bearing			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Stud wall - SPF	5.50"	4.25"	1.75"	223	446	669	1 1/4" Rim Board
2 - Stud wall - SPF	5.50"	5.50"	1.75"	126	252	378	Blocking

- Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.
- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Loads	Location	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform(PSF)	0 to 17' 5 1/2"	12"	20.0	40.0	Residential - Living Areas

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Forte Software Operator	Job Notes
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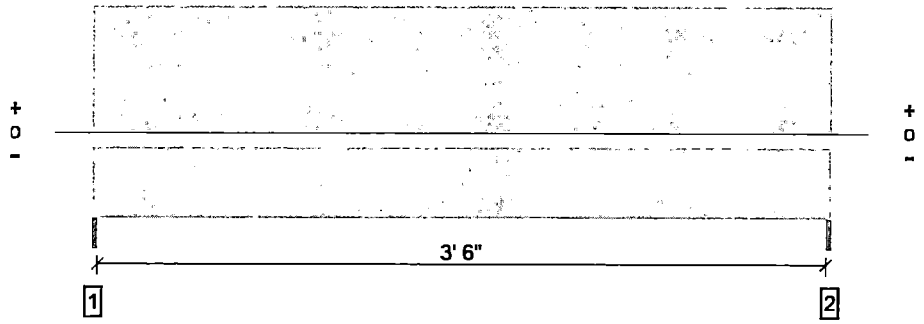
Overall Length: 3' 9"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	195 @ 0	1913 (1.50")	Passed (10%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	135 @ 7"	1708	Passed (8%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	183 @ 1' 10 1/2"	1649	Passed (11%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.005 @ 1' 10 1/2"	0.125	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.008 @ 1' 10 1/2"	0.188	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 3' 9" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Trimmer - SPF	1.50"	1.50"	1.50"	83	75	113	271	None
2 - Trimmer - SPF	1.50"	1.50"	1.50"	83	75	113	271	None

Loads	Location	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 3' 9"	2'	20.0	20.0	30.0	Residential - Living Areas

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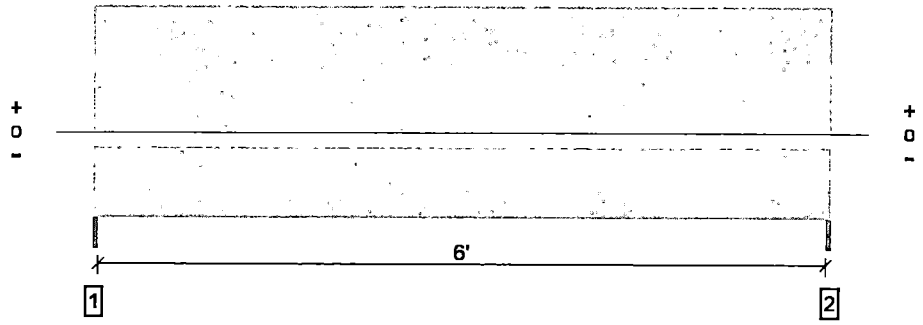
SUSTAINABLE FORESTRY INITIATIVE

Overall Length: 6' 3"



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Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	326 @ 0	1913 (1.50")	Passed (17%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	265 @ 5' 8"	1708	Passed (16%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	509 @ 3' 1 1/2"	1649	Passed (31%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.035 @ 3' 1 1/2"	0.208	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.061 @ 3' 1 1/2"	0.313	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (5/16").
- Bracing (Lu): All compression edges (top and bottom) must be braced at 6' 3" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Trimmer - SPF	1.50"	1.50"	1.50"	138	125	188	451	None
2 - Trimmer - SPF	1.50"	1.50"	1.50"	138	125	188	451	None

Loads	Location	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 6' 3"	2'	20.0	20.0	30.0	Residential - Living Areas

Weyerhaeuser Notes

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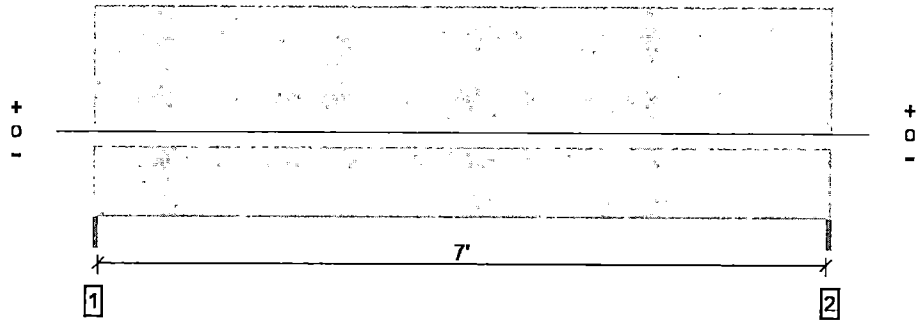
The product application, input design loads, dimensions and support information have been provided by Forte Software Operator

SUSTAINABLE FORESTRY INITIATIVE

Overall Length: 7' 3"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	378 @ 0	1913 (1.50")	Passed (20%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	317 @ 7"	1708	Passed (19%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	684 @ 3' 7 1/2"	1649	Passed (42%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.064 @ 3' 7 1/2"	0.242	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.111 @ 3' 7 1/2"	0.313	Passed (L/782)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (5/16").
- Bracing (Lu): All compression edges (top and bottom) must be braced at 7' 3" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Trimmer - SPF	1.50"	1.50"	1.50"	160	145	218	523	None
2 - Trimmer - SPF	1.50"	1.50"	1.50"	160	145	218	523	None

Loads	Location	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 7' 3"	2'	20.0	20.0	30.0	Residential - Living Areas

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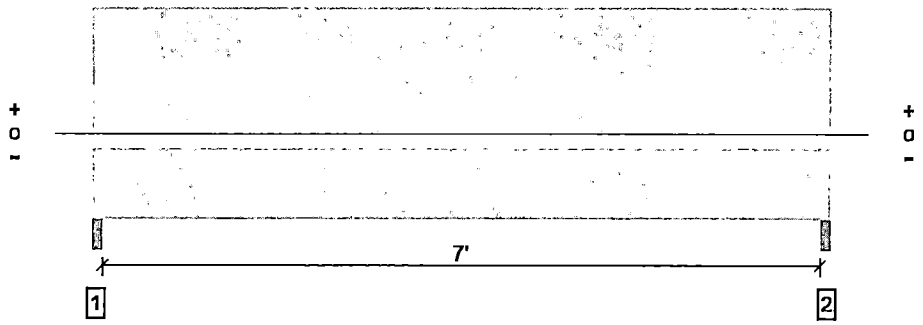
SUSTAINABLE FORESTRY INITIATIVE

Overall Length: 7' 6"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	2657 @ 1 1/2"	3825 (3.00")	Passed (69%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	1816 @ 6' 3 3/4"	3493	Passed (52%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	4655 @ 3' 9"	5306	Passed (88%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.052 @ 3' 9"	0.242	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.088 @ 3' 9"	0.363	Passed (L/984)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 6' 6 3/4" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Trimmer - SPF	3.00"	3.00"	2.08"	1082	1050	1575	3707	None
2 - Trimmer - SPF	3.00"	3.00"	2.08"	1082	1050	1575	3707	None

Loads	Location	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 7' 6"	14'	20.0	20.0	30.0	Residential - Living Areas

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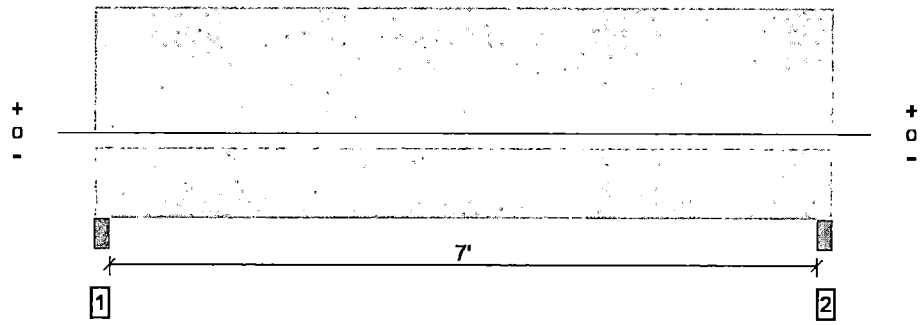
The product application, input design loads, dimensions and support information have been provided by Forte Software Operator

SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

Overall Length: 7' 9"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	4109 @ 3"	11419 (4.50")	Passed (36%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	2872 @ 6' 7"	7393	Passed (39%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	6967 @ 3' 10 1/2"	15016	Passed (46%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.093 @ 3' 10 1/2"	0.242	Passed (L/939)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.156 @ 3' 10 1/2"	0.363	Passed (L/558)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 7' 9" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Trimmer - SPF	4.50"	4.50"	1.62"	1668	1628	2441	5737	None
2 - Trimmer - SPF	4.50"	4.50"	1.62"	1668	1628	2441	5737	None

Loads	Location	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 7' 9"	21'	20.0	20.0	30.0	Residential - Living Areas

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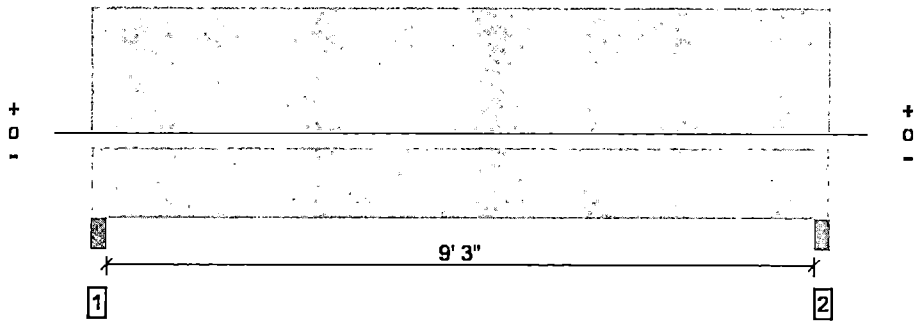
SUSTAINABLE FORESTRY INITIATIVE

Overall Length: 10'



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	5302 @ 3"	11419 (4.50")	Passed (46%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	4065 @ 1' 2"	7393	Passed (55%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	11962 @ 5'	15016	Passed (80%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.255 @ 5'	0.317	Passed (L/446)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.430 @ 5'	0.475	Passed (L/265)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 9' 5 7/16" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Trimmer - SPF	4.50"	4.50"	2.09"	2152	2100	3150	7402	None
2 - Trimmer - SPF	4.50"	4.50"	2.09"	2152	2100	3150	7402	None

Loads	Location	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 10'	21'	20.0	20.0	30.0	Residential - Living Areas

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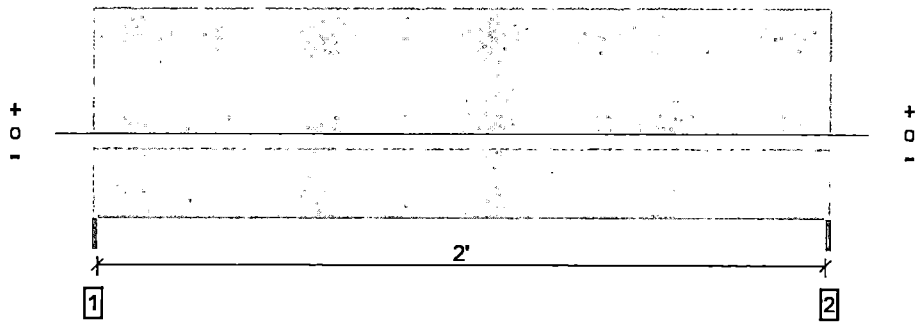
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

FORTE® **MEMBER REPORT** *second Floor-Roof support headers, Wall: Header H-207* **PASSED**

2 piece(s) 2 x 4 Spruce-Pine-Fir No. 1 / No. 2

Overall Length: 2' 3"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	115 @ 0	1913 (1.50")	Passed (6%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	73 @ 5"	1087	Passed (7%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	65 @ 1' 1 1/2"	770	Passed (8%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.002 @ 1' 1 1/2"	0.075	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.004 @ 1' 1 1/2"	0.112	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 2' 3" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Trimmer - SPF	1.50"	1.50"	1.50"	48	45	68	161	None
2 - Trimmer - SPF	1.50"	1.50"	1.50"	48	45	68	161	None

Loads	Location	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 2' 3"	2'	20.0	20.0	30.0	Residential - Living Areas

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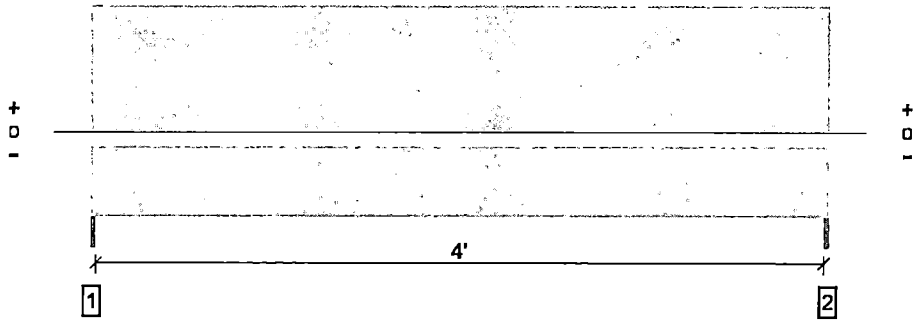
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

FORTE® **MEMBER REPORT** *second Floor-Roof support headers, Wall: Header H-208* **PASSED**

2 piece(s) 2 x 4 Spruce-Pine-Fir No. 1 / No. 2

Overall Length: 4' 3"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	218 @ 0	1913 (1.50")	Passed (11%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	175 @ 5"	1087	Passed (16%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	232 @ 2' 1 1/2"	770	Passed (30%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.029 @ 2' 1 1/2"	0.142	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.050 @ 2' 1 1/2"	0.213	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 4' 3" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Trimmer - SPF	1.50"	1.50"	1.50"	91	85	128	304	None
2 - Trimmer - SPF	1.50"	1.50"	1.50"	91	85	128	304	None

Loads	Location	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 4' 3"	2'	20.0	20.0	30.0	Residential - Living Areas

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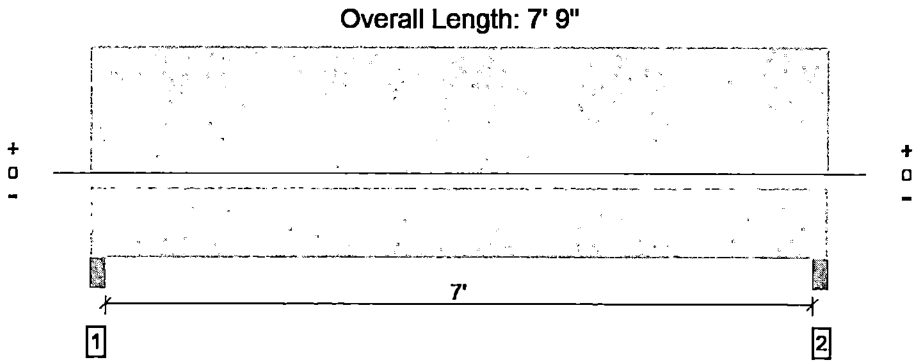
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

2 piece(s) 1 3/4 x 9 1/2" 2.0E Parallam® PSL



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Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	4109 @ 3"	11419 (4.50")	Passed (36%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	2872 @ 6' 7"	7393	Passed (39%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	6967 @ 3' 10 1/2"	15016	Passed (46%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.093 @ 3' 10 1/2"	0.242	Passed (L/939)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.156 @ 3' 10 1/2"	0.363	Passed (L/558)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 7' 9" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Trimmer - SPF	4.50"	4.50"	1.62"	1668	1628	2441	5737	None
2 - Trimmer - SPF	4.50"	4.50"	1.62"	1668	1628	2441	5737	None

Loads	Location	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 7' 9"	21'	20.0	20.0	30.0	Residential - Living Areas

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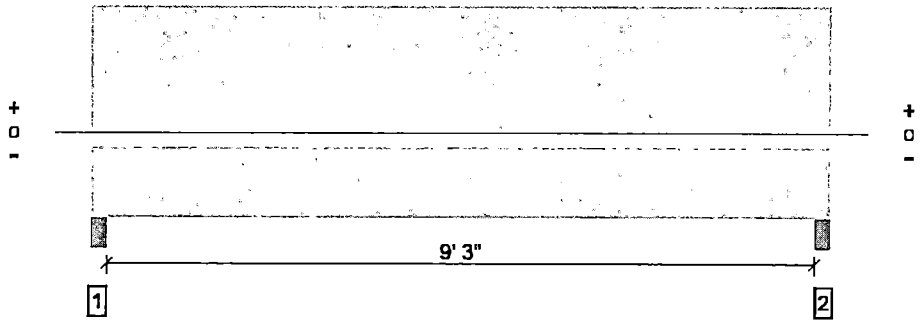
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

5/3/2013 10:07:43 PM
Forte v4.0, Design Engine: V5.6.1.203
13-159 WOOD FRAMING.4te

Overall Length: 10'



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	5302 @ 3"	11419 (4.50")	Passed (46%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	4065 @ 1' 2"	7393	Passed (55%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	11962 @ 5'	15016	Passed (80%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.255 @ 5'	0.317	Passed (L/446)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.430 @ 5'	0.475	Passed (L/265)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 9' 5 7/16" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Trimmer - SPF	4.50"	4.50"	2.09"	2152	2100	3150	7402	None
2 - Trimmer - SPF	4.50"	4.50"	2.09"	2152	2100	3150	7402	None

Loads	Location	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 10'	21'	20.0	20.0	30.0	Residential - Living Areas

Weyerhaeuser Notes

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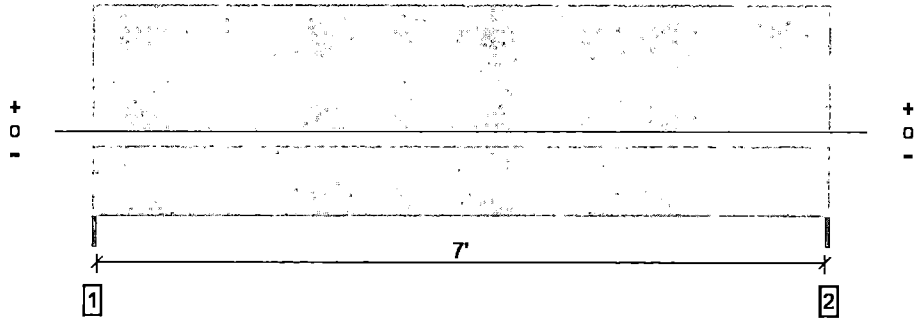
The product application, input design loads, dimensions and support information have been provided by Forte Software Operator

SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

Overall Length: 7' 3"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	378 @ 0	1913 (1.50")	Passed (20%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	317 @ 7"	1708	Passed (19%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	684 @ 3' 7 1/2"	1649	Passed (42%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.064 @ 3' 7 1/2"	0.242	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.111 @ 3' 7 1/2"	0.313	Passed (L/782)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (5/16").
- Bracing (Lu): All compression edges (top and bottom) must be braced at 7' 3" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Trimmer - SPF	1.50"	1.50"	1.50"	160	145	218	523	None
2 - Trimmer - SPF	1.50"	1.50"	1.50"	160	145	218	523	None

Loads	Location	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 7' 3"	2'	20.0	20.0	30.0	Residential - Living Areas

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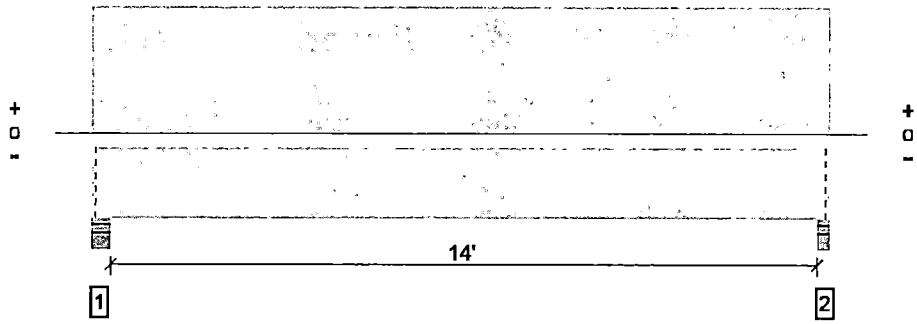
SUSTAINABLE FORESTRY INITIATIVE

Overall Length: 14' 9"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	6705 @ 14' 7"	7809 (3.50")	Passed (86%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	5526 @ 13' 5 5/8"	13861	Passed (40%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	23339 @ 7' 5 1/2"	34332	Passed (68%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.367 @ 7' 5 1/2"	0.475	Passed (L/466)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.625 @ 7' 5 1/2"	0.712	Passed (L/273)	--	1.0 D + 1.0 S (All Spans)

System : Roof
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC
Design Methodology : ASD
Member Pitch: 0/12

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 12' 10 3/4" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Stud wall - SPF	5.50"	5.50"	3.07"	2830	2685	4028	9543	Blocking
2 - Stud wall - SPF	3.50"	3.50"	3.00"	2767	2625	3938	9330	Blocking

- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Loads	Location	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 14' 9"	18'	20.0	20.0	30.0	Roof

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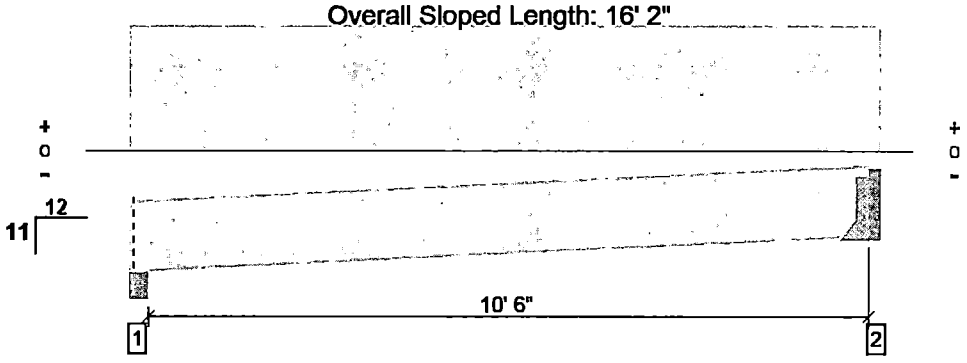
SUSTAINABLE FORESTRY INITIATIVE

FORTE **MEMBER REPORT** *second Floor-Roof support headers, Roof: Flush Beam H-213* **PASSED**

2 piece(s) 1 3/4 x 11 7/8" 2.0E Parallam® PSL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	5557 @ 10' 11 1/2"	5557 (2.12")	Passed (100%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	4794 @ 10' 2 3/4"	9241	Passed (52%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	14760 @ 5' 7 3/4"	22888	Passed (64%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.313 @ 5' 7 3/4"	0.480	Passed (L/553)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.606 @ 5' 7 3/4"	0.721	Passed (L/285)	--	1.0 D + 1.0 S (All Spans)

System : Roof
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC
Design Methodology : ASD
Member Pitch: 11/12

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 10' 3/16" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Beveled Plate - SPF	5.50"	5.50"	3.97"	2857	2033	3049	7939	Blocking
2 - Hanger on 11 7/8" SYP beam	3.50"	Hanger[1]	2.12"	2793	2018	3026	7837	See note [1]

- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.
- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.

Connector: Simpson Strong-Tie Connectors						
Support	Model	Seat Length	Top Nails	Face Nails	Member Nails	Accessories
2 - Face Mount Hanger	Connector not found	N/A	N/A	N/A	N/A	

Loads	Location	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 11' 3"	18'	20.0	20.0	30.0	Roof

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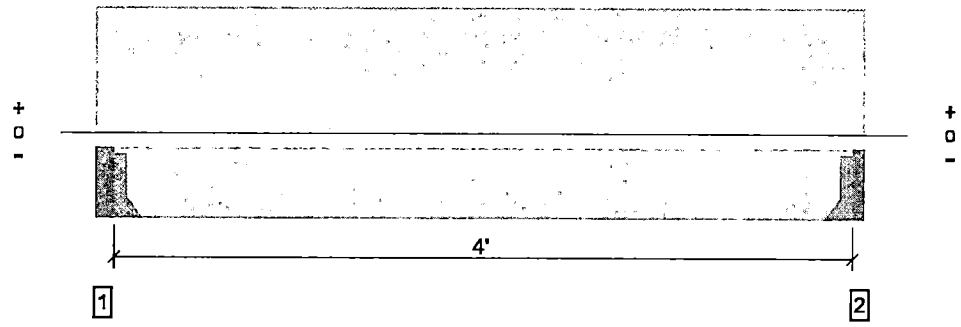
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

FORTE® **MEMBER REPORT** *second Floor-Roof support headers, Roof: Flush Beam H-214* **PASSED**

1 piece(s) 1 3/4 x 11 7/8" 2.0E Parallam® PSL

Overall Length: 4' 9"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	313 @ 5 1/2"	1969 (1.50")	Passed (16%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	158 @ 1' 5 3/8"	4620	Passed (3%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	313 @ 2' 5 1/2"	11444	Passed (3%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.002 @ 2' 5 1/2"	0.133	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.004 @ 2' 5 1/2"	0.200	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)

System : Roof
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC
Design Methodology : ASD
Member Pitch: 0/12

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 4' o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Hanger on 11 7/8" PSL beam	5.50"	Hanger[1]	1.50"	160	148	221	529	See note [1]
2 - Hanger on 11 7/8" PSL beam	3.50"	Hanger[1]	1.50"	150	138	206	494	See note [1]

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.

Connector: Simpson Strong-Tie Connectors						
Support	Model	Seat Length	Top Nails	Face Nails	Member Nails	Accessories
1 - Face Mount Hanger	IUS1.81/11.88	2.00"	N/A	10-10d x 1-1/2	2-10d x 1-1/2	
2 - Face Mount Hanger	IUS1.81/11.88	2.00"	N/A	10-10d x 1-1/2	2-10d x 1-1/2	

Loads	Location	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 4' 9"	3'	20.0	20.0	30.0	Roof

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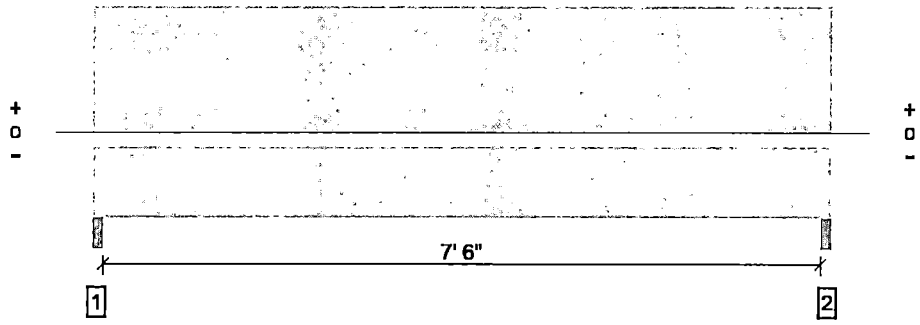
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

FORTE® **MEMBER REPORT** *second Floor-Roof support headers, Wall: Header H-215* **PASSED**

2 piece(s) 2 x 10 Spruce-Pine-Fir No. 1 / No. 2

Overall Length: 8'



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1428 @ 1 1/2"	3825 (3.00")	Passed (37%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	1064 @ 1' 1/4"	2872	Passed (37%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	2681 @ 4'	3946	Passed (68%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.062 @ 4'	0.258	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.105 @ 4'	0.387	Passed (L/889)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 8' o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Trimmer - SPF	3.00"	3.00"	1.50"	588	560	840	1988	None
2 - Trimmer - SPF	3.00"	3.00"	1.50"	588	560	840	1988	None

Loads	Location	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 8'	7'	20.0	20.0	30.0	Residential - Living Areas

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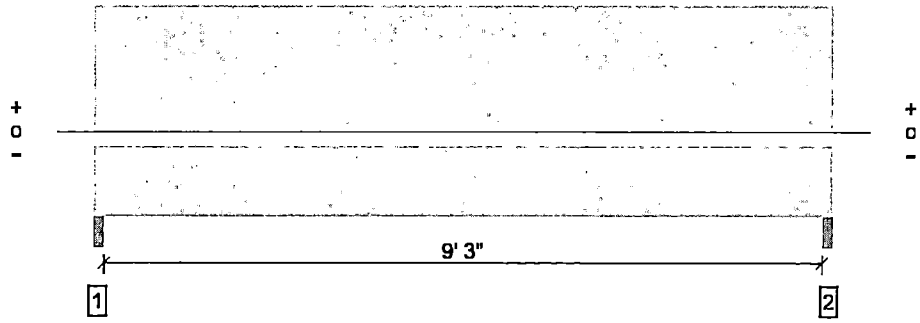
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

Overall Length: 9' 9"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	5152 @ 1 1/2"	7613 (3.00")	Passed (68%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	3646 @ 8' 6 1/8"	8035	Passed (45%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	11315 @ 4' 10 1/2"	19902	Passed (57%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.112 @ 4' 10 1/2"	0.317	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.231 @ 4' 10 1/2"	0.475	Passed (L/493)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 9' 9" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)					Accessories
	Total	Available	Required	Dead	Floor Live	Roof Live	Snow	Total	
1 - Trimmer - SPF	3.00"	3.00"	2.03"	2647	2243	731	1097	6718	None
2 - Trimmer - SPF	3.00"	3.00"	2.03"	2647	2243	731	1097	6718	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 9' 9"	11' 6"	20.0	40.0	-	-	Residential - Living Areas
2 - Uniform(PSF)	0 to 9' 9"	7' 6"	20.0	-	20.0	30.0	
3 - Uniform(PLF)	0 to 9' 9"	N/A	150.0	-	-	-	

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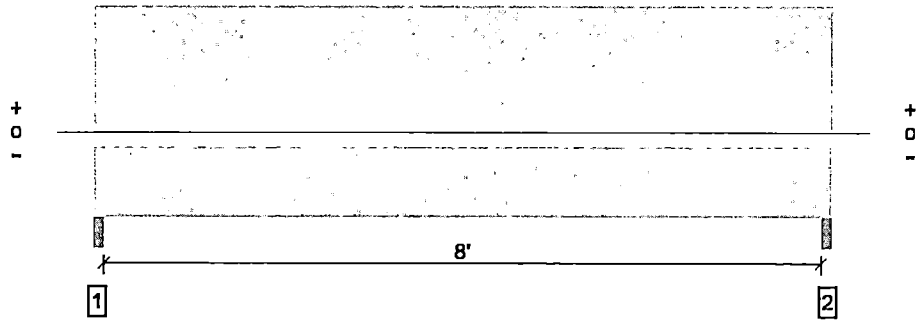
SUSTAINABLE FORESTRY INITIATIVE

Overall Length: 8' 6"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	2268 @ 1 1/2"	3825 (3.00")	Passed (59%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	1634 @ 7' 3 3/4"	3493	Passed (47%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	4539 @ 4' 3"	5306	Passed (86%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.047 @ 4' 3"	0.275	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.112 @ 4' 3"	0.412	Passed (L/887)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 7' 2 1/8" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Trimmer - SPF	3.00"	3.00"	1.78"	1311	638	956	2905	None
2 - Trimmer - SPF	3.00"	3.00"	1.78"	1311	638	956	2905	None

Loads	Location	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 8' 6"	7' 6"	20.0	20.0	30.0	
2 - Uniform(PLF)	0 to 8' 6"	N/A	150.0	-	-	

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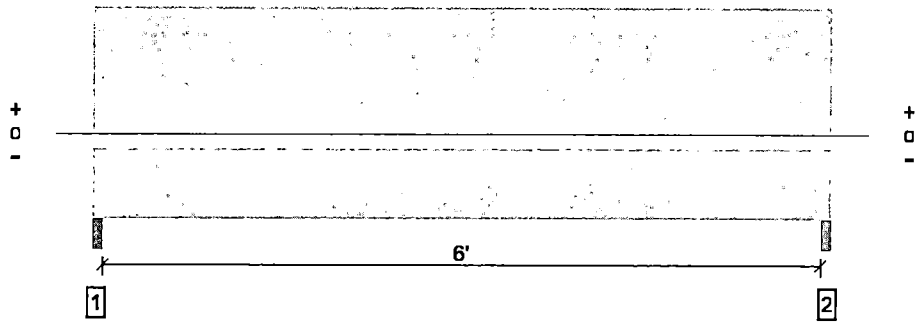
SUSTAINABLE FORESTRY INITIATIVE

FORTE® **MEMBER REPORT** *second floor-Floor support, Wall: Header H-3* **PASSED**

2 piece(s) 2 x 12 Spruce-Pine-Fir No. 1 / No. 2

Overall Length: 6' 6"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	3014 @ 1 1/2"	3825 (3.00")	Passed (79%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	1750 @ 5' 3 3/4"	3038	Passed (58%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	4143 @ 3' 3"	4614	Passed (90%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.030 @ 3' 3"	0.208	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.064 @ 3' 3"	0.313	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 6' 6" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)					Accessories
	Total	Available	Required	Dead	Floor Live	Roof Live	Snow	Total	
1 - Trimmer - SPF	3.00"	3.00"	2.36"	1588	1170	488	731	3977	None
2 - Trimmer - SPF	3.00"	3.00"	2.36"	1588	1170	488	731	3977	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 6' 6"	9'	20.0	40.0	-	-	Residential - Living Areas
2 - Uniform(PSF)	0 to 6' 6"	7' 6"	20.0	-	20.0	30.0	
3 - Uniform(PLF)	0 to 6' 6"	N/A	150.0	-	-	-	

Weyerhaeuser Notes

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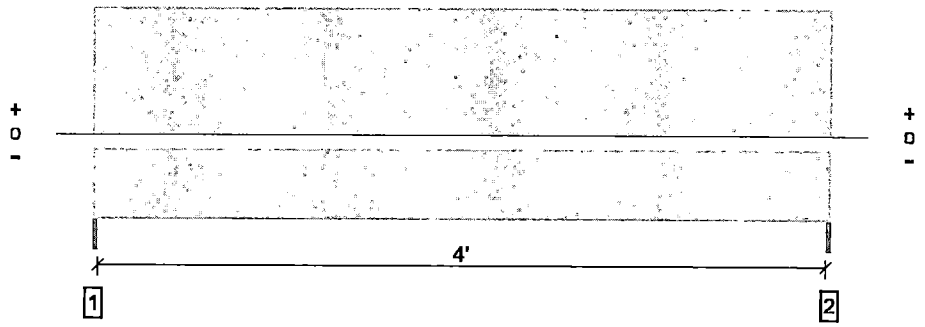
The product application, input design loads, dimensions and support information have been provided by Forte Software Operator

SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

Overall Length: 4' 3"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	766 @ 0	1913 (1.50")	Passed (40%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	500 @ 7"	1485	Passed (34%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	732 @ 2' 1 1/2"	1434	Passed (51%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.015 @ 2' 1 1/2"	0.142	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.045 @ 2' 1 1/2"	0.213	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 4' 3" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)					Accessories
	Total	Available	Required	Dead	Floor Live	Roof Live	Snow	Total	
1 - Trimmer - SPF	1.50"	1.50"	1.50"	519	170	106	159	954	None
2 - Trimmer - SPF	1.50"	1.50"	1.50"	519	170	106	159	954	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 4' 3"	2'	20.0	40.0	-	-	Residential - Living Areas
2 - Uniform(PSF)	0 to 4' 3"	2' 6"	20.0	-	20.0	30.0	
3 - Uniform(PLF)	0 to 4' 3"	N/A	150.0	-	-	-	

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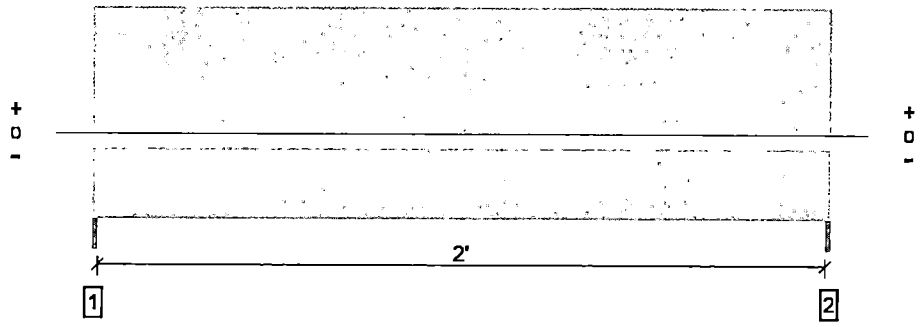
SUSTAINABLE FORESTRY INITIATIVE

Overall Length: 2' 3"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	405 @ 0	1913 (1.50")	Passed (21%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	176 @ 7"	1485	Passed (12%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	205 @ 1' 1 1/2"	1434	Passed (14%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.001 @ 1' 1 1/2"	0.075	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.004 @ 1' 1 1/2"	0.112	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 2' 3" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)					Accessories
	Total	Available	Required	Dead	Floor Live	Roof Live	Snow	Total	
1 - Trimmer - SPF	1.50"	1.50"	1.50"	275	90	56	84	505	None
2 - Trimmer - SPF	1.50"	1.50"	1.50"	275	90	56	84	505	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 2' 3"	2'	20.0	40.0	-	-	Residential - Living Areas
2 - Uniform(PSF)	0 to 2' 3"	2' 6"	20.0	-	20.0	30.0	
3 - Uniform(PLF)	0 to 2' 3"	N/A	150.0	-	-	-	

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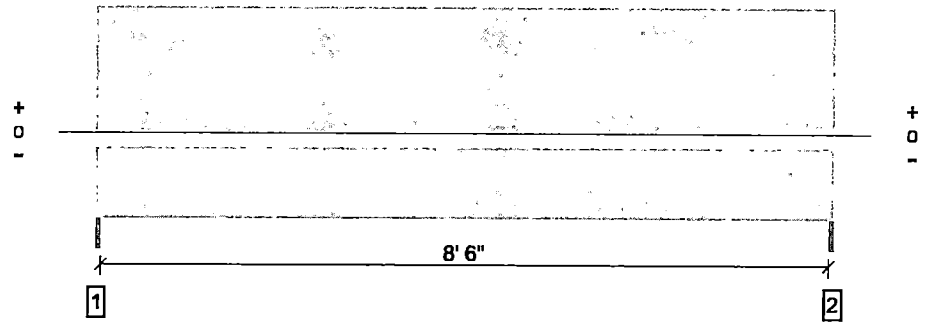
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

Overall Length: 8' 9"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	571 @ 0	1913 (1.50")	Passed (30%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	476 @ 8 3/4"	2251	Passed (21%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	1249 @ 4' 4 1/2"	2645	Passed (47%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.074 @ 4' 4 1/2"	0.292	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.129 @ 4' 4 1/2"	0.313	Passed (L/814)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (5/16").
- Bracing (Lu): All compression edges (top and bottom) must be braced at 8' 9" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Trimmer - SPF	1.50"	1.50"	1.50"	243	219	328	790	None
2 - Trimmer - SPF	1.50"	1.50"	1.50"	243	219	328	790	None

Loads	Location	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 8' 9"	2' 6"	20.0	20.0	30.0	

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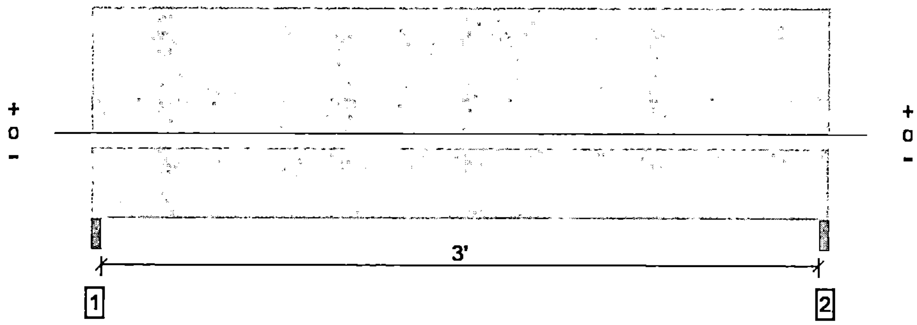
SUSTAINABLE FORESTRY INITIATIVE

2 piece(s) 2 x 8 Spruce-Pine-Fir No. 1 / No. 2

Overall Length: 3' 6"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	2943 @ 1 1/2"	3825 (3.00")	Passed (77%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	1322 @ 10 1/4"	1958	Passed (68%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	1948 @ 1' 9"	2300	Passed (85%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.016 @ 1' 9"	0.108	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.032 @ 1' 9"	0.162	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 3' 6" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)					Accessories
	Total	Available	Required	Dead	Floor Live	Roof Live	Snow	Total	
1 - Trimmer - SPF	3.00"	3.00"	2.31"	1427	1155	578	866	4026	None
2 - Trimmer - SPF	3.00"	3.00"	2.31"	1427	1155	578	866	4026	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 3' 6"	16' 6"	20.0	40.0	-	-	Residential - Living Areas
2 - Uniform(PSF)	0 to 3' 6"	16' 6"	20.0	-	20.0	30.0	
3 - Uniform(PLF)	0 to 3' 6"	N/A	150.0	-	-	-	

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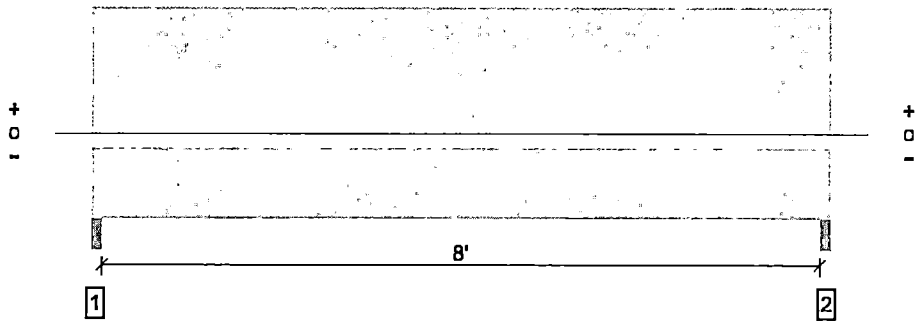
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

Overall Length: 8' 6"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	5628 @ 1 1/2"	7613 (3.00")	Passed (74%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	3892 @ 1' 2 7/8"	8035	Passed (48%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	11000 @ 4' 3"	19902	Passed (55%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.087 @ 4' 3"	0.275	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.173 @ 4' 3"	0.412	Passed (L/574)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 8' 6" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)					Accessories
	Total	Available	Required	Dead	Floor Live	Roof Live	Snow	Total	
1 - Trimmer - SPF	3.00"	3.00"	2.22"	2775	2720	723	1084	7302	None
2 - Trimmer - SPF	3.00"	3.00"	2.22"	2775	2720	723	1084	7302	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 8' 6"	16'	20.0	40.0	-	-	Residential - Living Areas
2 - Uniform(PSF)	0 to 8' 6"	8' 6"	20.0	-	20.0	30.0	
3 - Uniform(PLF)	0 to 8' 6"	N/A	150.0	-	-	-	

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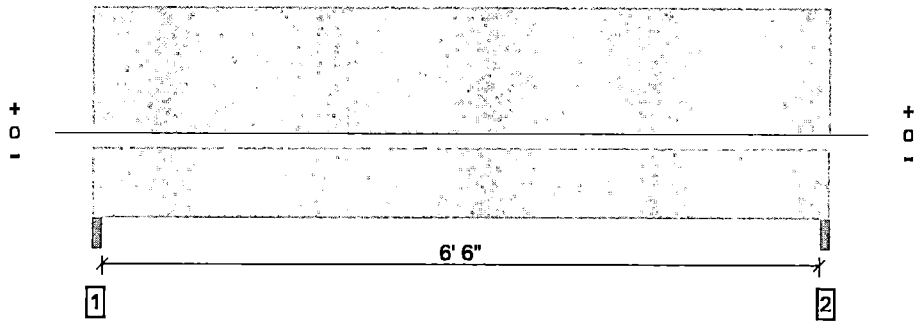
SUSTAINABLE FORESTRY INITIATIVE

Overall Length: 7'



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	5120 @ 1 1/2"	7613 (3.00")	Passed (67%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	3307 @ 5' 9 1/8"	9241	Passed (36%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Moment (Ft-lbs)	8332 @ 3' 6"	22888	Passed (36%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Live Load Defl. (in)	0.046 @ 3' 6"	0.225	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.093 @ 3' 6"	0.338	Passed (L/871)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 7' o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)					Accessories
	Total	Available	Required	Dead	Floor Live	Roof Live	Snow	Total	
1 - Trimmer - SPF	3.00"	3.00"	2.02"	2600	1260	1400	2100	7360	None
2 - Trimmer - SPF	3.00"	3.00"	2.02"	2600	1260	1400	2100	7360	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 7'	9'	20.0	40.0	-	-	Residential - Living Areas
2 - Uniform(PSF)	0 to 7'	20'	20.0	-	20.0	30.0	
3 - Uniform(PLF)	0 to 7'	N/A	150.0	-	-	-	

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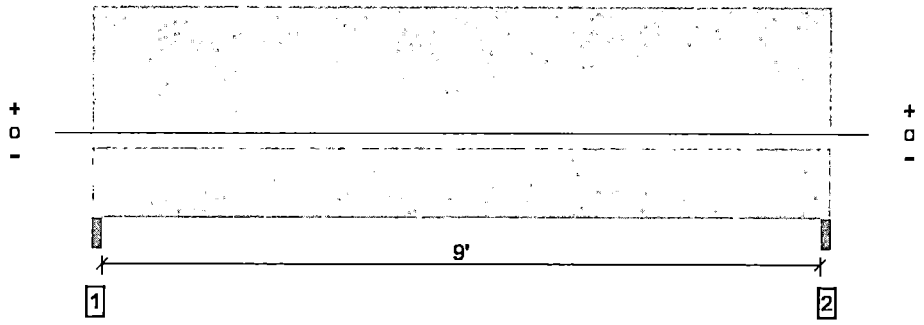
SUSTAINABLE FORESTRY INITIATIVE

Overall Length: 9' 6"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	6949 @ 1 1/2"	7613 (3.00")	Passed (91%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	5136 @ 8' 3 1/8"	9241	Passed (56%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Moment (Ft-lbs)	15647 @ 4' 9"	22888	Passed (68%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Live Load Defl. (in)	0.143 @ 4' 9"	0.308	Passed (L/778)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.290 @ 4' 9"	0.463	Passed (L/383)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 9' 4 1/16" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)					Accessories
	Total	Available	Required	Dead	Floor Live	Roof Live	Snow	Total	
1 - Trimmer - SPF	3.00"	3.00"	2.74"	3529	1710	1900	2850	9989	None
2 - Trimmer - SPF	3.00"	3.00"	2.74"	3529	1710	1900	2850	9989	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 9' 6"	9'	20.0	40.0	-	-	Residential - Living Areas
2 - Uniform(PSF)	0 to 9' 6"	20'	20.0	-	20.0	30.0	
3 - Uniform(PLF)	0 to 9' 6"	N/A	150.0	-	-	-	

Weyerhaeuser Notes

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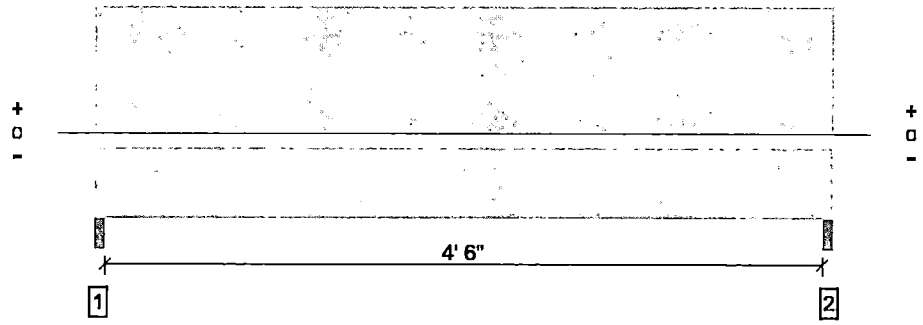
The product application, input design loads, dimensions and support information have been provided by Forte Software Operator

SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

Overall Length: 5'



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1239 @ 1 1/2"	3825 (3.00")	Passed (32%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	816 @ 4' 1 3/4"	1958	Passed (42%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	1397 @ 2' 6"	2300	Passed (61%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.017 @ 2' 6"	0.158	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.043 @ 2' 6"	0.237	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 5' o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)					Accessories
	Total	Available	Required	Dead	Floor Live	Roof Live	Snow	Total	
1 - Trimmer - SPF	3.00"	3.00"	1.50"	739	500	100	150	1489	None
2 - Trimmer - SPF	3.00"	3.00"	1.50"	739	500	100	150	1489	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 5'	5'	20.0	40.0	-	-	Residential - Living Areas
2 - Uniform(PSF)	0 to 5'	2'	20.0	-	20.0	30.0	
3 - Uniform(PLF)	0 to 5'	N/A	150.0	-	-	-	

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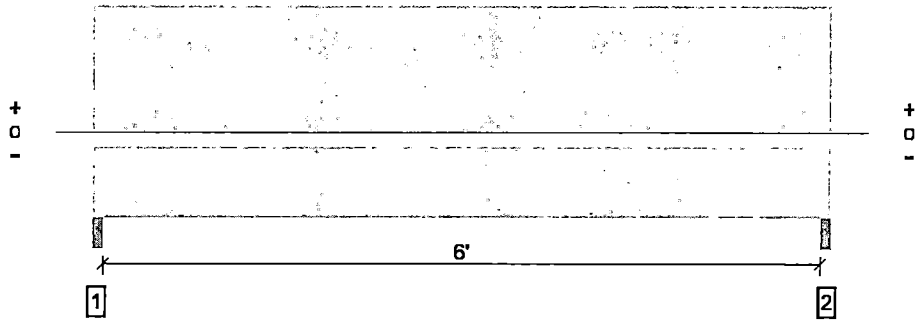
SUSTAINABLE FORESTRY INITIATIVE

FORTE **MEMBER REPORT** *second floor-Floor support, Wall: Header H-12* **PASSED**

2 piece(s) 2 x 12 Spruce-Pine-Fir No. 1 / No. 2

Overall Length: 6' 6"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1620 @ 1 1/2"	3825 (3.00")	Passed (42%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	1028 @ 5' 3 3/4"	3038	Passed (34%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	2434 @ 3' 3"	4614	Passed (53%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.014 @ 3' 3"	0.208	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.034 @ 3' 3"	0.313	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 6' 6" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)					Accessories
	Total	Available	Required	Dead	Floor Live	Roof Live	Snow	Total	
1 - Trimmer - SPF	3.00"	3.00"	1.50"	970	650	130	195	1945	None
2 - Trimmer - SPF	3.00"	3.00"	1.50"	970	650	130	195	1945	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 6' 6"	5'	20.0	40.0	-	-	Residential - Living Areas
2 - Uniform(PSF)	0 to 6' 6"	2'	20.0	-	20.0	30.0	
3 - Uniform(PLF)	0 to 6' 6"	N/A	150.0	-	-	-	

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The product application, input design loads, dimensions and support information have been provided by Forte Software Operator

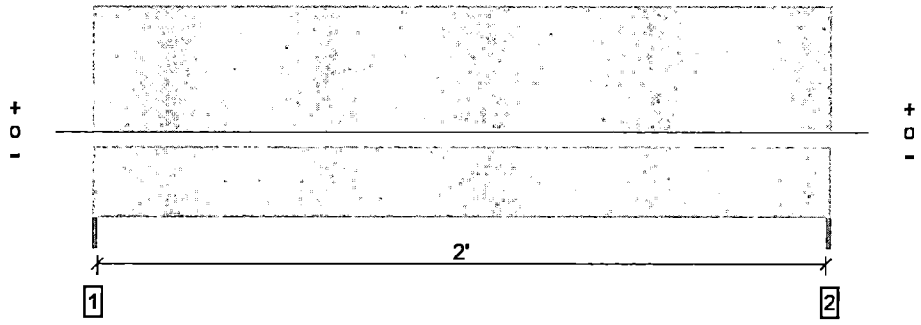
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

FORTE® **MEMBER REPORT** *second floor-Floor support, Wall: Header H-13* **PASSED**

2 piece(s) 2 x 6 Spruce-Pine-Fir No. 1 / No. 2

Overall Length: 2' 3"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	382 @ 0	1913 (1.50")	Passed (20%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	170 @ 1' 8"	1485	Passed (11%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	199 @ 1' 1 1/2"	1434	Passed (14%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.001 @ 1' 1 1/2"	0.075	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.003 @ 1' 1 1/2"	0.112	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 2' 3" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)					Accessories
	Total	Available	Required	Dead	Floor Live	Roof Live	Snow	Total	
1 - Trimmer - SPF	1.50"	1.50"	1.50"	263	90	45	68	466	None
2 - Trimmer - SPF	1.50"	1.50"	1.50"	263	90	45	68	466	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 2' 3"	2'	20.0	40.0	-	-	Residential - Living Areas
2 - Uniform(PSF)	0 to 2' 3"	2'	20.0	-	20.0	30.0	
3 - Uniform(PLF)	0 to 2' 3"	N/A	150.0	-	-	-	

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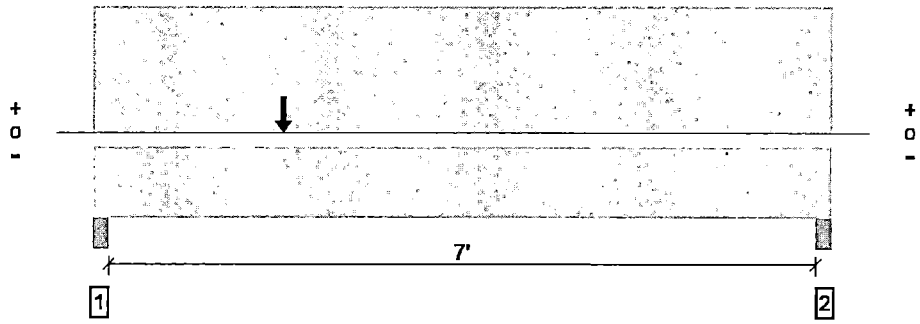
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

Overall Length: 7' 9"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	7718 @ 3"	11419 (4.50")	Passed (68%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	5965 @ 1' 4 3/8"	8035	Passed (74%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	10675 @ 2'	19902	Passed (54%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.073 @ 3' 8 3/16"	0.242	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.143 @ 3' 8 9/16"	0.363	Passed (L/609)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 7' 9" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)					Accessories
	Total	Available	Required	Dead	Floor Live	Roof Live	Snow	Total	
1 - Trimmer - SPF	4.50"	4.50"	3.04"	3699	3034	1550	2325	10608	None
2 - Trimmer - SPF	4.50"	4.50"	2.02"	2664	966	1550	2325	7505	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Point(lb)	2'	N/A	2000	4000	-	-	Residential - Living Areas
2 - Uniform(PSF)	0 to 7' 9"	20'	20.0	-	20.0	30.0	
3 - Uniform(PLF)	0 to 7' 9"	N/A	150.0	-	-	-	

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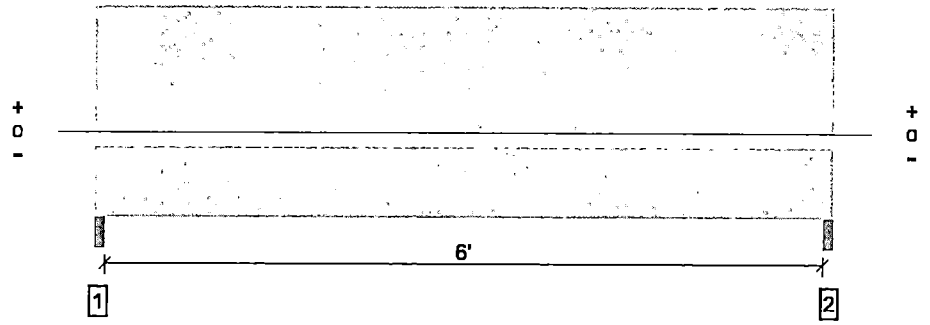
SUSTAINABLE FORESTRY INITIATIVE

FORTE® **MEMBER REPORT** *second floor-Floor support, Wall: Header H-15* **PASSED**

2 piece(s) 1 3/4 x 9 1/2" 2.0E Parallam® PSL

Overall Length: 6' 6"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	4259 @ 1 1/2"	7613 (3.00")	Passed (56%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	2894 @ 1' 1/2"	7393	Passed (39%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	6398 @ 3' 3"	15016	Passed (43%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.059 @ 3' 3"	0.208	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.112 @ 3' 3"	0.313	Passed (L/669)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 6' 6" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Roof Live	Snow	Total	
1 - Trimmer - SPF	3.00"	3.00"	1.68"	2016	1495	2243	5754	None
2 - Trimmer - SPF	3.00"	3.00"	1.68"	2016	1495	2243	5754	None

Loads	Location	Tributary Width	Dead (0.90)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 6' 6"	23'	20.0	20.0	30.0	
2 - Uniform(PLF)	0 to 6' 6"	N/A	150.0	-	-	

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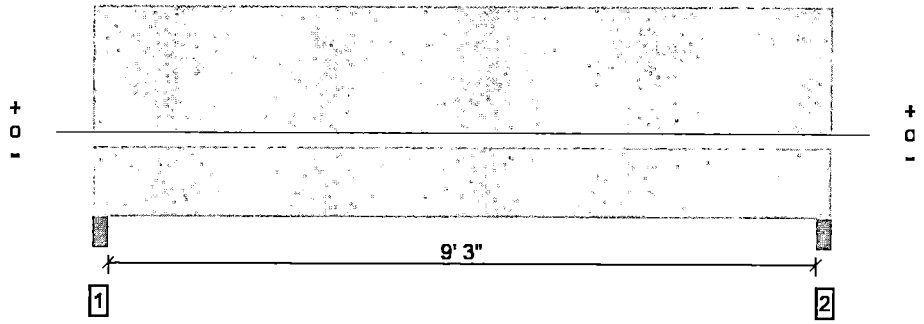
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

Overall Length: 10'



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	7315 @ 3"	11419 (4.50")	Passed (64%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	5319 @ 1' 4 3/8"	9241	Passed (58%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Moment (Ft-lbs)	16504 @ 5'	22888	Passed (72%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Live Load Defl. (in)	0.158 @ 5'	0.317	Passed (L/723)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.320 @ 5'	0.475	Passed (L/356)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 8' 7 15/16" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)					Accessories
	Total	Available	Required	Dead	Floor Live	Roof Live	Snow	Total	
1 - Trimmer - SPF	4.50"	4.50"	2.88"	3715	1800	2000	3000	10515	None
2 - Trimmer - SPF	4.50"	4.50"	2.88"	3715	1800	2000	3000	10515	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 10'	9'	20.0	40.0	-	-	Residential - Living Areas
2 - Uniform(PSF)	0 to 10'	20'	20.0	-	20.0	30.0	
3 - Uniform(PLF)	0 to 10'	N/A	150.0	-	-	-	

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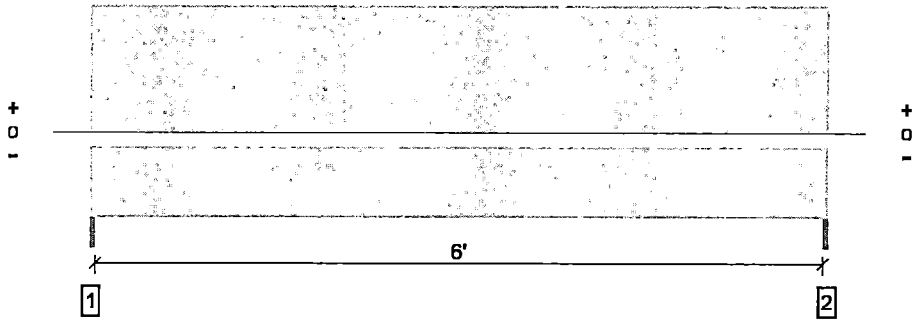
SUSTAINABLE FORESTRY INITIATIVE

Overall Length: 6' 3"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1131 @ 0	1913 (1.50")	Passed (59%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	780 @ 5' 6 1/4"	1958	Passed (40%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	1589 @ 3' 1 1/2"	2300	Passed (69%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.030 @ 3' 1 1/2"	0.208	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.093 @ 3' 1 1/2"	0.313	Passed (L/805)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (5/16").
- Bracing (Lu): All compression edges (top and bottom) must be braced at 6' 3" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)					Accessories
	Total	Available	Required	Dead	Floor Live	Roof Live	Snow	Total	
1 - Trimmer - SPF	1.50"	1.50"	1.50"	767	250	156	234	1407	None
2 - Trimmer - SPF	1.50"	1.50"	1.50"	767	250	156	234	1407	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Roof Live (non-snow: 1.25)	Snow (1.15)	Comments
1 - Uniform(PSF)	0 to 6' 3"	2'	20.0	40.0	-	-	Residential - Living Areas
2 - Uniform(PSF)	0 to 6' 3"	2' 6"	20.0	-	20.0	30.0	
3 - Uniform(PLF)	0 to 6' 3"	N/A	150.0	-	-	-	

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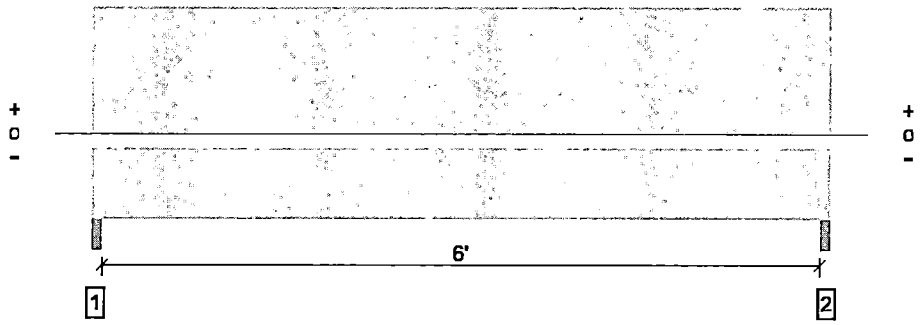
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

FORTE® **MEMBER REPORT** *second floor-Floor support, Wall: Header H-18* **PASSED**

2 piece(s) 1 3/4 x 11 7/8" 2.0E Parallam® PSL

Overall Length: 6' 6"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	3552 @ 1 1/2"	7613 (3.00")	Passed (47%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	2197 @ 5' 3 1/8"	8035	Passed (27%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	5337 @ 3' 3"	19902	Passed (27%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.035 @ 3' 3"	0.208	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.053 @ 3' 3"	0.313	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 6' 6" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Trimmer - SPF	3.00"	3.00"	1.50"	1212	2340	3552	None
2 - Trimmer - SPF	3.00"	3.00"	1.50"	1212	2340	3552	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform(PSF)	0 to 6' 6"	18'	20.0	40.0	Residential - Living Areas

Weyerhaeuser Notes

SUSTAINABLE FORESTRY INITIATIVE

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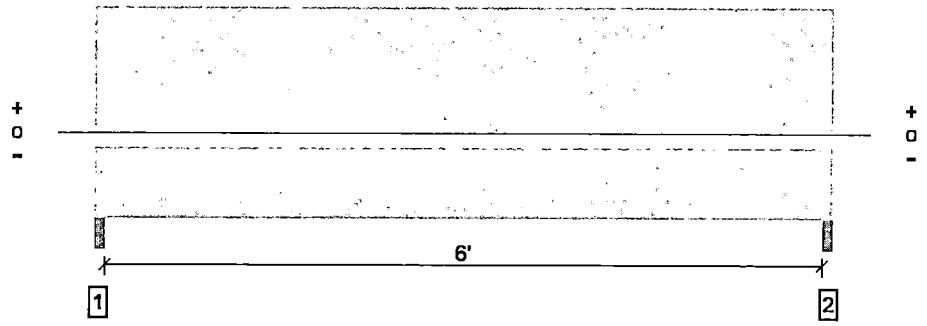
The product application, input design loads, dimensions and support information have been provided by Forte Software Operator

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

FORTE® **MEMBER REPORT** *second floor-Floor support, Wall: Header H-19* **PASSED**

2 piece(s) 2 x 10 Spruce-Pine-Fir No. 1 / No. 2

Overall Length: 6' 6"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	900 @ 1 1/2"	3825 (3.00")	Passed (24%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	618 @ 5' 5 3/4"	2498	Passed (25%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	1353 @ 3' 3"	3431	Passed (39%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.010 @ 3' 3"	0.208	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.034 @ 3' 3"	0.313	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 6' 6" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Trimmer - SPF	3.00"	3.00"	1.50"	640	260	900	None
2 - Trimmer - SPF	3.00"	3.00"	1.50"	640	260	900	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform(PSF)	0 to 6' 6"	2'	20.0	40.0	Residential - Living Areas
2 - Uniform(PLF)	0 to 6' 6"	N/A	150.0	-	

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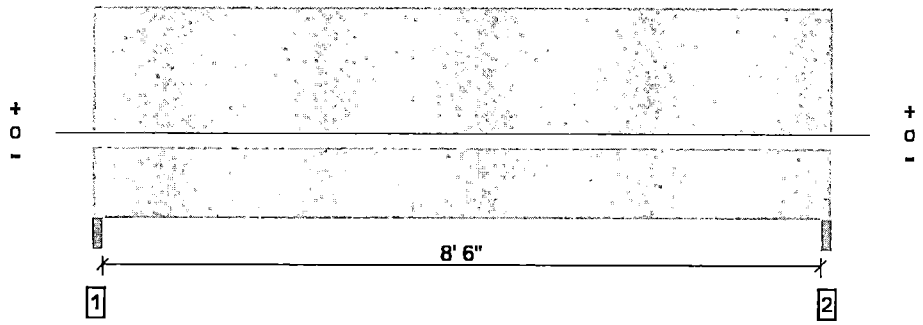
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

Overall Length: 9'



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1247 @ 1 1/2"	3825 (3.00")	Passed (33%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	964 @ 1' 1/4"	2498	Passed (39%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	2651 @ 4' 6"	3431	Passed (77%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.038 @ 4' 6"	0.292	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.132 @ 4' 6"	0.438	Passed (L/796)	--	1.0 D + 1.0 L (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 9' o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Trimmer - SPF	3.00"	3.00"	1.50"	887	360	1247	None
2 - Trimmer - SPF	3.00"	3.00"	1.50"	887	360	1247	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform(PSF)	0 to 9'	2'	20.0	40.0	Residential - Living Areas
2 - Uniform(PLF)	0 to 9'	N/A	150.0	-	

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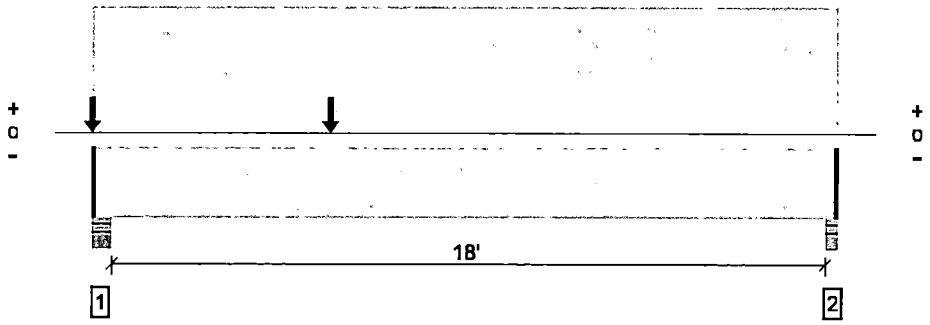
SUSTAINABLE FORESTRY INITIATIVE

Overall Length: 18' 9"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	2823 @ 18' 7"	3347 (2.25")	Passed (84%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	2763 @ 1' 5 3/8"	8035	Passed (34%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	13910 @ 8' 8"	19902	Passed (70%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.305 @ 9' 2 5/16"	0.608	Passed (L/718)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.893 @ 9' 3 7/8"	0.913	Passed (L/245)	--	1.0 D + 1.0 L (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 10' 8 7/16" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Stud wall - SPF	5.50"	4.25"	2.11"	2324	1568	3892	1 1/4" Rim Board
2 - Stud wall - SPF	3.50"	2.25"	1.90"	1959	892	2851	1 1/4" Rim Board

- Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform(PSF)	0 to 18' 9"	2'	20.0	40.0	Residential - Living Areas
2 - Point(lb)	6'	N/A	240	480	
3 - Uniform(PLF)	0 to 18' 9"	N/A	150.0	-	
4 - Point(lb)	0	N/A	240	480	

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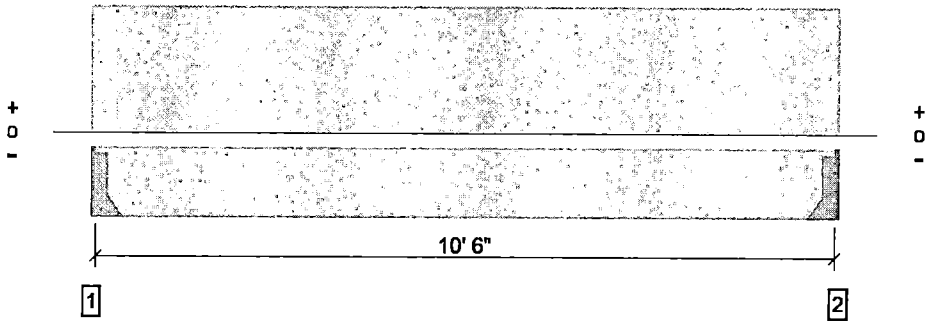
SUSTAINABLE FORESTRY INITIATIVE

Overall Length: 10' 9"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1486 @ 1 1/2"	3938 (1.50")	Passed (38%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	1206 @ 9' 7 5/8"	8035	Passed (15%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	3900 @ 5' 4 1/2"	19902	Passed (20%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.025 @ 5' 4 1/2"	0.350	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.090 @ 5' 4 1/2"	0.525	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 10' 6" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Hanger on 11 7/8" PSL beam	1.50"	Hanger[1]	1.50"	1089	430	1519	See note [1]
2 - Hanger on 11 7/8" PSL beam	1.50"	Hanger[1]	1.50"	1089	430	1519	See note [1]

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.

Connector: Simpson Strong-Tie Connectors

Support	Model	Seat Length	Top Nails	Face Nails	Member Nails	Accessories
1 - Face Mount Hanger	MIU3.56/11	2.50"	N/A	20-10d x 1-1/2	2-10d x 1-1/2	
2 - Face Mount Hanger	MIU3.56/11	2.50"	N/A	20-10d x 1-1/2	2-10d x 1-1/2	

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform(PSF)	0 to 10' 9"	2'	20.0	40.0	Residential - Living Areas
2 - Uniform(PLF)	0 to 10' 9"	N/A	150.0	-	

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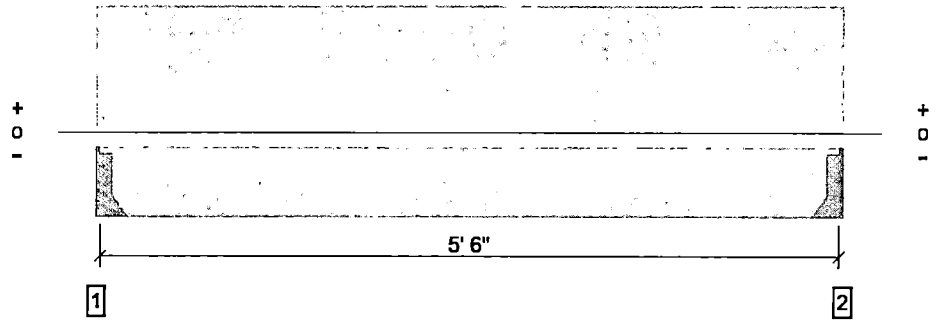
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

FORTE® **MEMBER REPORT** *second floor-Floor support, Floor: Flush Beam H-23* **PASSED**

2 piece(s) 1 3/4 x 11 7/8" 2.0E Parallam® PSL

Overall Length: 5' 9"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	3006 @ 1 1/2"	3938 (1.50")	Passed (76%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	1924 @ 4' 7 5/8"	8035	Passed (24%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	4133 @ 2' 10 1/2"	19902	Passed (21%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.023 @ 2' 10 1/2"	0.183	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.034 @ 2' 10 1/2"	0.275	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 5' 6" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Hanger on 11 7/8" PSL beam	1.50"	Hanger[1]	1.50"	1071	2070	3141	See note [1]
2 - Hanger on 11 7/8" PSL beam	1.50"	Hanger[1]	1.50"	1071	2070	3141	See note [1]

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.

Connector: Simpson Strong-Tie Connectors

Support	Model	Seat Length	Top Nails	Face Nails	Member Nails	Accessories
1 - Face Mount Hanger	Connector not found	N/A	N/A	N/A	N/A	
2 - Face Mount Hanger	Connector not found	N/A	N/A	N/A	N/A	

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform(PSF)	0 to 5' 9"	18'	20.0	40.0	Residential - Living Areas

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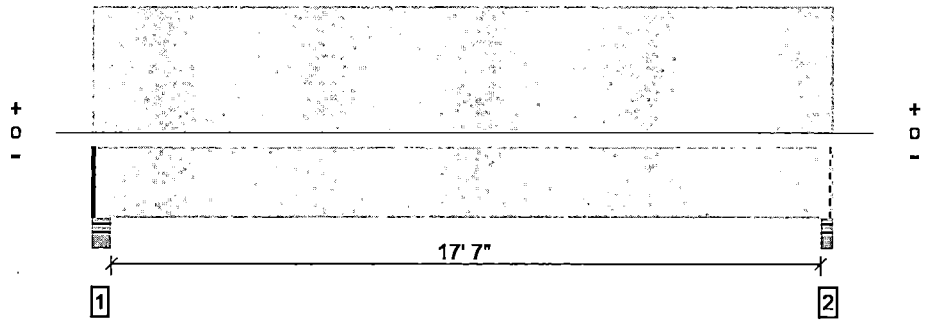
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

FORTE® **MEMBER REPORT** *second floor-Floor support, Floor: Flush Beam H-24* **PASSED**

2 piece(s) 1 3/4 x 11 7/8" 2.0E Parallam® PSL

Overall Length: 18' 4"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	2843 @ 18' 2"	5206 (3.50")	Passed (55%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	2442 @ 17' 5/8"	8035	Passed (30%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	12442 @ 9' 3"	19902	Passed (63%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.488 @ 9' 3"	0.594	Passed (L/439)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.764 @ 9' 3"	0.892	Passed (L/280)	--	1.0 D + 1.0 L (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 12' 2 11/16" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Stud wall - SPF	5.50"	4.25"	1.92"	1044	1850	2894	1 1/4" Rim Board
2 - Stud wall - SPF	3.50"	3.50"	1.91"	1026	1817	2843	Blocking

- Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.
- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform(PSF)	0 to 18' 4"	5'	20.0	40.0	Residential - Living Areas

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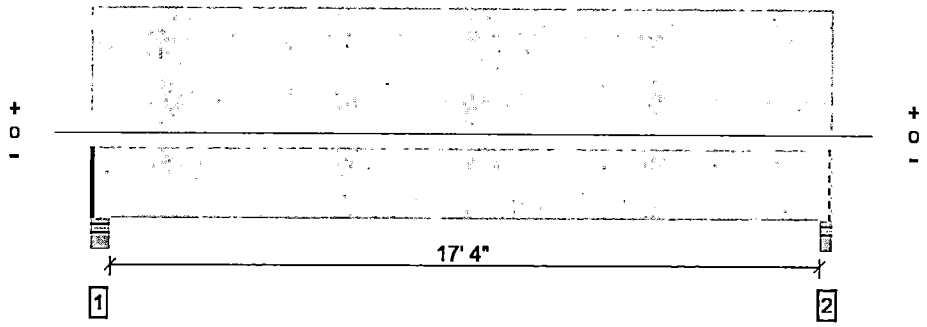
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

FORTE® **MEMBER REPORT** *second floor-Floor support, Floor: Flush Beam H-25* **PASSED**

2 piece(s) 1 3/4 x 11 7/8" 2.0E Parallam® PSL

Overall Length: 18' 1"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	2804 @ 17' 11"	5206 (3.50")	Passed (54%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	2403 @ 1' 5 3/8"	8035	Passed (30%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	12096 @ 9' 1 1/2"	19902	Passed (61%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.462 @ 9' 1 1/2"	0.586	Passed (L/457)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.723 @ 9' 1 1/2"	0.879	Passed (L/292)	--	1.0 D + 1.0 L (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 12' 7 1/2" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Stud wall - SPF	5.50"	4.25"	1.90"	1030	1825	2855	1 1/4" Rim Board
2 - Stud wall - SPF	3.50"	3.50"	1.88"	1012	1792	2804	Blocking

- Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.
- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform(PSF)	0 to 18' 1"	5'	20.0	40.0	Residential - Living Areas

Weyerhaeuser Notes

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The product application, input design loads, dimensions and support information have been provided by Forte Software Operator

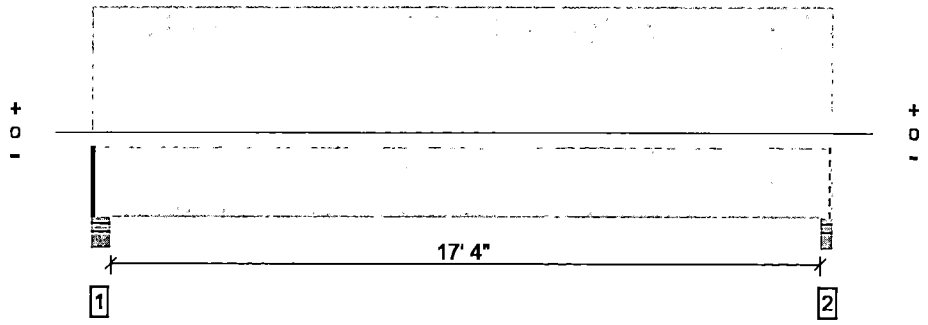
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

FORTE® **MEMBER REPORT** *second floor-Floor support, Floor: Flush Beam H-26* **FAILED**

4 piece(s) 1 3/4 x 11 7/8" 2.0E Parallam® PSL

Overall Length: 18' 1"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	7489 @ 17' 11"	10413 (3.50")	Passed (72%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	6418 @ 1' 5 3/8"	16071	Passed (40%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	32307 @ 9' 1 1/2"	39805	Passed (81%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.508 @ 9' 1 1/2"	0.586	Passed (L/415)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.965 @ 9' 1 1/2"	0.879	Failed (L/219)	--	1.0 D + 1.0 L (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 13' 10 13/16" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Stud wall - SPF	5.50"	4.25"	2.53"	3610	4015	7625	1 1/4" Rim Board
2 - Stud wall - SPF	3.50"	3.50"	2.52"	3547	3942	7489	Blocking

- Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.
- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform(PSF)	0 to 18' 1"	11'	20.0	40.0	Residential - Living Areas
2 - Uniform(PLF)	0 to 18' 1"	N/A	150.0	-	

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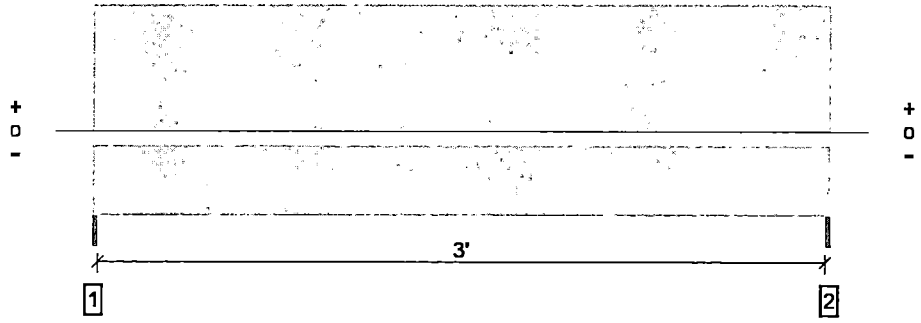
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

Overall Length: 3' 3"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1766 @ 0	1913 (1.50")	Passed (92%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	793 @ 2' 4 1/4"	2498	Passed (32%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	1435 @ 1' 7 1/2"	3431	Passed (42%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.007 @ 1' 7 1/2"	0.108	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.010 @ 1' 7 1/2"	0.162	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 3' 3" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Trimmer - SPF	1.50"	1.50"	1.50"	596	1170	1766	None
2 - Trimmer - SPF	1.50"	1.50"	1.50"	596	1170	1766	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform(PSF)	0 to 3' 3"	18'	20.0	40.0	Residential - Living Areas

Weyerhaeuser Notes

SUSTAINABLE FORESTRY INITIATIVE

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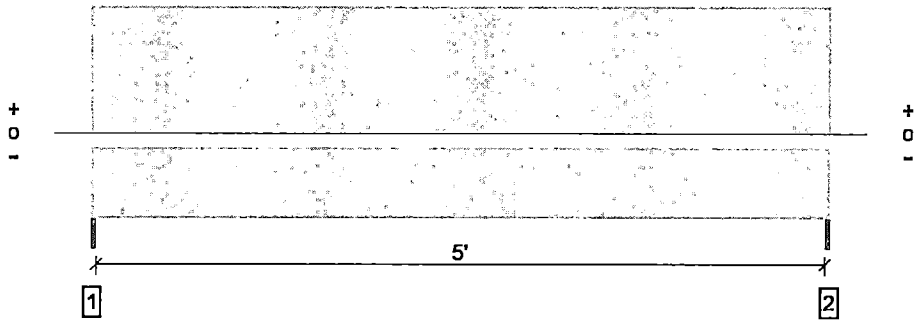
The product application, input design loads, dimensions and support information have been provided by Forte Software Operator

FORTE® **MEMBER REPORT** *second floor-Floor support, Wall: Header H-28* **PASSED**

2 piece(s) 1 3/4 x 11 7/8" 2.0E Parallam® PSL

Overall Length: 5' 3"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1452 @ 0	3806 (1.50")	Passed (38%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	835 @ 4' 1 5/8"	8035	Passed (10%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	1905 @ 2' 7 1/2"	19902	Passed (10%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.010 @ 2' 7 1/2"	0.175	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.015 @ 2' 7 1/2"	0.262	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 5' 3" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Trimmer - SPF	1.50"	1.50"	1.50"	507	945	1452	None
2 - Trimmer - SPF	1.50"	1.50"	1.50"	507	945	1452	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform(PSF)	0 to 5' 3"	9'	20.0	40.0	Residential - Living Areas

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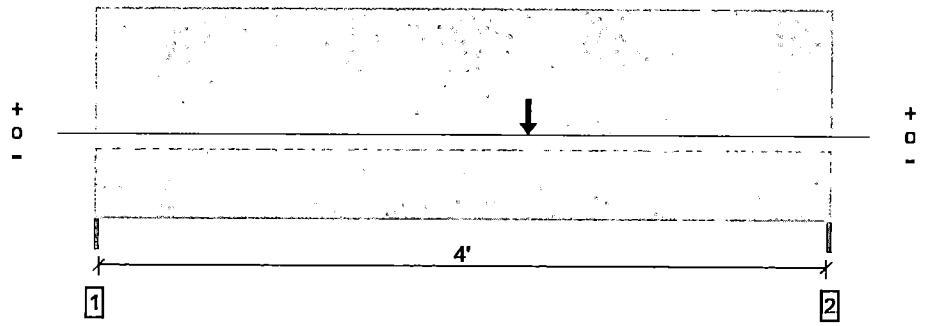
SUSTAINABLE FORESTRY INITIATIVE

Forte Software Operator	Job Notes
Robert Williams, PE TDI Associates, Inc. Architects & Engineers (262) 437-0400 bob.w@tdiae.com	Stratton Residence Joy Peot Shields Architect Fox Point, Wisconsin TDI project no. 13-159

Overall Length: 4' 3"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1545 @ 4' 3"	1913 (1.50")	Passed (81%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	1249 @ 3' 2 1/4"	3038	Passed (41%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	2277 @ 2' 6"	4614	Passed (49%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.007 @ 2' 1 7/8"	0.142	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.013 @ 2' 1 13/16"	0.213	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 4' 3" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.
- Applicable calculations are based on NDS 2005 methodology.

Supports	Bearing			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Trimmer - SPF	1.50"	1.50"	1.50"	644	615	1259	None
2 - Trimmer - SPF	1.50"	1.50"	1.50"	740	805	1545	None

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform(PSF)	0 to 4' 3"	2'	20.0	40.0	Residential - Living Areas
2 - Uniform(PLF)	0 to 4' 3"	N/A	150.0	-	
3 - Point(lb)	2' 6"	N/A	540	1080	

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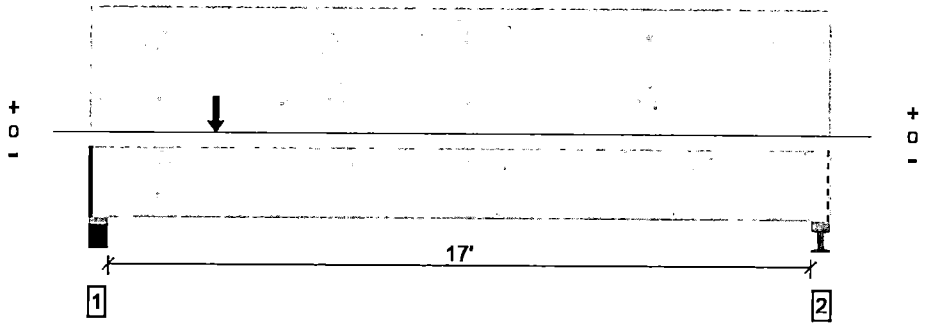
SUSTAINABLE FORESTRY INITIATIVE

Overall Length: 17' 11"



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.;Drawing is Conceptual

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	9351 @ 4"	9483 (4.25")	Passed (99%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	9163 @ 1' 5 3/8"	12053	Passed (76%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	24355 @ 3'	29854	Passed (82%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.500 @ 8' 5/8"	0.575	Passed (L/414)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.778 @ 8' 1"	0.863	Passed (L/266)	--	1.0 D + 1.0 L (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Bracing (Lu): All compression edges (top and bottom) must be braced at 11' 3/16" o/c unless detailed otherwise. Proper attachment and positioning of lateral bracing is required to achieve member stability.

Supports	Bearing			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Total	
1 - Plate on concrete - SPF	5.50"	4.25"	4.19"	3236	6127	9363	1 1/4" Rim Board
2 - Plate on steel - SPF	5.50"	5.50"	1.50"	1027	1706	2733	Blocking

- Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.
- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform(PSF)	0 to 17' 11"	2'	20.0	40.0	Residential - Living Areas
2 - Point(lb)	3'	N/A	3200	6400	

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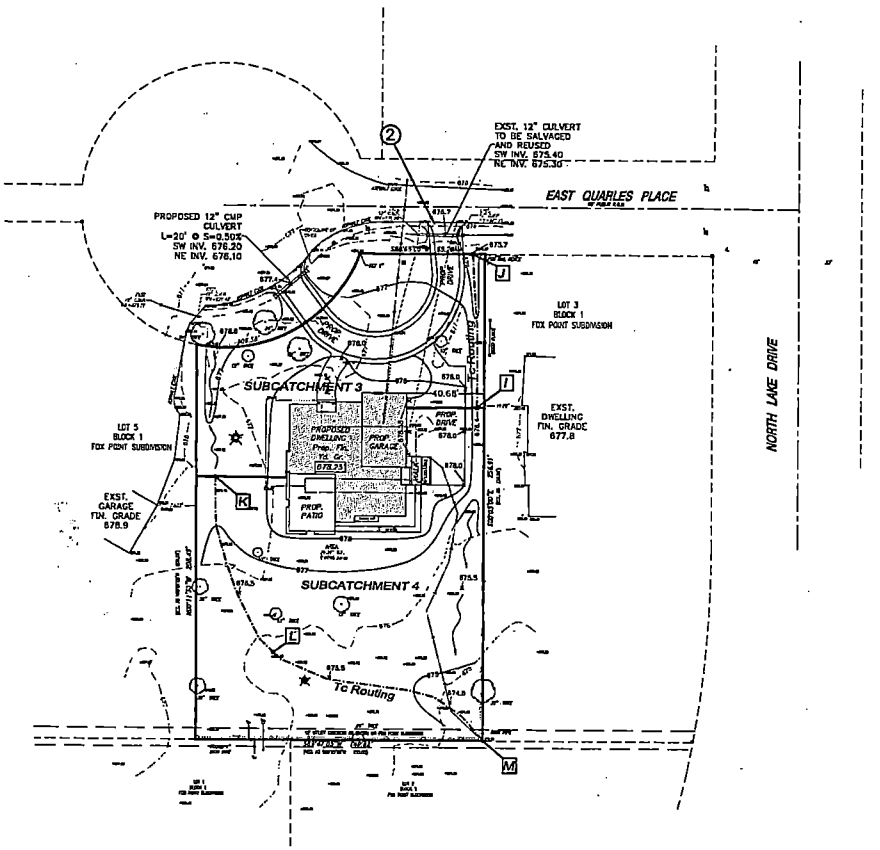
SUSTAINABLE FORESTRY INITIATIVE

CJ engineering
civil design and consulting
9208 W. Center Street
Suite 214
Milwaukee, WI 53222
PH. (414) 443-1312
FAX (414) 443-1317
www.cj-engineering.com

1015 EAST QUARLES PLACE

FOX POINT, WI



LEGEND

- EXISTING CONTOUR
- EXISTING ELEVATION
- EXST. TREE TO REMAIN
- EXST. GAS LINE
- EXST. TELEPHONE LINE
- EXST. OVERHEAD WIRE
- PROPOSED CONTOUR
- PROPOSED ELEVATION
- PROPOSED SWALE/DRAINAGE

IMPERVIOUS SURFACE CALCULATION:

EXISTING	4,870 S.F.
PROPOSED	8,846 S.F.

NOTE:
1. PROPOSED SITE INCREASES IMPERVIOUS SURFACE BY 0.091 ACRES AND DISTURBES APPROXIMATELY 31,350 SQ. FT. THEREFORE, NO STORM WATER MANAGEMENT NECESSARY PER WISCONSIN DEPARTMENT OF COMMERCE COMM 21.125 AND WDNR NR 151.

STORM WATER CALCULATION:

	STORM EVENT		
AREA	2-YEAR	10-YEAR	100-YEAR
EXISTING	0.82 CFS	1.64 CFS	3.63 CFS
PROPOSED	1.00 CFS	1.90 CFS	4.05 CFS

Graphic Scale
30 0 15 30 60
(IN FEET)
1 INCH = 30 FT.

CJEI322RI
JUNE 14, 2013
REV. 08-22-13

PROPOSED CONDITIONS

cj engineering
civil design and consulting
9205 W. Center Street
Suite 214
Milwaukee, WI 53222
PH. (414) 443-1312
FAX (414) 443-1317
www.cj-engineering.com

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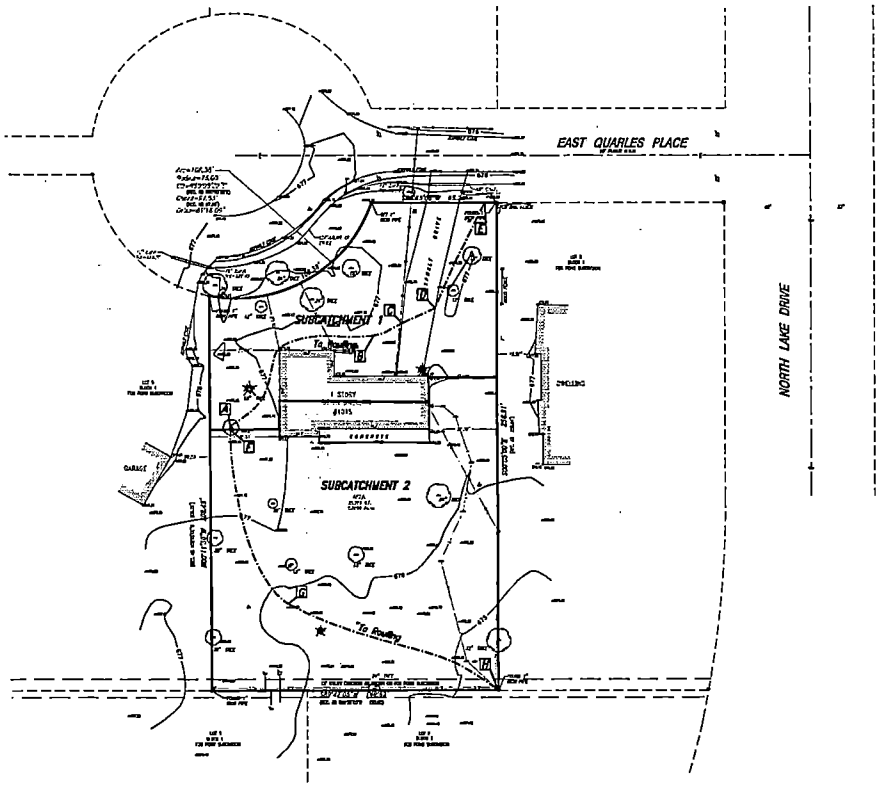


Graphic Scale
30 0 15 30 60
(IN FEET)
1 INCH = 30 FT.

1015 EAST QUARLES PLACE

FOX POINT, WI

CJEI322RI
JUNE 14, 2013
REV. 08-22-13

EXISTING CONDITIONS

Paul Blossfield

Phone (414) 358-1613
Fax (414) 358-1698

Frank
ARMSTRONG
enterprises, Inc.

• asphalt paving • tennis court construction •
• design • build • maintenance •

8535 W. Kaul Avenue
Milwaukee, WI 53225

Rec # 13279

9/16/57
ck.

VILLAGE OF FOX POINT

MILWAUKEE COUNTY, WISCONSIN

No. 4401

APPLICATION FOR PERMIT

TO THE INSPECTION DEPARTMENT:

The undersigned hereby applies for a permit for the execution of electrical installation for light, heat or power, as hereinafter prescribed.

1. Location 1015 East Quarles Place
   (Give exact street and number. Do not give corner.)
2. Owner Norbert Friedlen
3. Lot ____ Block ____ Subdivision ____
4. Building or structure residence
5. Contractor The Jonas Co. Inc. License No. ____

		Number	Rate of Fees		Fees
6.	Lighting Outlets		@	$ .10	
7.	Fixtures	4	"	.05	.20
8.	Range Circuit or Outlet	dryer outlet	"	1.00	1.00
9.	Range Connection		"	1.00	
10.	Water Heaters & other Heating Devices	1st Kilowatt	"	1.00	
11.	Refrigerating Machines	Each Additional Kilowatt .10	"	2.00	
12.	Oil Burners and Stokers		"	1.00	
13.	Temporary Permits	Inspection per Hour	"	2.00	
14.	Motors	1 dishwasher & 1 disposall H.P.-H.P.-H.P. per H.P.	"	.10	.20
15.	Studded Lights including their Individual Outlets		"	.05	
16.	Rectifiers and Transformers		"	1.00	

Estimated cost $ ____ Total fees 1.40

Date of inspection: Wiring Will call 19 ____ Fixtures Will call 19 ____

Note: Minimum Fee $1.00

Enclosed please find $ ____

It is hereby agreed between the undersigned person, firm or corporation and the Village of Fox Point that for and in consideration of the premises and of the permit for the execution of the electrical installation, for light, heat or power, as particularly described in this application, the work will be done in accordance with all of the provisions of all ordinances regulating the installation of electrical work, electric wiring and apparatus in the Village of Fox Point and all of the subsequent amendments thereto.

Signed B F Friedl

Address 8416 W. Lisbon Avenue

INSPECTION DEPARTMENT
VILLAGE OF FOX POINT
7200 North Santa Monica Blvd.
Milwaukee, Wisconsin 53217

APPLICATION FOR CERTIFICATE OF OCCUPANCY

No..... Date.....

Address 1015 E QUARLES Fox Point, Wisconsin

Type of Occupancy SINGLE FAMILY

Type of Former Occupancy SAME

Owner of Building FREDERICK STRATION III AND KERI SARAJIAN

Building Owner's Address 3009 N. HACKETT AVE MILWAUKEE WI 53211
(Street) (City)

Building Owner's Telephone No. 414-534-6695

Name of Business or Firm.....

Location of Business or Firm in Building.....

Telephone No. of Business or Firm.....

Maximum Number of Employees — Male..... Female.....

Former Address of Business or Firm.....

Business or Firm Owner's Name.....

Owner's Residence Address.....
(Street) (City)

Owner's Residential Telephone No.....

If certificate of occupancy will not be issued unless repairs or alterations are performed, they will be made by:

Applicant..... Owner SS Occupant..... Other.....

JP Stratton III
Applicant's Signature

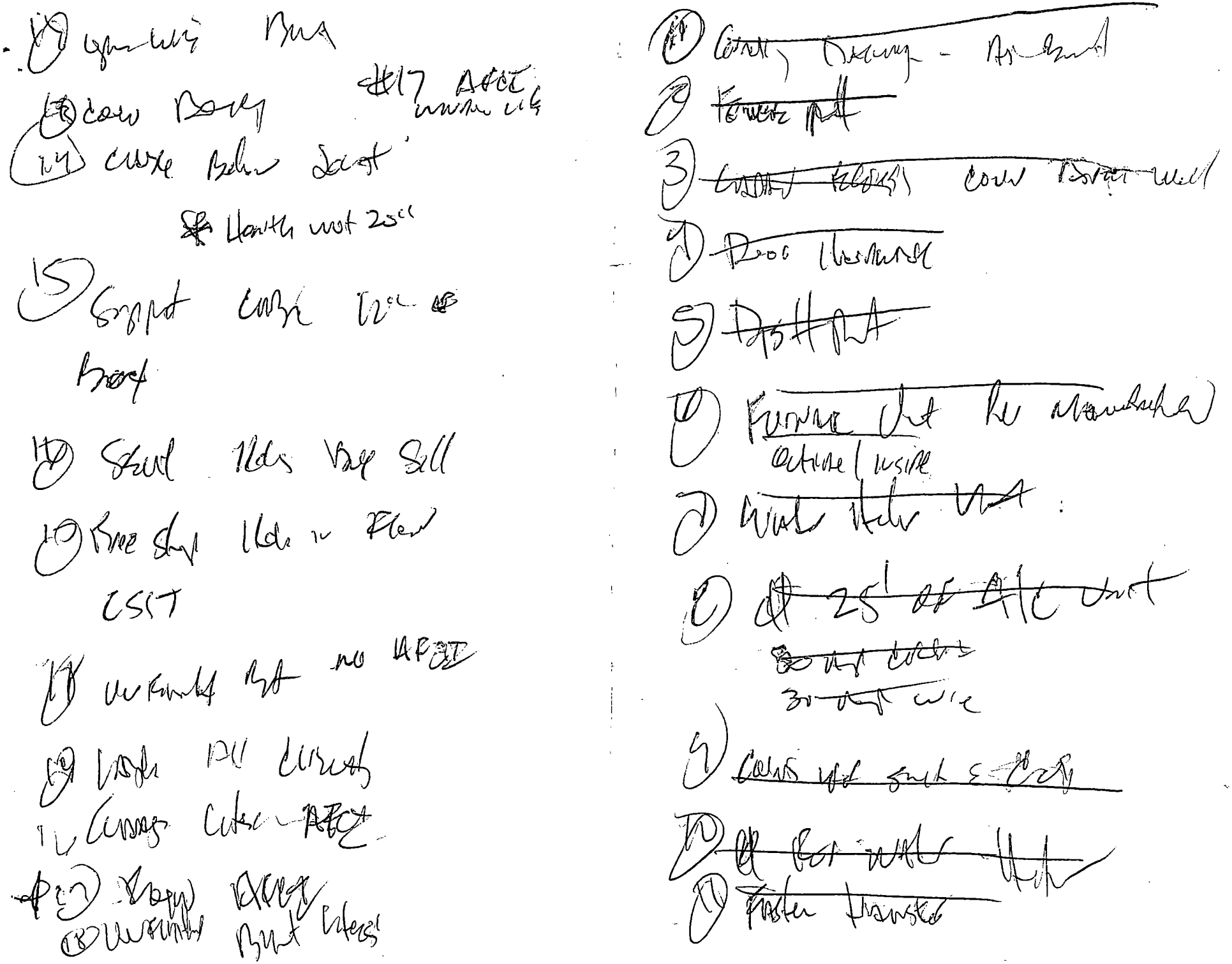
2/24/14
Approved Date

$100
Fee

Permit Issued

Rec #45403

On Hold
See Attached



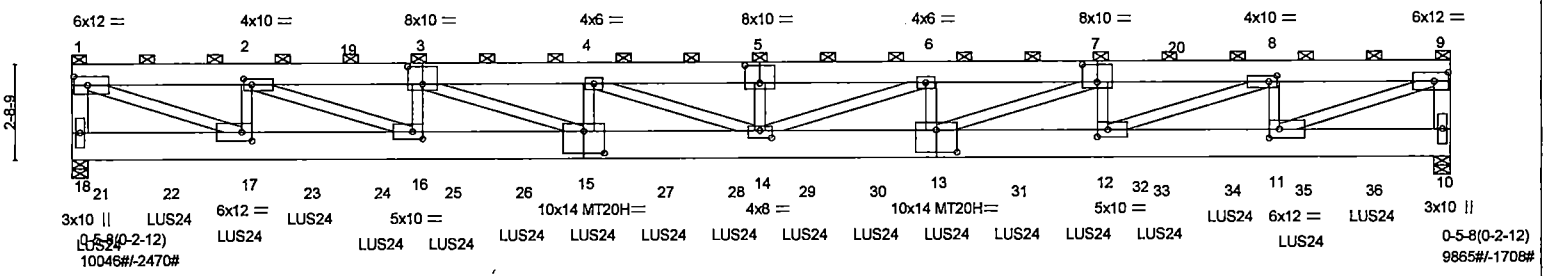
Job	Truss	Truss Type	Qty	Ply	Stratton Residence
R-5902-13	FG	FLAT GIRDER	1	3	Job Reference (optional)

Accurate Housing Systems Inc., East Troy, WI, Matt Barker — 7.430 s Jul 25 2013 MiTek Industries, Inc. Thu Jan 02 08:54:48 2014 Page 1
ID:dKBECIYCF3wFjS4?RITmqzze1kC-xtWjxI8Df4IWoTxuw1qJq4KAc0PkiFKLuJSYOkzzRn5

4-11-13	9-9-14	14-7-15	19-6-0	24-4-1	29-2-2	34-0-3	39-0-0
4-11-13	4-10-1	4-10-1	4-10-1	4-10-1	4-10-1	4-10-1	4-11-13

Scale = 1:65.0

MARK "UP" ON TRUSSES IN SHOP!!



4-11-13	9-9-14	14-7-15	19-6-0	24-4-1	29-2-2	34-0-3	39-0-0
4-11-13	4-10-1	4-10-1	4-10-1	4-10-1	4-10-1	4-10-1	4-11-13

Plate Offsets (X,Y): [1:0-4-12,0-3-0], [2:0-2-12,0-2-0], [3:0-5-0,0-6-0], [5:0-5-0,0-6-0], [7:0-5-0,0-6-0], [8:0-2-12,0-2-0], [9:0-4-12,0-3-0], [11:0-3-8,0-3-0], [12:0-3-8,0-2-8], [13:0-7-0,0-7-8], [14:0-4-0,0-2-8], [15:0-7-0,0-7-8], [16:0-3-8,0-2-8], [17:0-3-8,0-3-0]

LOADING (psf)		SPACING 8-2-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase	1.15	TC	0.62	Vert(LL)	-1.22	14	>378	360	MT20	197/144
TCDL	10.0	Lumber Increase	1.15	BC	0.71	Vert(TL)	-1.66	14	>279	180	MT20H	148/108
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.82	Horz(TL)	0.15	10	n/a	n/a		
BCDL	10.0	Code IRC2006/TPI2002		(Matrix-M)		Wind(LL)	0.51	14	>911	240	Weight: 995 lb	FT = 10%

LUMBER
TOP CHORD 2x8 SP 2400F 2.0E
BOT CHORD 2x10 SP 2400F 2.0E
WEBS 2x4 SPF Stud *Except*
W1: 2x6 SPF No.2, W2: 2x4 SPF 2100F 1.8E, W4: 2x4 SPF 1650F 1.5E

BRACING
TOP CHORD 2-0-0 oc purlins (5-6-11 max.), except end verticals
(Switched from sheeted: Spacing > 2-0-0).
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 18=10046/0-5-8, 10=9865/0-5-8
Max Horz 18=239(LC 4)
Max Uplift18=-2470(LC 3), 10=-1708(LC 4)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-18=-9092/2177, 1-2=-20034/4838, 2-19=-33963/8025, 3-19=-33963/8025, 3-4=-42692/9814, 4-5=-45323/10006, 5-6=-45323/10006, 6-7=-42571/8820, 7-20=-33720/6552, 8-20=-33720/6552, 8-9=-19875/3698, 9-10=-9024/1689
BOT CHORD 18-21=-436/1111, 21-22=-436/1111, 17-22=-436/1111, 17-23=-4942/20034, 23-24=-4942/20034, 16-24=-4942/20034, 16-25=-8209/34338, 25-26=-8209/34338, 15-26=-8209/34338, 15-27=-9951/42881, 27-28=-9951/42881, 14-28=-9951/42881, 14-29=-8952/42763, 29-30=-8952/42763, 13-30=-8952/42763, 13-31=-6690/34100, 31-32=-6690/34100, 12-32=-6690/34100, 12-33=-3712/19875, 33-34=-3712/19875, 11-34=-3712/19875, 11-35=-248/1104, 35-36=-248/1104, 10-36=-248/1104
WEBS 1-17=-4931/20569, 2-17=-7282/1686, 2-16=-3485/15101, 3-16=-5224/1127, 3-15=-1903/9146, 4-15=-2640/458, 4-14=-198/2647, 5-14=-1434/330, 6-14=-1255/2775, 6-13=-2684/872, 7-13=-2408/9274, 7-12=-5285/1339, 8-12=-3125/15010, 8-11=-7247/1578, 9-11=-3765/20403

NOTES
1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x8 - 2 rows staggered at 0-9-0 oc.
Bottom chords connected as follows: 2x10 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; enclosed; MWFRS (low-rise) gable end zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
4) Provide adequate drainage to prevent water ponding.
5) All plates are MT20 plates unless otherwise indicated.
6) Plates checked for a plus or minus 5 degree rotation about its center.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 18=2470, 10=1708.
10) This truss is designed in accordance with the 2006 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
11) "Semi-rigid pitchbreaks including heels" Member end fixity model was used in the analysis and design of this truss.
12) Use Simpson Strong-Tie LUS24 (4-10d Girder, 2-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 0-9-4 from the left end to 36-9-4 to connect truss(es) D2 (1 ply 2x4 SPF), D3 (1 ply 2x4 SPF), D4 (1 ply 2x4 SPF), D5 (1 ply 2x4 SPF), D6 (1 ply 2x4 SPF) to front face of bottom chord.
13) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard
[redacted] + Roof Live (balanced) + Uninhab. Attic Storage: Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-9=-327, 10-18=-82
Concentrated Loads (lb)
Vert: 15=-224(F) 17=-219(F) 13=-249(F) 21=-219(F) 22=-219(F) 23=-224(F) 24=-224(F) 25=-224(F) 26=-224(F) 27=-224(F) 28=-224(F) 29=-224(F) 30=-224(F) 31=-193(F) 32=-193(F) 33=-193(F) 34=-193(F) 35=-193(F) 36=-287(F)

(Greg) (915-7493)

1) Plaster fire on outside wall - Collapse easily
2) Cut Truss reinforces with wafer board (East Second Story)
3) Outlet missing from Bedroom (East upstairs) (Exceeds 2'00" of wall space). (4½')

~~4) Smoke detector for door~~

4) ~~(2) 2ply 2's Same 1'5" 4 carry roof Trusses (Second floor) West (Stairs)~~
3 Ply Roof truss for to be tied down with 3 H+S3
5) Insulate Garage R-4.2 Reading Strips R-8 used (2).
SR 322.42

6) Need it in Dining 6L12 ceiling

7) 4 Ply Beam @ Stairway
Framed Differently than Plan
Currently supported by 2-ply Beam

8)

9) PVC cla type

11-22-13 Scott,

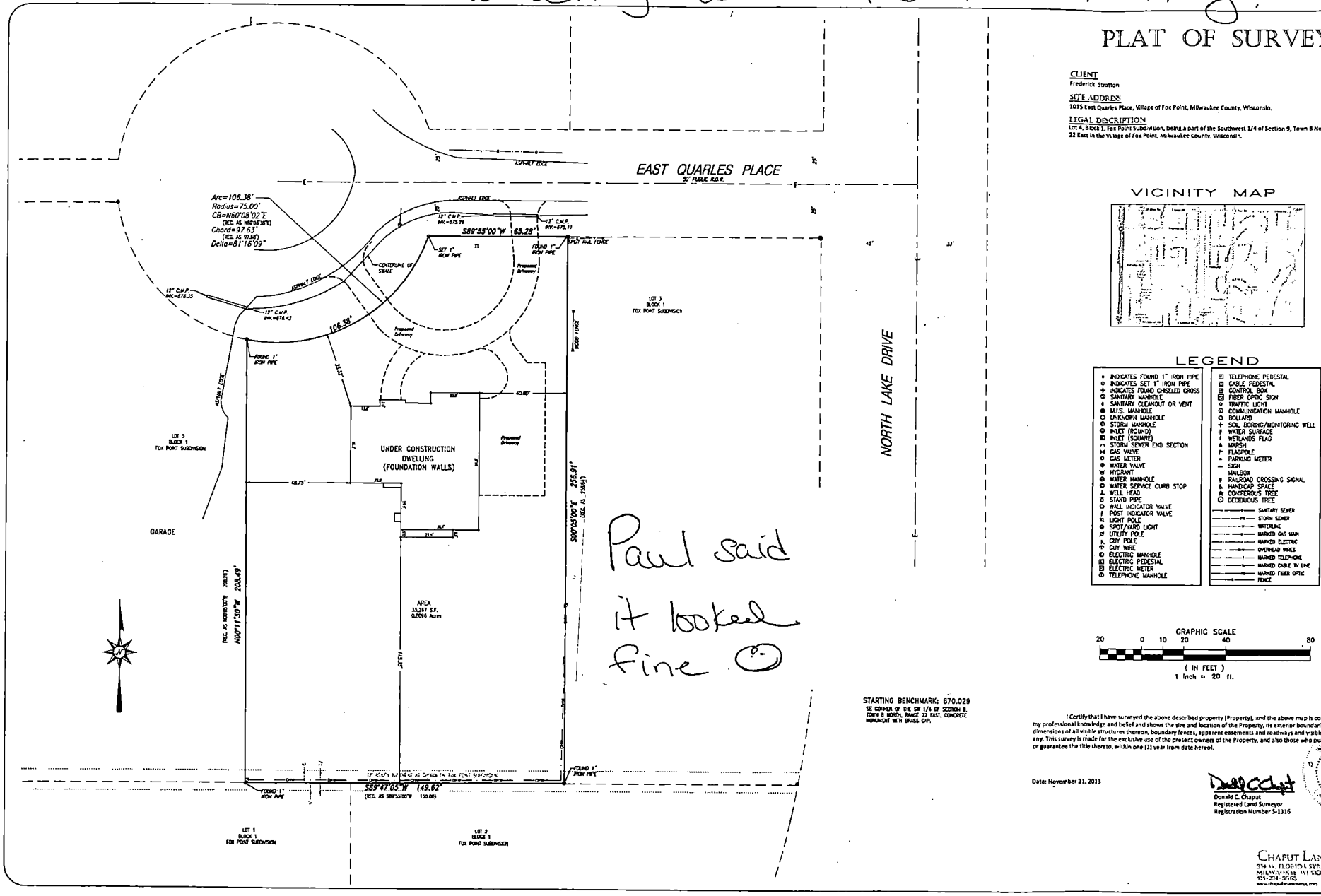
Independent Picked up larger AS-BUILT to verify will return later on monday.

PLAT OF SURVEY

CLIENT
Frederick Stratton

SITE ADDRESS
1015 East Quarles Place, Village of Fox Point, Milwaukee County, Wisconsin.

LEGAL DESCRIPTION
Lot 4, Block 1, Fox Point Subdivision, being a part of the Southwest 1/4 of Section 9, Town 8 North, Range 22 East in the Village of Fox Point, Milwaukee County, Wisconsin.



Paul said it looked fine ☺

STARTING BENCHMARK: 670.029
SE CORNER OF THE SW 1/4 OF SECTION 9, TOWN 8 NORTH, RANGE 22 EAST, CONCRETE MONUMENT WITH BRASS CAP.

VICINITY MAP

LEGEND

- INDICATES FOUND 1" IRON PIPE
- INDICATES SET 1" IRON PIPE
- INDICATES FOUND CHISELED CROSS
- SANITARY MANHOLE
- SANITARY CLEANOUT OR VENT
- M.I.S. MANHOLE
- UNKNOWN MANHOLE
- STORM MANHOLE
- INLET (ROUND)
- INLET (SQUARE)
- STORM SEWER END SECTION
- GAS VALVE
- GAS METER
- WATER VALVE
- HYDRANT
- WATER MANHOLE
- WATER SERVICE CURB STOP
- WELL HEAD
- STAND PIPE
- WALL INDICATOR VALVE
- POST INDICATOR VALVE
- LIGHT POLE
- SPOT/YARD LIGHT
- UTILITY POLE
- GUY POLE
- GUY WIRE
- ELECTRIC MANHOLE
- ELECTRIC PEDESTAL
- ELECTRIC METER
- TELEPHONE MANHOLE
- TELEPHONE PEDESTAL
- CABLE PEDESTAL
- CONTROL BOX
- FIBER OPTIC SIGN
- TRAFFIC LIGHT
- COMMUNICATION MANHOLE
- BOLLARD
- SOIL BORING/MONITORING WELL
- WATER SURFACE
- WETLANDS FLAG
- MARSH
- FLAGPOLE
- PARKING METER
- SIGN
- MAILBOX
- RAILROAD CROSSING SIGNAL
- HANDICAP SPACE
- CONIFEROUS TREE
- DECIDUOUS TREE
- SANITARY SEWER
- STORM SEWER
- WATERLINE
- MARKED GAS MAIN
- MARKED ELECTRIC
- OVERHEAD WIRES
- MARKED TELEPHONE
- MARKED CABLE TV LINE
- MARKED FIBER OPTIC
- FENCE

GRAPHIC SCALE
20 0 10 20 40 80
(IN FEET)
1 inch = 20 ft.

I Certify that I have surveyed the above described property (Property), and the above map is correct to the best of my professional knowledge and belief and shows the size and location of the Property, its exterior boundaries, the location and dimensions of all visible structures thereon, boundary fences, apparent easements and roadways and visible encroachments, if any. This survey is made for the exclusive use of the present owners of the Property, and also those who purchase/mortgage, or guarantee the title thereto, within one (1) year from date hereof.

Date: November 21, 2013

Donald C. Chaput
Registered Land Surveyor
Registration Number S-1316

CHAPUT LAND SURVEYS LLC
214 W. FLORIDA STREET
MILWAUKEE, WI 53204
414-254-9068

Drawing No. 1477-plat/tm

CHAPUT LAND SURVEYS LLC

Plumber Clarence N Hahn No. 859 (O.K.)

Drainlayer ?

Address 7904 W Brooks

Application and Record

Owner R E Barbee

Address 1015 E Burnham Pl

Fox Point, Wis., Nov 8 1947

To the VILLAGE OF FOX POINT, PLUMBING AND WATER INSPECTION DEPARTMENT: The undersigned hereby make application to do the work of plumbing consisting of

laying a 4 inch clay drain pipe from **Main** to **Curb** to **lot line** to premises at No. 1015 E Burnham Pl.

Remarks: 0

laying a 1 inch copper service pipe from **Main** to **Curb**; a 1" inch service pipe from **curb** to **building** at No. 1015 E Burnham Pl.

Remarks: 0

PERMITS ISSUED

Kind	No.
Sewer and Plumbing	799
Water	543
Street	542
Meter	528

the following premises owned by

Name of owner Address

Description	Lot	Block
Lge Pump Set. 10 room S W 1/4 Sec 5 T 8 N-R 22 E	4	1

In the performance of this work the undersigned Plumber or Drain Layer hereby agree to be bounden by and submit to all statutes, city ordinances, and rules and regulations prescribed by the Village Board for the government of Plumbers and House Drain Layers.

License No. 3317 Clarence N Hahn Plumber

FIXTURES WITH DRAIN OR WATER CONNECTIONS

	No.		No.
Bath Tubs	1	Wash Basins	2
Ice Box		Water Closets	2
Laundry Tubs	1	Showers	
Sanitary Bubblers		Basement Drains	1
Sinks	1	H W-HEATER	1
Urinals			

FEES

Setting Pump 1.16

Storm Sewer Connection $

Sanitary Sewer Connection 3 8.30

Water Connection

Fixtures 8 @ 50 4.00

Hot Water Heater 7.00

Water Meter 3/4" Meter 2.00

Total 11.46

Deposit to cover street repairs 25.00

O K Plank Permit Clerk

A 1" inch Copper water service pipe was laid in E. Burnham Pl.

Curb box is located 6 ft feet E. of Lot 233 feet of

A 6 inch Clay sanitary sewer connection was made in Virginia Rd. E. of

feet S. of manhole East on E. Burnham Pl.

A inch storm sewer connection was made in

feet of manhole

Outside Drain	House Drain	Report	Inspection Soil and Under Floor	Report	Final Inspection	Return	Water
							On
					10/1/48		On
			14 Sufficient				Off
							Off

Installation Approved O K Plank 10/1/48 Application Approved O K Plank Nov 8 1948

W 36 S 74

O K Plank Water and Plumbing Inspector

REMARKS

3/4" Badger Meter #3063854

1 1/6 Pressure.

50 — 500

66 — 25 330

8.30

Water for Const of Clay Pd. by Plumber 26.00 4 ft.

#26.50 Permit #375

COMPLAINT RECORD

Water turned Sep. 11/18/47

Stop Box 8 1/2 N. of Col Line on 32 1/2 S. of water gate.

Sewer Sep. 11/17/47

Meter Set 10/1/48

Water Tower

Owner R. E. Bethke

Plumber Clarence N. Hahn

W-543
Permit No. S-799 859 Entered

Street 1015 E. Quarles Place

775

Plumber J C Sebolsky No. 2681

Drainlayer

Address 4924 - 91st

Tel. No. Sp 1-2800

Application and Record

Owner H Friedlen

Address 1015 - Quarles

Date 9-11-57, 19

To the VILLAGE OF FOX POINT, PLUMBING AND WATER INSPECTION DEPARTMENT: The undersigned hereby make application to do the work of plumbing - drainlaying - consisting of

laying a ..... inch ..... drain pipe from Main to Lot line to Building

laying a ..... inch ..... service pipe from Main to Lot line to Building

at

1015 - E Quarles Fox Point, Wis.

Address at which work is to be done

PERMITS ISSUED	
Kind	No.
Sewer and Plumbing	2658
Water	—
Street	—
Meter	—
Water Usage	—

Subdivision	Lot	Block

In the performance of this work the undersigned Plumber or Drain Layer hereby agrees to be bounden by and submit to all statutes, city ordinances, and rules and regulations prescribed by the Village Board for the government of Plumbers and House Drain Layers.

License No. 2967 J C Sebolsky Plumber

FIXTURES WITH DRAIN OR WATER CONNECTIONS

	No.		No.
Hose Bibs		Dishwasher	1
Bath Tubs		Wash Basins	
Sump Pump		Water Closets	
Laundry Tubs	1	Showers	
Sanitary Bubblers		Basement Drains	
Sinks		Garbage Disposal	1
Water Heater		Sprinkling System	

FEES

Water Usage	$
Sanitary Sewer Connection	
Water Connection	
Water Heater	
Fixtures	$1.50
Water Meter	
Total	Min Fee $3.00
Deposit to cover street repairs	—

Earl H. Plank Permit Clerk

A ..... inch ..... water service pipe was laid in .....

Curb box is located ..... feet ..... of ..... feet ..... of .....

..... inch ..... Water Meter No. ..... Date Installed .....

A ..... inch ..... sanitary sewer connection was made in .....

..... feet ..... of manhole .....

A ..... inch ..... storm sewer connection was made in .....

..... feet ..... of manhole .....

Outside Sewer	Report	Inside Sewer	Report	Rough In Plumbing Inspection	Report	Final Inspection	Report
						2-27-58	OK.

Installation Approved 2-27-58 Application Approved 9-11-57, 19

As Built .....

Water and Plumbing Inspector

REMARKS

Installing additional fixtures in Old House.

DISCREPANCY RECORD

W.U. Rec #3834

9/11/57

Owner N. Fiedler

Plumber J. C. Lebolsky

Permit No. 2681

Street 1015 E. Ogden Pl.

775

Filing Fee $75 Rec# 439837

Village of Fox Point
7200 N. Santa Monica Blvd.
Fox Point, WI 53217
(414) 351-8900

Date Submitted 10-17-13 No. 15547

APPLICATION FOR BUILDING

The undersigned hereby applies for a permit to build, in accordance with the information tabulated hereafter,

Type of Project RESIDENCE (Residence, Garage, Store, Office, School, Fence, Shed, Sign, Swimming Pool, Etc.) Address 1015 EQUARLES PL

Lot _____ Block _____ Subdivision _____ District _____

Does contemplated structure violate the Village zoning ordinance? NO

Height of Structure 29'6" - 2 STORY (stories or feet)

Width (parallel to highway) EW 60'6" (feet) Depth (perpendicular to highway) NS 65' (feet)

Distance: Street Line to Front Line of Structure 25.22' (feet)

Distance: Side Lot Line to Structure 47.22' @ WEST 40.68 @ EAST

Type of Construction: FRAME (Frame, Brick-tile, etc.) Exterior finish SIDING / STONE VENEER (Stucco, Siding, Brick Veneer, Etc.)

Height of front yard above street grade _____

Number of rooms 9 Baths 4 1/2

Estimated cost Garage 3 CAR

Building T.B.D.

Structure _____

Is there a private garage? YES ATTACHED

Does the contemplated garage violate the Village zoning ordinance? _____

Size 23X39' Number of Stalls 3 Where Situated ATTACHED

Have plans been submitted to the Wisconsin Department of Industry, Labor and Human Relations for examination and approval? NO

Have plans been approved as being in compliance with all applicable sections of the Wisconsin Administrative code? NO

Herewith are filed the following duplicate plans _____ in number, which I certify I will conform to in the work hereby applied for:

Remarks: NEW HOME W/ ATTACHED GARAGE

Herewith are filed the specifications that describe the work in question and as shown on plans above submitted.

In making the application the undersigned agrees to obey the Fox Point Building and Zoning Codes pertaining to the erection of all structures and also agrees to obey all other ordinances of the Village of Fox Point.

The undersigned, owner or being duly authorized so to do, hereby gives express authorization to the Village of Fox Point, its officers, agents and employees, to enter upon the premises herein described and fill up any excavation, or tear down, remove or enclose the unfinished structure for which a permit is herein requested in the event of cessation of the building, whenever the Building Inspector shall determine that such premises in the unfinished condition of the structure are dangerous to members of the public, including children, even though trespassers. The undersigned further hereby waives all statutory notices and consents to the determination by the Village Board and the levy and placing upon the tax roll of a special assessment in the amount of the cost to the Village, including customary Village overhead charges incurred in filling up any such excavation or tearing down, removing or enclosing any such unfinished structure.

We hereby agree to provide a house number plate or sign readily observable from the public highway which will be installed not less than 15 days after the structure is occupied.

Owner of Structure RICK STRATTON Arch. or Contr. PEET SHIELDS ARCHITECTURE JOY PEET-SHIELDS AIA

Address 3009 N HACKETT AVE Address 3033 N. HACKETT AVE

City MILWAUKEE State WI Zip 53211 City MILWAUKEE State WI Zip 53211

Phone 414-534-6695 Phone 414 313 2306

Size of Structure _____ (sq. ft.) Permit Fee 1930 Receipt 45403 11/1/13

Dwelling Contractor Certification No. _____ Expires _____

Dwelling Contractor Qualifier Certification No. _____ Expires _____

Building Contractor Certification No. _____ Applicant Signature _____ (Architect, Owner, Builder)

Date of Approved 11/1/13 (Builder Inspector)

INSPECTION REPORT AND NOTICE OF NONCOMPLIANCE

Report Date:	Inspection Date	Permit No.:	State Seal #	Parcel No:
10/15/14	10/15/14	15547	428230	

Project Address	Subdivision	Lot No.:	Block No.:
1015 E Glencoe Place			

Inspection Type(s): ☐ Footing ☐ Erosion Control ☐ Foundation ☐ Bsmt Drain Tile ☐ Underslab Plbg ☐ Rough HVAC
☒ Rough Plumbing ☒ Rough Electrical ☒ Construction ☐ Insulation/Energy ☐ Final ☐ Other:

Area Inspected, if Partial Inspection:

If Final Inspection, Occupancy May: ☐ Take Place Now ☐ Take Place Temporarily for _____ days
☐ Not Take Place Until The Items Below Are Corrected and Inspected
☐ Other:

Owner:	**Contractor:**
Rick Strathaw	Applebrook Construction
3009 N Hackett Ave.	3430 W. County Line Road
Milwaukee, WI 53211	Mequon, WI 53092

AN INSPECTION OF THE ABOVE PREMISES HAS DISCLOSED THE FOLLOWING NONCOMPLIANCES: ☐ **None Noted**

ORDER NO.	CODE SECTION	FINDINGS AND REQUIREMENTS
1	SPS 322.37(3)	Remove Plaster ring From thermal Envelope
2	SPS 321.27(2)b	Provide Engineering For Floor Alteration/modified trusses
3	NEC 210.52(A)(1)	Provide Additional outlet in Second Story (East) Bedroom (6-12 note)
4	SPS 321.27(3)	Provide Additional restraint For 3-ply Beam (uplift)
5	NEC 210.52(A)(1)	Provide Additional outlet in Dining Room (6/12 note)
10	SPS 322.10(2)	Provide revised Plans For Stairway Alterations
7	SPS 382.31(6)(a)	Extend Soil Stack Through the roof System
8	SPS 321.27(7)	Provide Manufactured Exterior Trusses Bracing As Per The truss Manufacturer

IMPORTANT: Please report when violations are corrected. AVOID DELAY

NOTICE OF NONCOMPLIANCE: All cited violations shall be corrected within 30 days after written notification unless an extension of time is granted. Each day that the violation continues after notice shall constitute a separate offense and is subject to remedies and penalties by the authority having jurisdiction. Appeals per ch. 68, WI Stats. and s. Comm 20.21.

Enforcing Jurisdiction:	Bldg Location Muni #	Authority By Municipal Ordinance Section::
☐ Town ☒ Village ☐ City ☐ County **OF:** ☐ State Staff ☐ State Insp Agency#	41 -- 126	

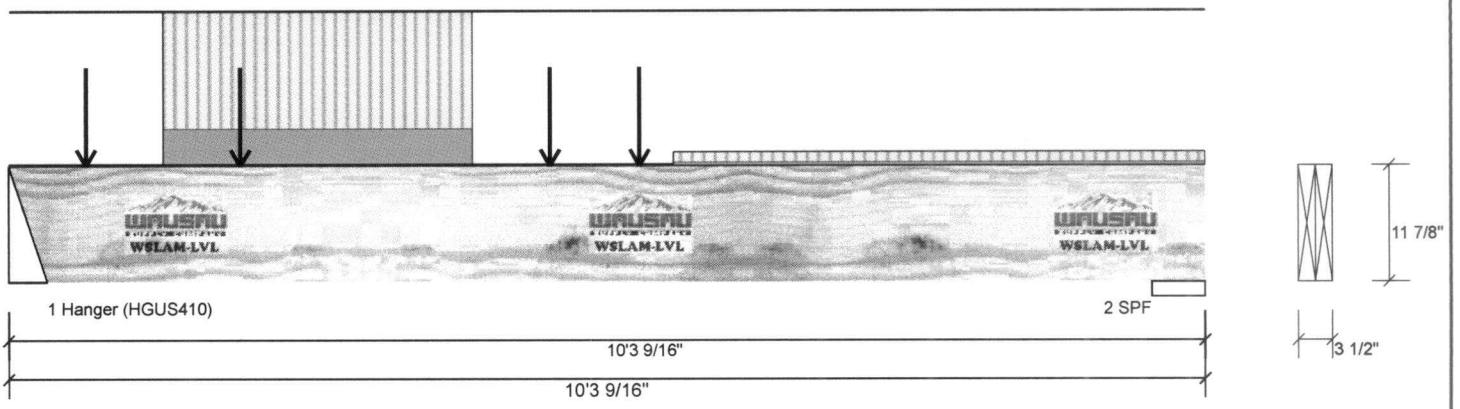
Inspector's Name:	Violations Explained To:	Compliance Date:
Scott Miller	Contractor	30 DAYS

Inspector's Address:	Office Hours:	Telephone No:
Village Hall 7200 W South Milwaukee Blvd	8-930AM/1230-130PM	(414) 351-8840

Orders Referred for Followup Legal Action To:	Date	Noncompliances Verified to Still Exist? (If needed, notate orders above.) ☐ Yes ☐ No	Additional Fees Collected(+)/Refunded(-) By State-Contracted Agency $ Since Original Permit Issuance:

Distribution: ☐Ply 1 – Contractor ☐Ply 2 – Inspector/State ☐Ply 3 - Owner ☐Ply 4 - File

	Client Butler chase		Shipping
Project Name: Butler Chase stratton	Job#: Stratton	Quantity 1 (2pcs.)	Description: second Floor
G8-C 2.0E WS-LAM LVL 1.750" X 11.875" 2-Ply - PASSED			10/20/2014 8:34 AM Page 1 of 1 Designer:



Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC 2012 / IRC
Deflection LL:	360	Load Sharing:	No
Deflection TL:	240	Deck:	3/4" OSB Nailed and Glued
Importance:	Normal	Vibration:	Not Checked
Temperature:	Temp <= 100°F		

Reactions

Brg	Live	Dead	Snow	Wind	Const
1	2811	941	0	0	0
2	1781	641	0	0	0

Bearings

Bearing	Input Length	In Analysis	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - Hanger	4.000"	1.500"	84%	941 / 2811	3751	L	D+L
2 - SPF	5.500"	1.750"	93%	641 / 1781	2421	L	D+L

Analysis	Actual	Location	Allowed	Capacity	Load Comb.	Ld. Case
Moment	10258 ft-lb	5'3 3/4"	21295 ft-lb	0.482 (48%)	D+L	L
Shear	3144 lb	1'1 5/8"	7897 lb	0.398 (40%)	D+L	L
LL Defl inch	0.104 (L/1108)	4'11 3/16"	0.321 (L/360)	0.320 (32%)	L	L
TL Defl inch	0.141 (L/822)	4'11 3/16"	0.482 (L/240)	0.290 (29%)	D+L	L

Design OK.

Design Notes

1 Girders are designed to be supported on the bottom edge only.
2 Multiple plies must be fastened together as per manufacturer's details.
3 Top loads must be supported equally by all plies.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Const.	Comments
1	Tie-In	0-0-0 to 2-0-0	(Span)0-1-12	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
2	Tie-In	0-0-0 to 10-3-9	(Span)0-1-12	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
3	Point	0-8-0		Far Face	99 lb	331 lb	0 lb	0 lb	0 lb	J5
4	Point	0-8-0		Near Face	126 lb	425 lb	0 lb	0 lb	0 lb	
5	Part. Uniform	1-4-0 to 4-0-0		Far Face	90 PLF	298 PLF	0 PLF	0 PLF	0 PLF	
6	Point	2-0-0		Near Face	76 lb	256 lb	0 lb	0 lb	0 lb	
7	Tie-In	2-0-0 to 3-4-0	(Span)0-1-12	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
8	Tie-In	3-4-0 to 5-8-12	(Span)0-1-12	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
9	Point	4-8-0		Far Face	94 lb	314 lb	0 lb	0 lb	0 lb	J5
10	Point	5-5-4		Far Face	792 lb	2302 lb	0 lb	0 lb	0 lb	G7
11	Tie-In	5-8-12 to 10-3-9	(Span)1-4-0	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
	Self Weight				11 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

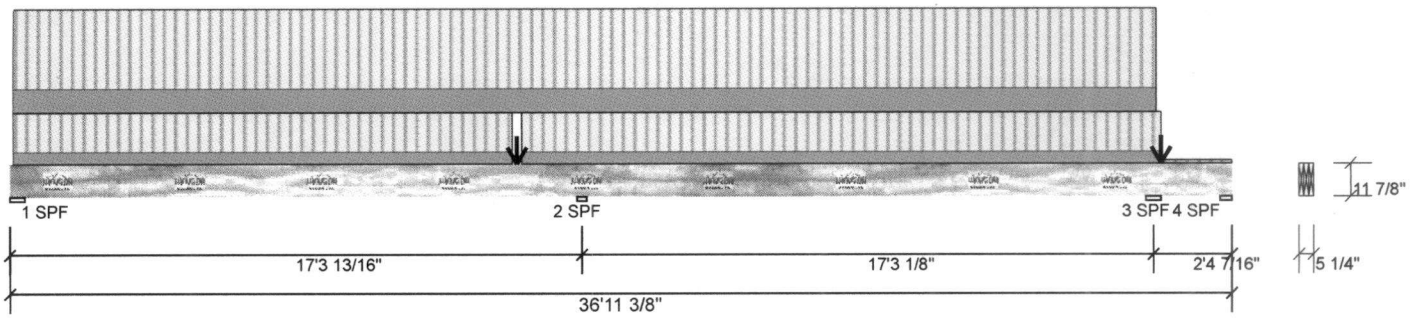
1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation
6. For flat roofs provide proper drainage to prevent ponding

Wisconsin Building Supply
W232 N 5700 HWY 74, Wi
US
53089

	Client Butler chase		Shipping
Project Name: Butler Chase stratton	Job#: Stratton	Quantity 1 (3pcs.)	Description: second Floor
G2-A 2.0E WS-LAM LVL 1.750" X 11.875" 3-Ply - PASSED			10/20/2014 8:34 AM Page 1 of 1 Designer:



Type:	Girder	Application:	Floor
Plies:	3	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC 2012 / IRC
Deflection LL:	360	Load Sharing:	Yes
Deflection TL:	240	Deck:	3/4" OSB Nailed and Glued
Importance:	Normal	Vibration:	Not Checked
Temperature:	Temp <= 100°F		

Reactions

Brg	Live	Dead	Snow	Wind	Const
1	578	275	0	0	0
2	3896	1587	0	0	0
3	2682	749	0	0	0
4	779	-95	0	0	0

Bearings

Bearing	Input Length	In Analysis	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	1.500"	25%	275 / 578	853	L_L	D+L
2 - SPF	3.500"	3.500"	70%	1587 / 3896	5483	LL_	D+L
3 - SPF	5.500"	5.500"	28%	749 / 2682	3431	_LL	D+L
4 - SPF	4.250"	1.500"	24%	-95 / 779	684 (-813)	L_L	D+L

Analysis	Actual	Location	Allowed	Capacity	Load Comb.	Ld. Case
Neg Moment	-6122 ft-lb	17'3 13/16"	33220 ft-lb	0.184 (18%)	D+L	LL_
Moment pos	4052 ft-lb	10'1 9/16"	33220 ft-lb	0.122 (12%)	D+L	L_L
Shear	4312 lb	16'3 15/16"	11845 lb	0.364 (36%)	D+L	LL_
LL Defl inch	0.099 (L/2059)	9'3"	0.564 (L/360)	0.170 (17%)	L	L_L
TL Defl inch	0.135 (L/1501)	9'1 5/16"	0.846 (L/240)	0.160 (16%)	D+L	L_L

Design OK.

Design Notes

1 Girders are designed to be supported on the bottom edge only.
2 Multiple plies must be fastened together as per manufacturer's details.
3 Top loads must be supported equally by all plies.
4 Tie-down connection required at bearing 4 for uplift 813 lb (Combination D+L, Load Case _L_).

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Const.	Comments
1	Tie-In	0-1-4 to 15-2-8	(Span) 0-10-10	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
2	Tie-In	0-1-4 to 34-7-15	(Span)1-9-6	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
3	Point	15-4-4		Far Face	941 lb	2811 lb	0 lb	0 lb	0 lb	G8
4	Tie-In	15-6-0 to 34-9-11	(Span) 0-10-10	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
5	Tie-In	34-9-11 to 36-11-6	(Span)0-0-10	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
6	Point	34-9-11		Near Face	435 lb	1445 lb	0 lb	0 lb	0 lb	
	Self Weight				16 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation
6. For flat roofs provide proper drainage to prevent ponding

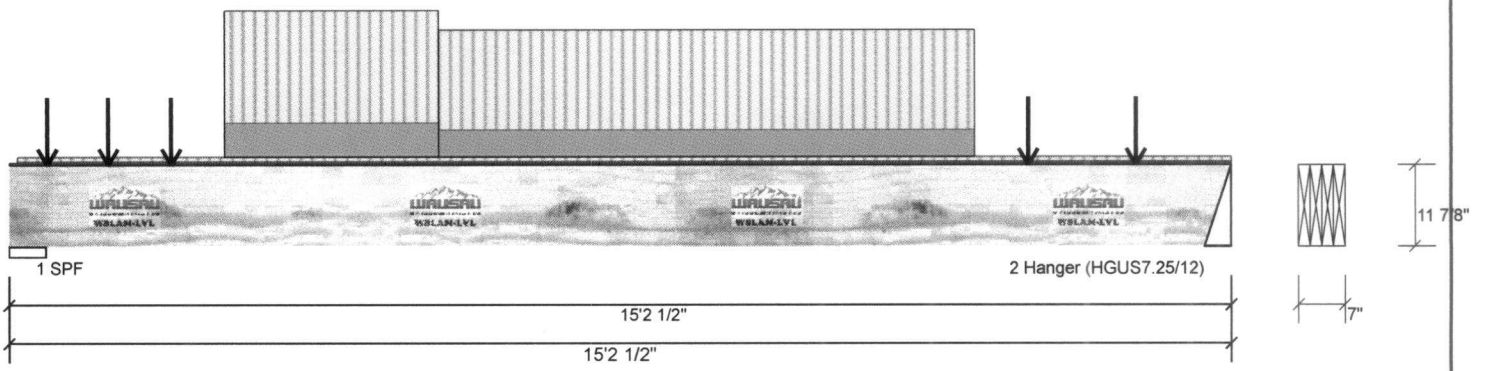
Wisconsin Building Supply
W232 N 5700 HWY 74, Wi
US
53089

La Ann Price
W232 N 5700 HWY 74
Sussex,WI 53089
262-246-0611

	Client Butler chase		Shipping
Project Name: Butler Chase stratton	Job#: Stratton	Quantity 1 (4pcs.)	Description: second Floor

G7-A 2.0E WS-LAM LVL 1.750" X 11.875" 4-Ply - PASSED

10/20/2014 8:33 AM
Page 1 of 2
Designer:



Type:	Girder	Application:	Floor
Plies:	4	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC 2012 / IRC
Deflection LL:	360	Load Sharing:	Yes
Deflection TL:	240	Deck:	3/4" OSB Nailed and Glued
Importance:	Normal	Vibration:	Not Checked
Temperature:	Temp <= 100°F		

Reactions

Brg	Live	Dead	Snow	Wind	Const
1	2908	1090	0	0	0
2	2302	792	0	0	0

Bearings

Bearing	Input Length	In Analysis	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	5.500"	1.500"	90%	1090 / 2908	3998	L	D+L
2 - Hanger	4.000"	1.500"	35%	792 / 2302	3094	L	D+L

Analysis	Actual	Location	Allowed	Capacity	Load Comb.	Ld. Case
Moment	13124 ft-lb	7'6"	44293 ft-lb	0.296 (30%)	D+L	L
Shear	3515 lb	1'4 5/8"	15794 lb	0.223 (22%)	D+L	L
LL Defl inch	0.182 (L/957)	7'7 1/2"	0.485 (L/360)	0.380 (38%)	L	L
TL Defl inch	0.244 (L/716)	7'7 1/2"	0.727 (L/240)	0.340 (34%)	D+L	L

Design OK.

Design Notes

1 Girders are designed to be supported on the bottom edge only.
2 Multiple plies must be fastened together as per manufacturer's details.
3 Top loads must be supported equally by all plies.

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Const.	Comments
1	Tie-In	0-0-0 to 2-0-4	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
2	Tie-In	0-1-4 to 15-2-8	(Span)0-9-4	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
3	Point	0-5-12		Far Face	165 lb	235 lb	0 lb	0 lb	0 lb	G3
4	Point	1-2-14		Far Face	91 lb	303 lb	0 lb	0 lb	0 lb	J7
5	Tie-In	2-0-4 to 3-4-4	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
6	Point	2-0-4		Far Face	125 lb	415 lb	0 lb	0 lb	0 lb	J7
7	Part. Uniform	2-8-4 to 5-4-4		Far Face	118 PLF	392 PLF	0 PLF	0 PLF	0 PLF	
8	Tie-In	3-4-4 to 4-8-4	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
9	Tie-In	4-8-4 to 6-0-4	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
10	Part. Uniform	5-4-4 to 12-0-4		Far Face	93 PLF	348 PLF	0 PLF	0 PLF	0 PLF	
11	Tie-In	6-0-4 to 7-4-4	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
12	Tie-In	7-4-4 to 8-8-4	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	

Continued on page 2...

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

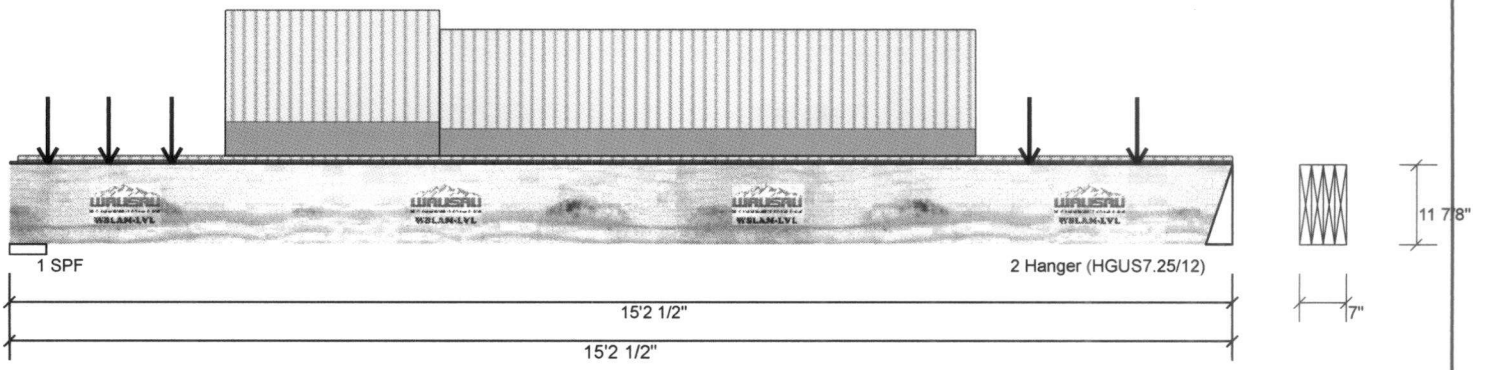
1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation
6. For flat roofs provide proper drainage to prevent ponding

Wisconsin Building Supply
W232 N 5700 HWY 74, Wi
US
53089

		Client Butler chase		Shipping
Project Name: Butler Chase stratton	Job#: Stratton	Quantity 1 (4pcs.)	Description: second Floor	
G7-A 2.0E WS-LAM LVL 1.750" X 11.875" 4-Ply - PASSED				10/20/2014 8:33 AM Page 2 of 2 Designer:



...Continued from page 1

ID	Load Type	Location	Trib Width	Side	Dead	Live	Snow	Wind	Const.	Comments
13	Tie-In	8-8-4 to 10-0-4	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
14	Tie-In	10-0-4 to 11-4-4	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
15	Tie-In	11-4-4 to 12-8-4	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
16	Tie-In	12-8-4 to 15-2-8	(Span)0-3-8	Top	12 PSF	40 PSF	0 PSF	0 PSF	0 PSF	
17	Point	12-8-4		Far Face	122 lb	456 lb	0 lb	0 lb	0 lb	J9
18	Point	14-0-4		Far Face	34 lb	114 lb	0 lb	0 lb	0 lb	J2
	Self Weight				22 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

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6. For flat roofs provide proper drainage to prevent ponding

Wisconsin Building Supply
W232 N 5700 HWY 74, Wi
US
53089

INSPECTION REPORT AND NOTICE OF NONCOMPLIANCE

Report Date:	Inspection Date	Permit No.:	State Seal #	Parcel No:
10/15/14	10/15/14	15547	428230	

Project Address	Subdivision	Lot No.:	Block No.:
1015 E Quaker Place			

Inspection Type(s): ☐ Footing ☒ Rough Plumbing ☐ Erosion Control ☒ Rough Electrical ☐ Foundation ☒ Construction ☐ Bsmt Drain Tile ☐ Insulation/Energy ☐ Underslab Plbg ☐ Final ☐ Rough HVAC ☐ Other:

Area Inspected, if Partial Inspection:

If Final Inspection, Occupancy May: ☐ Take Place Now ☐ Take Place Temporarily for ______ days ☐ Not Take Place Until The Items Below Are Corrected and Inspected ☐ Other:

Owner:	Contractor:
Rick Stratton	Applebrook Construction
3009 N Hackett Ave.	3430 W. County Line Road
Milwaukee, WI 53211	Mequon, WI 53092

AN INSPECTION OF THE ABOVE PREMISES HAS DISCLOSED THE FOLLOWING NONCOMPLIANCES: ☐ **None Noted**

ORDER NO.	CODE SECTION	FINDINGS AND REQUIREMENTS
1	SPS 322.37(3)	Remove Plastic 'ring' From thermal Envelope
2	SPS 321.27(2)b	Provide Engineering For Field 'Altered/modified trusses'
3	NEC 210.52(A)(1)	Provide additional outlet in Second Story (EAST) Bedroom (6-12 rule)
4	SPS 321.27(3)	Provide additional restraint for 3-ply Beam (uplift)
5	NEC 210.52(A)(1)	Provide additional outlet in Dining Room (6/12 rule)
6	SPS 320.10(2)	Provide 'revised' plans for Stairway Alterations
7	SPS 382.31(6)(a)	Extend 'Soil Stack' Through the roof System
8	SPS 321.27(7)	Provide additional Lateral Truss Bracing As Per the truss manufacturer

IMPORTANT: Please report when violations are corrected. AVOID DELAY

NOTICE OF NONCOMPLIANCE: All cited violations shall be corrected within 30 days after written notification unless an extension of time is granted. Each day that the violation continues after notice shall constitute a separate offense and is subject to remedies and penalties by the authority having jurisdiction. Appeals per ch. 68, WI Stats. and s. Comm 20.21.

Enforcing Jurisdiction: ☐ Town ☒ Village ☐ City ☐ County **OF:** ☐ State Staff ☐ State Insp Agency#

Bldg Location Muni #: 41 -- 126

Authority By Municipal Ordinance Section::

Inspector's Name: Scott Miller

Violations Explained To: Contractor

Compliance Date: 30 DAYS

Inspector's Address: Village Hall / 7200 N Santa Monica Blvd

Office Hours: 8-930AM/1230-130PM

Telephone No: (414) 351-8800

Orders Referred for Followup Legal Action To:

Date

Noncompliances Verified to Still Exist? (If needed, notate orders above.) ☐ Yes ☐ No

Additional Fees Collected(+)/Refunded(-) By State-Contracted Agency $ Since Original Permit Issuance:

Distribution: ☐ Ply 1 – Contractor ☐ Ply 2 – Inspector/State ☐ Ply 3 - Owner ☐ Ply 4 - File

SBD-6025 (R 06/05)

SEWER AND PLUMBING DEPARTMENT

Permit No. 2658 Application No. 2681 Fox Point, Wis. Sept 11 19 57

Permission is hereby given to do the necessary draining and plumbing work on the premises of H. Friedlen described as follows:

Description	Lot	Blk.	Subd.

being No. 1015 on the South side of E. Quarles Pl.

The above named is permitted to employ J. C. Sebolsky a Licensed Plumber for the purpose of laying a — inch — Sanitary Sewer drain pipe from Main to Curb to Lot line to Premises. Connection to be made in —— feet — of

Or of laying a inch Storm Sewer Drain pipe

Fixtures with drain or water connection

	No.		No.		No.
Bath tubs		Sump Pump		Wash Basins	
Laundry tubs	1	Sinks		Water Closets	
Basement drains		Showers		Hot Water Heater	
Dishwasher	1			Garbage Disposal	1

as per application made subject to the Rules and Regulations of the Village Board and of Ordinance No. 56 of the Village of Fox Point and amendments.

Received for Permit $ Min Fee $3.00 Dollars

Received for Fixtures $ Dollars

Carl H. Plank
PLUMBING INSPECTOR

PERMIT CLERK

E&F
775

INSPECTION APPROVAL

Permit 7907 Date 11/1/71

TO DEPT. OF BUILDING INSPECTION
VILLAGE OF FOX POINT

Please be advised that the undersigned has made a Heating plant Electrical Inspection of the residence of Mr. H. Fuellen located at 1015-E Quarles and hereby approves same.

REMARKS:

Signed Walter J. Kaiser
WALTER J. KAISER
ELECTRICAL INSPECTOR
VILLAGE OF FOX POINT

SEWER AND PLUMBING DEPARTMENT

Permit No. 7889 Application No. 7925 Fox Point, WI 2/15/96 19__

Permission is hereby given to do the necessary plumbing work on the premises of described as follows:

Lot Block Subdivision

Located at 1015 Charles Place

The above named is permitted to employ Don Regner

License No. MPRA 3227 for the purpose of laying a ____ inch ____

Sanitary Building Sewer from Main to Curb to Lot line to Premises. Connection to be made in ____ feet ____ of ____

Laying a ____ inch ____ Building Storm Sewer ____

Fixtures with drain or water connection:

	No.		No.		No.
Hose Bibs		Water Heaters	1	Water Closets	
Bath Tubs		Wash Mach Waste		Showers	
Sump Pumps		Bidets		Floor Drains	
Laundry Trays		Catch Basins		Food Waste Grinders	
Drinking Fountains		Dishwashers		Sprinkling Systems	
Sinks		Wash Basins		Urinals	

as per application made subject to the Rules and Regulations of the Village Board and of Chapter 12 of the Fox Point Village Code.

Building Sewer $ ____ Fixtures $ 40

Building Drain $ ____ Rec'd for Permit $ Receipt #3435

Plumbing Inspector

SEWER AND PLUMBING DEPARTMENT

Permit No. 9500 Application No. 9611 Fox Point, WI 1/4/02 20__

Permission is hereby given to do the necessary plumbing work on the premises of _____ described as follows:

Lot _____ Block _____ Subdivision _____

Located at 1015 E Quarles Place

The above named is permitted to employ Gene Fels

License No. 22395 for the purpose of laying a _____ inch _____

Sanitary Building Sewer from Main to Curb to Lot line to Premises. Connection to be made in _____ feet _____ of _____

Laying a _____ inch _____ Building Storm Sewer _____

Fixtures with drain or water connection:

	No.		No.		No.
Hose Bibs		Water Heaters	1	Water Closets	
Bath Tubs		Wash Mach Waste		Showers	
Sump Pumps		Bidets		Floor Drains	
Laundry Trays		Catch Basins		Food Waste Grinders	
Drinking Fountains		Dishwashers		Sprinkling Systems	
Sinks		Wash Basins		Urinals	

as per application made subject to the Rules and Regulations of the Village Board and of Chapter 12 of the Fox Point Village Code.

Building Sewer $ _____ Fixtures $ 40.00

Building Drain $ _____ Rec'd for Permit $ Receipt # 3168

Plumbing Inspector

SEWER AND PLUMBING DEPARTMENT

Permit No. 12851 Application No. 12897 Fox Point, WI 10/21/13 20___

Permission is hereby given to do the necessary plumbing work on the premises of

described as follows:

Lot Block Subdivision

Located at 1015 E Dunlek Place

The above named is permitted to employ Wayne Overlien

License No. 227764 for the purpose of laying a_____ inch_____.

Sanitary Building Sewer from Main to Curb to Lot line to Premises. Connection to be made in

_____ feet_____ of_____

Laying a_____ inch_____ Building Storm Sewer_____

Fixtures with drain or water connection:

	No.		No.		No.
Hose Bids		Water Heaters		Water Closets	
Bath Tubs		Wash Machine Waste		Showers	
Sump Pumps		Bidets		Floor Drains	
Laundry Trays		Catch Basins		Floor Waste Grinders	
Drinking Fountains		Dishwashers		Sprinkling Systems	
Sinks		Wash Basins		Urinals	
Cap Sewer Lateral					

as per application made subject to the Rules and Regulations of the Village Board and of Chapter 12 of the Fox Point Village Code.

Building Sewer $_____ Fixtures $_____

Building Drain $_____ Rec'd for permit $ 60.00

Receipt #

Plumbing Inspector

45311

SEWER AND PLUMBING DEPARTMENT

Permit No. 13052 Application No. 13099 Fox Point, WI 7/7/15 20___

Permission is hereby given to do the necessary plumbing work on the premises of described as follows:

Lot Block Subdivision

Located at 1015 E. Dunlap Place

The above named is permitted to employ Wayne Dunlap

License No. 22764 for the purpose of laying a 4 inch PVC.

Sanitary Building Sewer from Main to Curb to Lot line to Premises. Connection to be made in

feet of

Laying a [redacted] inch [redacted] Building Storm Sewer

Fixtures with drain or water connection:

	No.		No.		No.
Hose Bids		Water Heaters		Water Closets	
Bath Tubs		Wash Machine Waste		Showers	
Sump Pumps		Bidets		Floor Drains	
Laundry Trays		Catch Basins		Floor Waste Grinders	
Drinking Fountains		Dishwashers		Sprinkling Systems	
Sinks		Wash Basins		Urinals	
1 1/4" Water Curbing					

as per application made subject to the Rules and Regulations of the Village Board and of Chapter 12 of the Fox Point Village Code.

Building Sewer $_____ Fixtures $_____

Building Drain $_____ Rec'd for permit $ 100.00

Receipt # 47785

Plumbing Inspector

SEWER AND PLUMBING DEPARTMENT

Permit No. 13027 Application No. 13050 Fox Point, WI 5/8/14 20___

Permission is hereby given to do the necessary plumbing work on the premises of

_____ described as follows:

Lot Block Subdivision

_____

_____

Located at 1015 E Quadrangle Place

The above named is permitted to employ Christopher Hicks

License No. 220705 for the purpose of laying a_____ inch_____.

Sanitary Building Sewer from Main to Curb to Lot line to Premises. Connection to be made in

_____ feet_____ of_____

Laying a_____ inch_____ Building Storm Sewer_____

Fixtures with drain or water connection:

	No.		No.		No.
Hose Bids	3	Water Heaters	1	Water Closets	7
Bath Tubs	1	Wash Machine Waste	1	Showers	3
Sump Pumps	1	Bidets		Floor Drains	1
Laundry Trays	1	Catch Basins		Floor Waste Grinders	1
Drinking Fountains		Dishwashers	1	Sprinkling Systems	
Sinks	1	Wash Basins	8	Urinals	

as per application made subject to the Rules and Regulations of the Village Board and of Chapter 12 of the Fox Point Village Code.

Building Sewer $_____ Fixtures $_____

Building Drain $_____ Rec'd for permit $ 290.00

Receipt # 46826

_____

Plumbing Inspector

SEWER AND PLUMBING DEPARTMENT

Permit No. 799 Application No. 859 Fox Point, Wis. 11/8 1947

Permission is hereby given to do the necessary draining and plumbing work on the premises of R. E. Bilkie described as follows:

Description	Lot	Blk.	Subd.
Fox Point Sub.	4	1	

being No. 1015 on the S side of E. Quarton Pl

The above named is permitted to employ C. Hafkin a Licensed Plumber for the purpose of laying a 6 inch Clay Sanitary Sewer drain pipe from Main to Curb to Lot line to Premises. Connection to be made in Main 98.6 feet E of E of Lot Main Interceptor

Or of laying a 1 inch E Quarton Pl Storm Sewer Drain pipe

Fixtures with drain or water connection

	No.		No.		No.
Bath tubs	1	Urinals		Wash Basins	2
Laundry tubs	1	Sinks	1	Water Closets	2
Basement drains	1			Hot Water Heater	1

as per application made subject to the Rules and Regulations of the Village Board and of Ordinance No. 56 of the Village of Fox Point and amendments. 8.30

1 6 Trunk.
Hot Water Heater
8 @ .50

Received for Permit $ 1.00 Dollars

Received for Fixtures $ 4.00 Dollars

$ 13.30

O. Blank
PLUMBING INSPECTOR

O. Blank.
PERMIT CLERK

WATER PERMIT

Permit No. 543 Application No. 859 Fox Point, Wis. 11/3 19 47

Permission is hereby given to do the work necessary to supply with water the premises of R. F. Buthke described as follows:

Description	Lot	Blk.
Fox Point Sub.	4	1

being No. 1015 on the S side of E. Quadrant St.

The above named is permitted to employ C. Hakes a Licensed Plumber for the purpose of laying a 1 inch Cu. Tube service pipe from Main to Curb: a ..... inch ..... service pipe from curb to building at 1015 E. Quadrant St.

as per application made subject to the Rules and Regulations of the Village Board and of Ordinance No. 66 of the Village of Fox Point and amendments.

Settling Tank 9/16

Received for Permit $..... Dollars

Received for Fixtures $..... Dollars

Returns must be made on all work done.

V. Plank
WATER INSPECTOR

V. Plank
PERMIT CLERK

Issued 7/17/70

VILLAGE OF FOX POINT
MILWAUKEE COUNTY, WISCONSIN

No. 1589

APPLICATION FOR PERMIT

TO THE INSPECTION DEPARTMENT:

The undersigned hereby applies for a permit for the execution of electrical installation for light, heat or power, as hereinafter prescribed.

1. Location 1015-E- Quarles Place
(Give exact street and number. Do not give corner.)
2. Owner R Bethke
3. Lot 4 Block 1 Subdivision Fox Point Subd.
4. Building or structure
5. Contractor Oliver Brower. License No. 38

	Number	Rate of Fees		Fees
6. Lighting Outlets	50	@	$ .10	5 00
7. Fixtures	7	"	.05	35
8. Range Circuit or Outlet	1	"	1.00	1 00
9. Range Connection		"	1.00	
10. Water Heaters & other Heating Devices	1	1st Kilowatt	" 1.00	1 00
11. Refrigerating Machines		Each Additional Kilowatt	" .10 2.00	
12. Oil Burners and Stokers			" 1.00	
13. Temporary Permits		Inspection per Hour	" 2.00	
14. Motors		H.P.-H.P.-H.P. per H.P.	" .10	
15. Studded Lights including their Individual Outlets			" .05	
16. Rectifiers and Transformers			" 1.00	
Estimated cost $		Total fees		$7 35

Date of inspection { Wiring Will Notify 19 ; Fixtures 19 }

Note: Minimum Fee $1.00

Enclosed please find $

It is hereby agreed between the undersigned person, firm or corporation and the Village of Fox Point that for and in consideration of the premises and of the permit for the execution of the electrical installation, for light, heat or power, as particularly described in this application, the work will be done in accordance with all of the provisions of all ordinances regulating the installation of electrical work, electric wiring and apparatus in the Village of Fox Point and all of the subsequent amendments thereto.

Signed Oliver Brower

Address 3663 E Squire Ave.

VILLAGE OF FOX POINT

MILWAUKEE COUNTY, WISCONSIN

7-18-52

No. 3228 check

APPLICATION FOR PERMIT

TO THE INSPECTION DEPARTMENT:

The undersigned hereby applies for a permit for the execution of electrical installation for light, heat or power, as hereinafter prescribed.

1. Location 1015 E Dawes
(Give exact street and number. Do not give corner.)
2. Owner R Bethke
3. Lot ..... Block ..... Subdivision .....
4. Building or structure Residence
5. Contractor Green Tree Electric Licence No. .....

		Number	Rate of Fees	Fees
6.	Lighting Outlets		@	$ .10
7.	Fixtures		"	.05
8.	Range Circuit or Outlet		"	1.00
9.	Range Connection		"	1.00
10.	Water Heaters & other Heating Devices		1st Kilowatt "	1.00 1.00
11.	Refrigerating Machines		Each Additional Kilowatt "	.10 2.00
12.	Oil Burners and Stokers		"	1.00
13.	Temporary Permits		Inspection per Hour "	2.00
14.	Motors		H.P.-H.P.-H.P. per H.P. "	.10
15.	Studded Lights including their Individual Outlets		"	.05
16.	Rectifiers and Transformers		"	1.00

Estimated cost $ ..... Total fees #1.00

Date of inspection { Wiring July 18 1952 / Fixtures ..... 19 ..... }

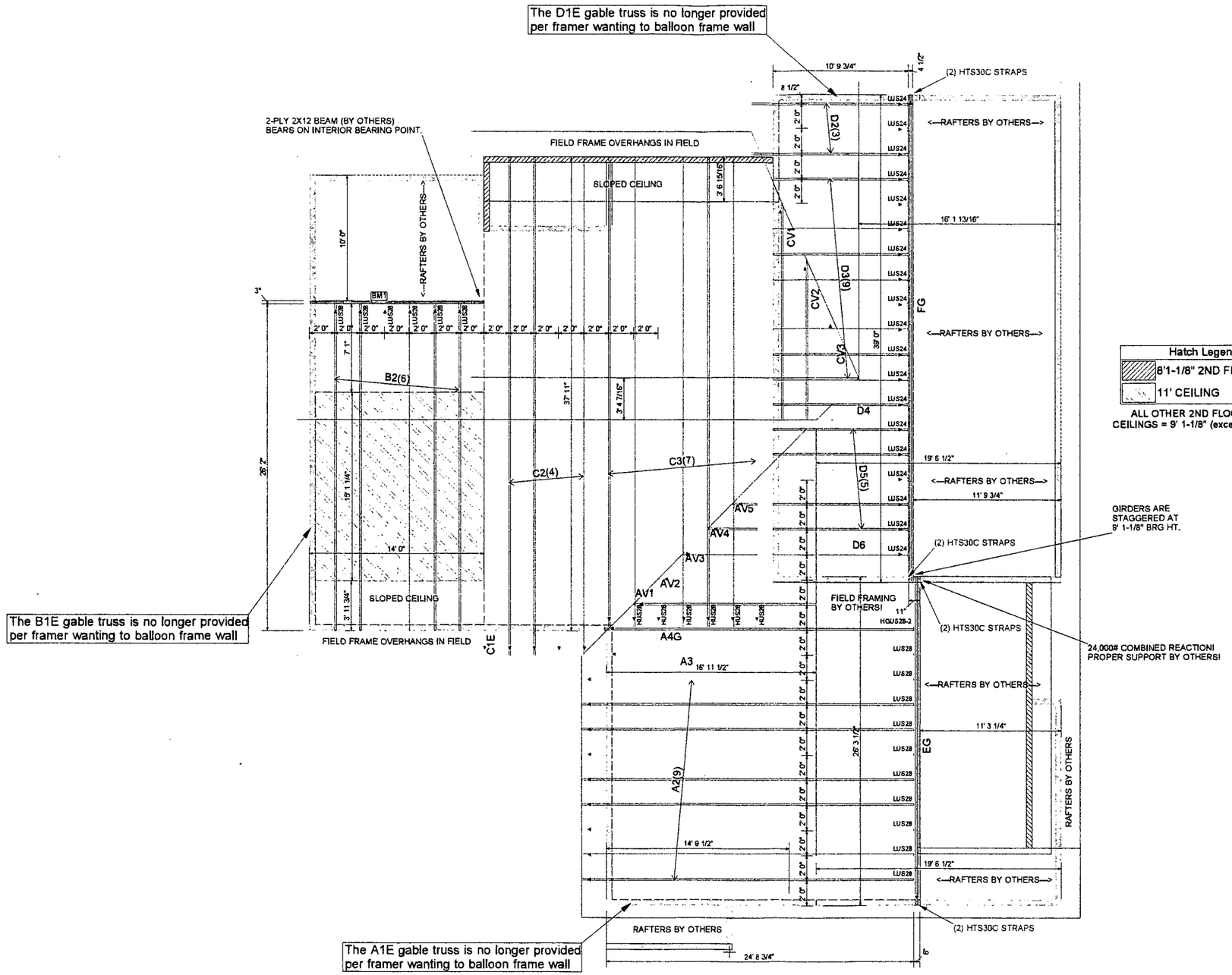
Note: Minimum Fee $1.00

Enclosed please find $ .....

It is hereby agreed between the undersigned person, firm or corporation and the Village of Fox Point that for and in consideration of the premises and of the permit for the execution of the electrical installation, for light, heat or power, as particularly described in this application, the work will be done in accordance with all of the provisions of all ordinances regulating the installation of electrical work, electric wiring and apparatus in the Village of Fox Point and all of the subsequent amendments thereto.

Signed Green Tree Electric

Address 6933-71 Port Washington Road



GENERAL ROOF NOTES:

* The intent of this document is to show truss placement only. Contractor is responsible for verifying all dimensions and wall heights with the blueprint prior to setting the trusses.

* All dimensions are Feet - Inches - Fractions.

* The Left end of truss (as shown on Drawing) is the end of truss shown with a triangle on placement plan.This is also the painted end of the truss.

* Truss over-framing &/or header material by others (unless otherwise noted). Refer to truss reactions.

* -Sheath under all valleys (if applicable). Uniformly distribute all over-framing to trusses below as to not impose point-loads.

* See drawings for multi-ply girder connections and permanent bracing locations (if applicable)

* See included documents for important bracing information. Refer to job package for additional info & hanger information.

* Cutting or altering trusses without engineer approval voids warranty. Contact AHS prior to cutting any trusses.

Hatch Legend
8'1-1/8" 2ND FLR WALLS
11' CEILING

ALL OTHER 2ND FLOOR WALLS & CEILINGS = 9' 1-1/8" (except where sloped).

Wisconsin BUILDING SUPPLY

Job Notes:

Contractor:
Latest Plan Date: 8/23/13
Latest Layout Date: 10/16//14

HANGER SCHEDULE:
(19) LUS24
(16) LUS28
(6) HUS28
(1) HGUS28-2

UPLIFT CLIP/STRAP SCHEDULE:
(55) H2.5T
(8) HTS30C uplift twist straps.

ACCURATE HOUSING SYSTEMS INC
2624 Corporate Circle, East Troy, WI 53120
P: 262-642-3800 F: 262-642-2716
www.accuratehousing.com

Work Order #: R-5902-13
Client: Wisconsin Building Supply - Su

Job Name: Stratton Residence
Model:
Elevation:
Lot/Sub:

Truss Tech:
Justin Nagl Ext. 7013

Industry Resources: Connectors: strongtie.com/products/Alpha_list.html Engineering: mitek-us.com *("Engineering" pulldown)* Bracing, etc: sbcindustry.com *("Info for Builders" pulldown)*

Second Floor

I Joist

Qty	Label	Description	Width	Depth	Length
8	J1	WSI 40	2.313	11.875	4
1	J2	WSI 40	2.313	11.875	5
10	J4	WSI 40	2.313	11.875	7
1	J5	WSI 40	2.313	11.875	8
25	J6	WSI 40	2.313	11.875	9
4	J9	WSI 40	2.313	11.875	17
7	J11	WSI 40	2.313	11.875	18
9	J10	WSI 40	2.313	11.875	19
4	J12	WSI 40	2.313	11.875	20
8	J15	WSI 40	2.313	11.875	35
3	J13	WSI 40	2.313	11.875	37
6	J16	WSI 40	2.313	11.875	38
25	J18	WSI 90	3.5	11.875	23
4	J19	WSI 90	3.5	11.875	24

LVL

Qty	Label	Description	Width	Depth	Length
2	B1	2.0E WS-LAM LVL	1.75	9.5	2
2	B2	2.0E WS-LAM LVL	1.75	9.5	11
2	G3	2.0E WS-LAM LVL	1.75	11.875	5
2	G4	2.0E WS-LAM LVL	1.75	11.875	6
2	G5	2.0E WS-LAM LVL	1.75	11.875	7
4	G6	2.0E WS-LAM LVL	1.75	11.875	10
4	G7	2.0E WS-LAM LVL	1.75	11.875	11
2	G13	2.0E WS-LAM LVL	1.75	11.875	12
3	G8	2.0E WS-LAM LVL	1.75	11.875	14
4	G11	2.0E WS-LAM LVL	1.75	11.875	16
6	G9	2.0E WS-LAM LVL	1.75	11.875	18
4	G10	2.0E WS-LAM LVL	1.75	11.875	19
2	G14	2.0E WS-LAM LVL	1.75	11.875	20

Rim Board

Qty	Label	Description	Thickne...	Depth	Length
20	R1	APA EWS Rim Board Plus 1.125 X 11.875	1.125	11.875	12

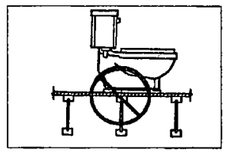
Blocking

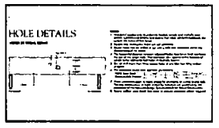
Qty	Label	Description	Width	Depth	Length
1	Blk1	WSI 40	2.313	11.875	25-0-0

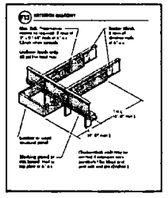
Hanger

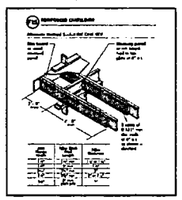
Qty	Label	Description
9	H4	HU410 (Min)
1	H5	HU412 (Max)
94	H3	IUS2.37/11.88 (Min)

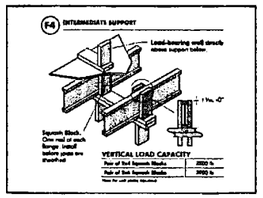
- CAT walk
- Drawing Header
-

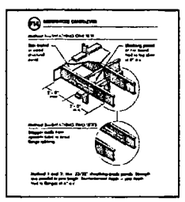












Wisconsin Building Supply
W232 N 5700 HWY 74
Sussex, Wi
US
53089

Client: Butler chase

Project: Stratton Second Floor

Created: 10/21/2013 12:00:00 AM

Designer: LNP

Sales Rep: Bob Yurk

Second Floor	
Design Method	ASD
Building Code	IRC/IRC
Floor	
Loads	
Live	40
Dead	12
Deflection Joist	
LL Span /	480
TL Span /	240
LL Cant /	180
TL Cant /	180
Deflection Girder	
LL Span /	360
TL Span /	240
LL Cant /	180
TL Cant /	180
Decking	
Deck	OSB
Fastener	Nailed & Glued
Thickness	3/4"

Wisconsin BUILDING SUPPLY ESTIMATING

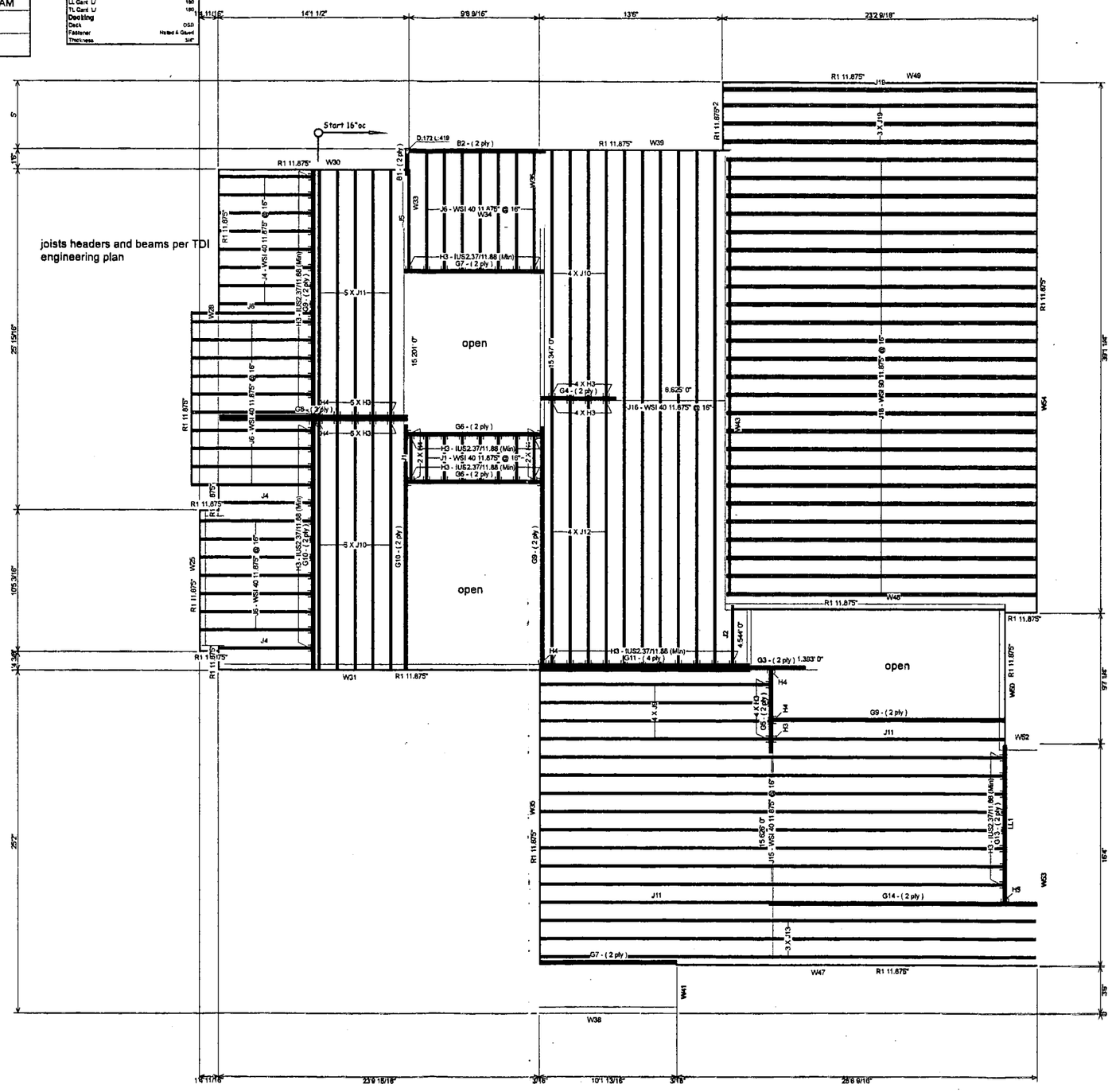
Lu Ann Price
W232 N 5700 HWY 74
Sussex,WI 53089
262-246-0611

Legend

PS	Point Load Support
	Load from Above
	Wall
	APA EWS Rim Board Plus 1.125 X 11.875
	WSI 40 11.875
	WSI 90 11.875
	2.0E WS-LAM LVL 1.75 X 9.5 (Dropped)
	2.0E WS-LAM LVL 1.75 X 11.875

1. Framing placement and all dimensions to be verified by Customer.
2. Ladder Frame perpendicular to parallel walls to 1st joist 32"oc typical.
3. For bottom chord bracing of I-Joist refer to beam calc notes
4. Additional added Joist for Partition Wall Above

Scale 1/4 inch : 1 ft.

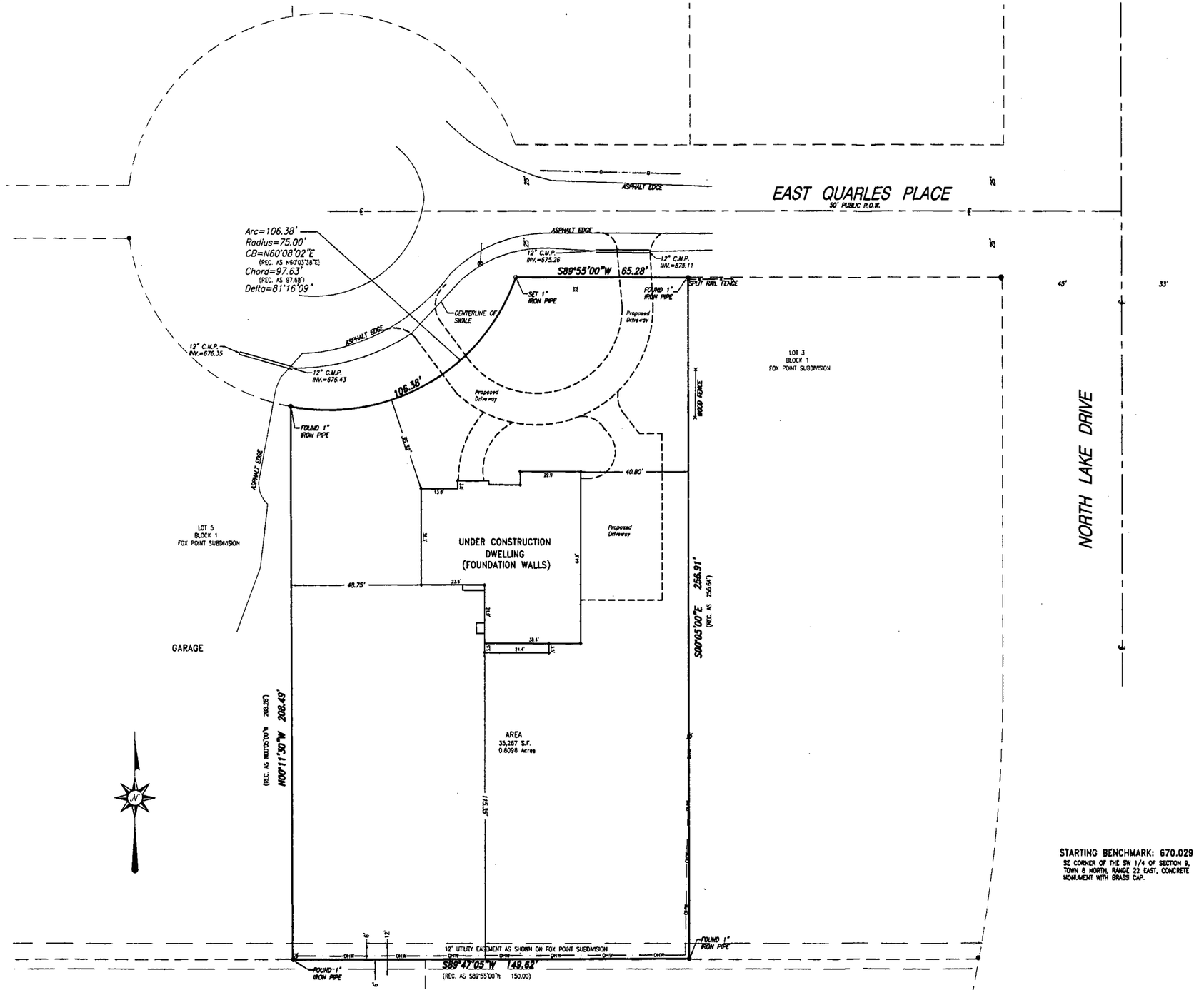


PLAT OF SURVEY

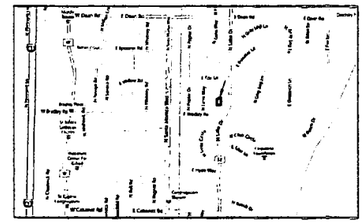
CLIENT
Frederick Stratton

SITE ADDRESS
1015 East Quarles Place, Village of Fox Point, Milwaukee County, Wisconsin.

LEGAL DESCRIPTION
Lot 4, Block 1, Fox Point Subdivision, being a part of the Southwest 1/4 of Section 9, Town 8 North, Range 22 East in the Village of Fox Point, Milwaukee County, Wisconsin.



VICINITY MAP



LEGEND

- INDICATES FOUND 1" IRON PIPE
- INDICATES SET 1" IRON PIPE
- INDICATES FOUND CHISELED CROSS
- SANITARY MANHOLE
- SANITARY CLEANOUT OR VENT
- M.I.S. MANHOLE
- UNKNOWN MANHOLE
- STORM MANHOLE
- INLET (ROUND)
- INLET (SQUARE)
- STORM SEWER END SECTION
- GAS VALVE
- GAS METER
- WATER VALVE
- HYDRANT
- WATER MANHOLE
- WATER SERVICE CURB STOP
- WELL HEAD
- STAND PIPE
- WALL INDICATOR VALVE
- POST INDICATOR VALVE
- LIGHT POLE
- SPOT/YARD LIGHT
- UTILITY POLE
- GUY POLE
- GUY WIRE
- ELECTRIC MANHOLE
- ELECTRIC PEDESTAL
- ELECTRIC METER
- TELEPHONE MANHOLE
- TELEPHONE PEDESTAL
- CABLE PEDESTAL
- CONTROL BOX
- FIBER OPTIC SIGN
- TRAFFIC LIGHT
- COMMUNICATION MANHOLE
- BOLLARD
- SOIL BORING/MONITORING WELL
- WATER SURFACE
- WETLANDS FLAG
- MARSH
- FLAGPOLE
- PARKING METER
- SIGN
- MAILBOX
- RAILROAD CROSSING SIGNAL
- HANDICAP SPACE
- CONIFEROUS TREE
- DECIDUOUS TREE
- SANITARY SEWER
- STORM SEWER
- WATERLINE
- MARKED GAS MAIN
- MARKED ELECTRIC
- OVERHEAD WIRES
- MARKED TELEPHONE
- MARKED CABLE TV LINE
- MARKED FIBER OPTIC
- FENCE

GRAPHIC SCALE
20 0 10 20 40 80
(IN FEET)
1 Inch = 20 ft.

STARTING BENCHMARK: 670.029
SE CORNER OF THE SW 1/4 OF SECTION 9, TOWN 8 NORTH, RANGE 22 EAST, CONCRETE MONUMENT WITH BRASS CAP.

I Certify that I have surveyed the above described property (Property), and the above map is correct to the best of my professional knowledge and belief and shows the size and location of the Property, its exterior boundaries, the location and dimensions of all visible structures thereon, boundary fences, apparent easements and roadways and visible encroachments, if any. This survey is made for the exclusive use of the present owners of the Property, and also those who purchase, mortgage, or guarantee the title thereto, within one (1) year from date hereof.

Date: November 21, 2013

Donald C. Chaput
Registered Land Surveyor
Registration Number S-1316

DONALD C. CHAPUT S-1316 MILWAUKEE WI — WISCONSIN — LAND SURVEYOR

CHAPUT LAND SURVEYS LLC
234 W. FLORIDA STREET
MILWAUKEE, WI 53204
414-224-8068
www.chaputlandsurveys.com

Drawing No. 1477-plS/tjn

CHAPUT LAND SURVEYS LLC

EAST QUARLES PLACE
50' PUBLIC R.O.W.

NORTH LAKE DRIVE

ASPHALT EDGE
676
677
678
675
671
25'
45'
33'
SET 1" IRON PIPE
SPLIT RAIL FENCE
FOUND 1" IRON PIPE
24" TREE
18" TREE
12" TREE
15" TREE
10" TREE
35.27'
40.68'
31.42'
19.80'
DWELLING
GARAGE
20.23'
36.33'
256.91'
208.49'
N00°11'30"W
Existing Building footprint 2663 sf
Open Area - 32604 Sf
8.25% coverage
New Building footprint 3277 sf
Open Area - 31990 Sf
10.15% coverage
AREA
35,267 S.F.
0.8096 Acres
12' UTILITY EASEMENT AS SHOWN ON FOX POINT SUBDIVISION
S89°47'05"W 149.62'
(REC. AS S89°55'00"W 150.00)
FOUND 1" IRON PIPE
6'
12'

First Floor

I Joist

Qty	Label	Description	Width	Depth	Length
5	J2	WSI 40	2.313	11.875	5
3	J3	WSI 40	2.313	11.875	6
2	J7	WSI 40	2.313	11.875	10
1	J8	WSI 40	2.313	11.875	11
7	J20	WSI 40	2.313	11.875	33
4	J14	WSI 40	2.313	11.875	35
23	J15	WSI 40	2.313	11.875	37
10	J17	WSI 40	2.313	11.875	39

LVL

Qty	Label	Description	Width	Depth	Length
2	G1	2.0E WS-LAM LVL	1.75	11.875	5
1	G2	2.0E WS-LAM LVL	1.75	11.875	18
4	G12	2.0E WS-LAM LVL	1.75	11.875	33

Rim Board

Qty	Label	Description	Thickne...	Depth	Length
22	R1	APA EWS Rim Board Plus 1.125 X 11.875	1.125	11.875	12

Blocking

Qty	Label	Description	Width	Depth	Length
1	Blk2	1.5E WS-LAM LVL	1.25	11.875	43-0-0
1	Blk1	WSI 40	2.313	11.875	34-0-0

Hanger

Qty	Label	Description
1	H2	IUS1.81/11.88 (Min)
6	H3	IUS2.37/11.88 (Min)
1	H1	Unknown Hanger

Wisconsin Building Supply
W232 N 5700 HWY 74
Sussex, Wi
US
53089

Client: Butler chase
Project: Stratton — First Floor
Created: 10/21/2013 12:00:00 AM
Designer: LNP
Sales Rep: Bob Yurk

First Floor

Design Method	ASD
Building Code	IBC/IRC
Floor Loads	
Live	40
Dead	12
Deflection Joist	
LL Span L/	480
TL Span L/	240
LL Cant L/	180
TL Cant L/	180
Deflection Girder	
LL Span L/	360
TL Span L/	240
LL Cant L/	180
TL Cant L/	180
Decking	
Deck	OSB
Fastener	Nailed & Glued
Thickness	3/4"

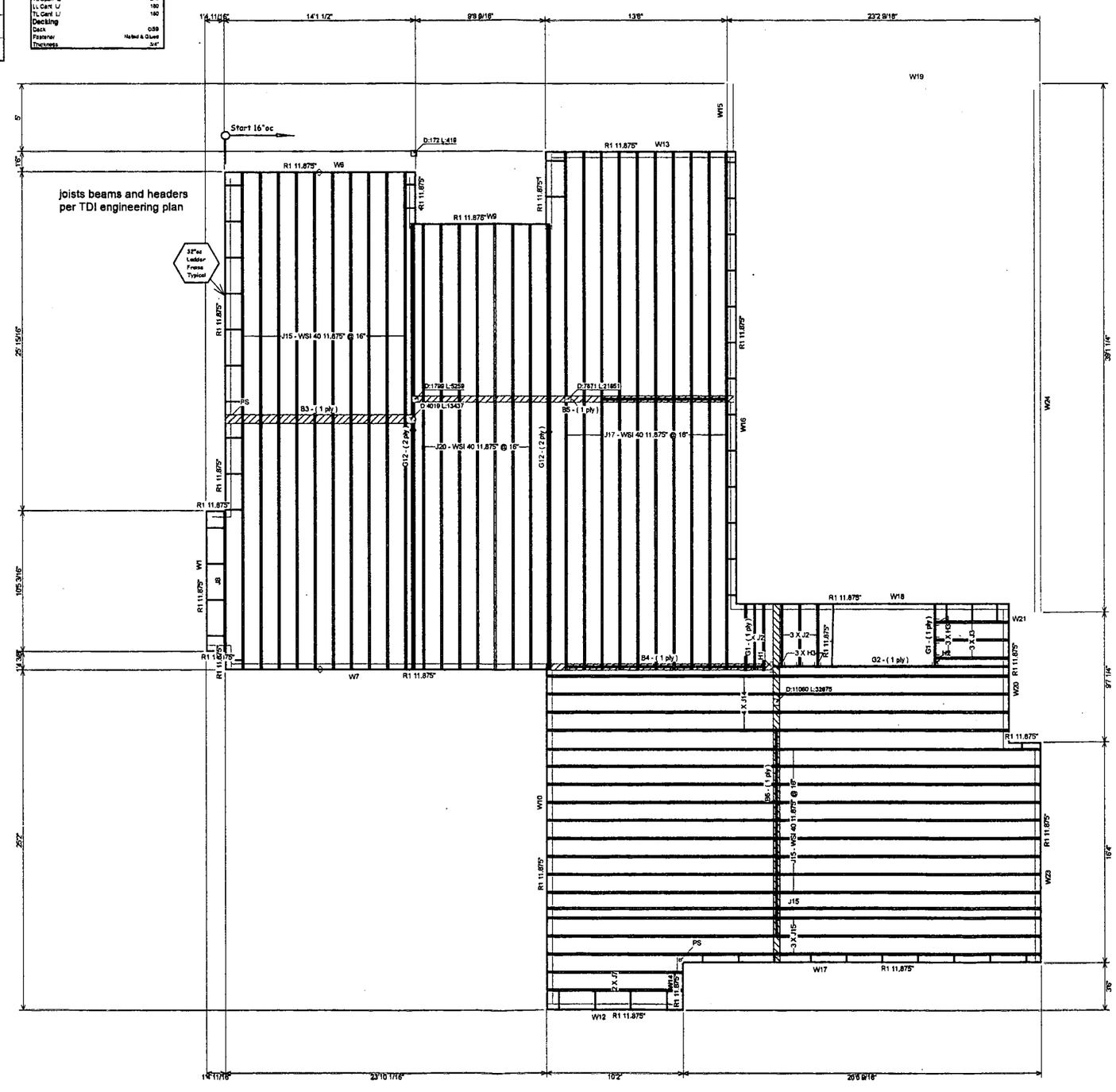
Wisconsin BUILDING SUPPLY ESTIMATING

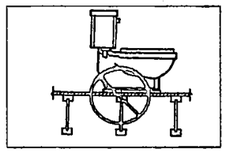
Lu Ann Price
W232 N 5700 HWY 74
Sussex,WI 53089
262-246-0611

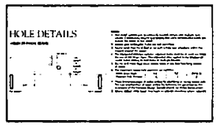
Legend
- Point Load Support
- Load from Above
- Wall
- APA EWS Rim Board Plus 1.125 X 11.875
- WSI 40 11.875
- 2.0E WS-LAM LVL 1.75 X 11.875
- Beam By Others 6 X 18 (Dropped)
- Beam By Others 6 X 18 (Dropped)

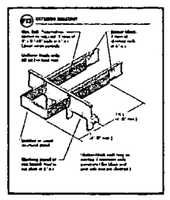
1. Framing placement and all dimensions to be verified by Customer.
2. Ledger Frame perpendicular to parallel walls to 1st joist 32"oc typical.
3. For bottom chord bracing of I-Joist refer to beam calc notes
4. Additional added Joist for Partition Wall Above

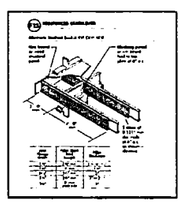
Scale 1/4 inch : 1 ft.

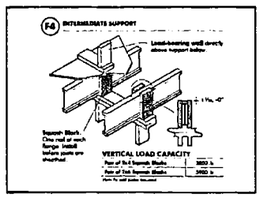


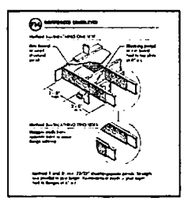












second Floor
I Joist

Qty	Label	Description	Width	Depth	Length
7	J1	WSI 40	2.313	11.875	4-0-0
1	J2	WSI 40	2.313	11.875	5-0-0
20	J3	WSI 40	2.313	11.875	7-0-0
17	J4	WSI 40	2.313	11.875	9-0-0
4	J5	WSI 40	2.313	11.875	16-0-0
1	J6	WSI 40	2.313	11.875	18-0-0
15	J6	WSI 40	2.313	11.875	19-0-0
4	J7	WSI 40	2.313	11.875	20-0-0
2	J12	WSI 40	2.313	11.875	22-0-0
7	J13	WSI 40	2.313	11.875	35-0-0
3	J14	WSI 40	2.313	11.875	37-0-0
8	J9	WSI 40	2.313	11.875	38-0-0
25	J10	WSI 90	3.5	11.875	23-0-0
3	J11	WSI 90	3.5	11.875	24-0-0

LVL

Qty	Label	Description	Width	Depth	Length
2	B1	2.0E WS-LAM LVL	1.75	11.875	2-0-0
3	G6	2.0E WS-LAM LVL	1.75	11.875	6-0-0
2	B2	2.0E WS-LAM LVL	1.75	11.875	11-0-0
12	G8	2.0E WS-LAM LVL	1.75	11.875	11-0-0
3	G6	2.0E WS-LAM LVL	1.75	11.875	15-0-0
4	G7	2.0E WS-LAM LVL	1.75	11.875	16-0-0
2	G3	2.0E WS-LAM LVL	1.75	11.875	18-0-0
6	G4	2.0E WS-LAM LVL	1.75	11.875	19-0-0
2	G1	2.0E WS-LAM LVL	1.75	11.875	20-0-0
3	G2	2.0E WS-LAM LVL	1.75	11.875	37-0-0

Rim Board

Qty	Label	Description	Thickness	Depth	Length
27	R1	WS LVL 1.5E 1.25 X 11.875	1.25	11.875	12

Hanger

Qty	Label	Description
1	H2	HGUS410
1	H1	HGUS7.25/12
9	H4	HU410 (Max)
94	H3	IUS2.37/11.88 (Max)

Wisconsin Building Supply	
W232 N 5700 HWY 74	
Sussex, WI	
US	
53089	
Client	
Butler chase	
Project	
Stratton	
Created	
October 16, 2014	
Sales Rep	
Bob Yurk	
Designer	
Lnp	

second Floor	
Design Method	ASD
Building Code	IBC/IRC
Floor	
Loads	
Live	40
Dead	12
Deflection Joist	
LL Span L/	480
TL Span L/	240
LL Cant L/	180
TL Cant L/	180
Deflection Girder	
LL Span L/	360
TL Span L/	240
LL Cant L/	180
TL Cant L/	180
Decking	
Deck	OSB
Fastener	Nailed & Glued
Thickness	3/4"

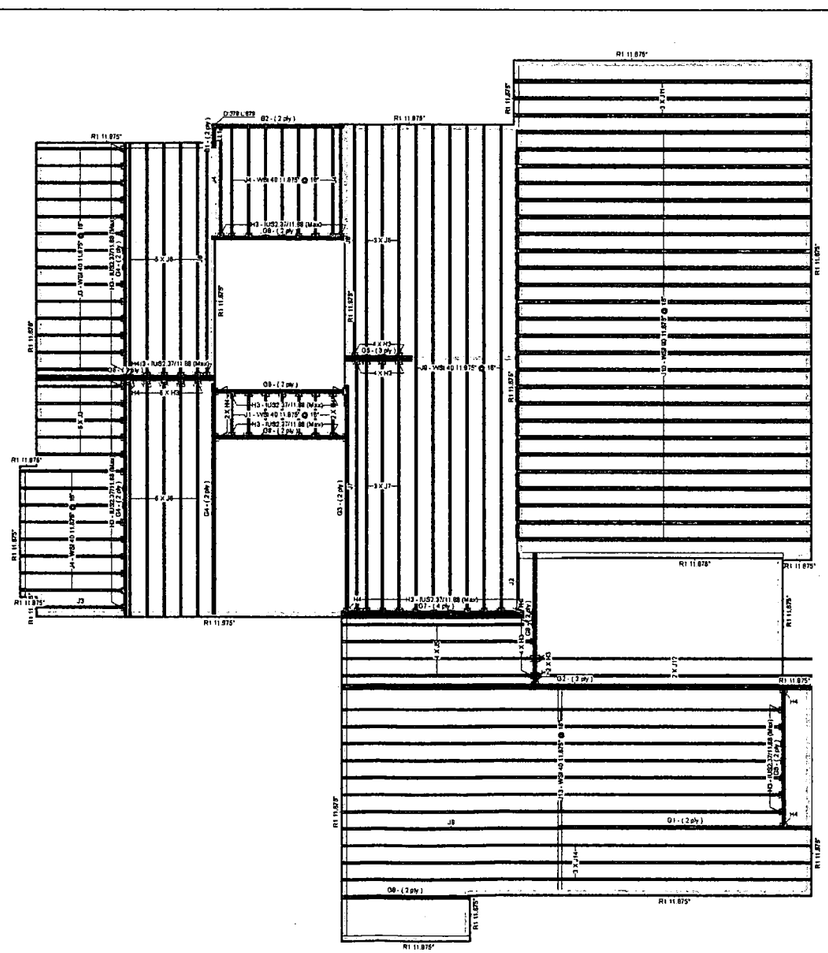
Wisconsin BUILDING SUPPLY ESTIMATING

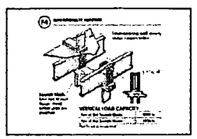
La Ann Price
W232 N 5700 HWY 74
Sussex,WI 53089
262-246-0511

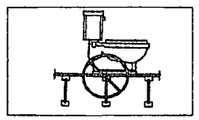
1. Framing placement and all dimensions to be verified by Customer
2. Ladder Frame perpendicular to parallel walls to 1st joist 32"oc typical.
3. For bottom chord bracing of I-Joist refer to beam calc notes
4. Additional added Joist for Partition Wall Above

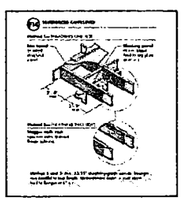
second Floor

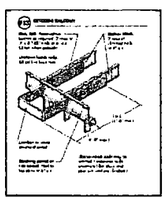
Scale 3/16 Inch : 1 ft

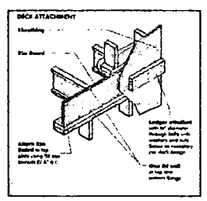


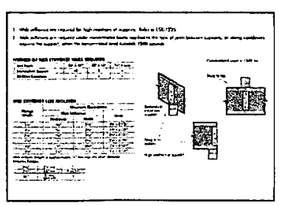


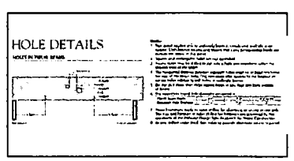












BCSI-B1 SUMMARY SHEET - GUIDE FOR HANDLING, INSTALLING, RESTRAINING AND BRACING OF TRUSSES

Spans over 60′ may require complex permanent bracing. Please always consult a Registered Design Professional.

B1 B1

WARNING!

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to **BCSI – Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses*** for more detailed information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the **BCSI-B3*** for more information. All other permanent bracing design is the responsibility of the building designer.

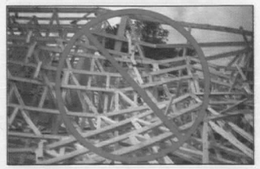
NOTAS GENERALES

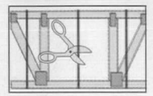
*Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arriostre diagonal temporales. Use las recomendaciones de manejo, instalación, restricción y arriostre temporal de los trusses. Vea el folleto **BCSI – Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arriostre de los Trusses de Madera Conectados con Placas de Metal**** para información más detallada.*

*Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen **BCSI-B3**** para más información. El resto de los diseños de arriostres permanentes son la responsabilidad del diseñador del edificio.*

WARNING The consequences of improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious personal injury or death.

¡ADVERTENCIA! El resultado de un manejo, levantamiento, instalación, restricción y arrisotre incorrecto puede ser la caída de la estructura o aún peor, heridos o muertos.



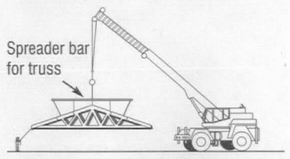


CAUTION Exercise care when removing banding and handling trusses to avoid damaging trusses and prevent injury. Wear personal protective equipment for the eyes, feet, hands and head when working with trusses.

¡CAUTELA! Utilice cautela al quitar las ataduras o los pedazos de metal de sujetar para evitar daño a los trusses y prevenir herida personal. Lleve el equipo protectivo personal para ojos, pies, manos y cabeza cuando trabaja con trusses.

CAUTION Use special care in windy weather or near power lines and airports.

¡CAUTELA! Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.

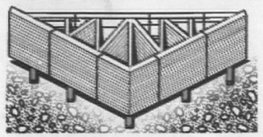


HANDLING — *MANEJO*

NOTICE Avoid lateral bending.
Evite la flexión lateral.

NOTICE The contractor is responsible for properly receiving, unloading and storing the trusses at the jobsite. Unload trusses to smooth surface to prevent damage.

El contratista tiene la responsabilidad de recibir, descargar y almacenar adecuadamente los trusses en la obra. Descargue los trusses en la tierra liso para prevenir el daño.



☑ Trusses may be unloaded directly on the ground at the time of delivery or stored temporarily in contact with the ground after delivery. If trusses are to be stored for more than one week, place blocking of sufficient height beneath the stack of trusses at 8′ (2.4 m) to 10′ (3 m) on-center (o.c.).

Los trusses pueden ser descargados directamente en el suelo en aquel momento de entrega o almacenados temporalmente en contacto con el suelo después de entrega. Si los trusses estarán guardados para más de una semana, ponga bloqueando de altura suficiente detrás de la pila de los trusses a 8 hasta 10 pies en centro (o.c.).

☑ For trusses stored for more than one week, cover bundles to protect from the environment.

Para trusses guardados por más de una semana, cubra los paquetes para protegerlos del ambiente.

Refer to **BCSI*** for more detailed information pertaining to handling and jobsite storage of trusses.

*Vea el folleto **BCSI**** para información más detallada sobre el manejo y almacenado de los trusses en área de trabajo.

☑ Use proper rigging and hoisting equipment.

Use equipo apropiado para levantar e improvisar.

⊘ **DO NOT** store unbraced bundles upright.

***NO** almacene verticalmente los trusses sueltos.*

⊘ **DO NOT** store on uneven ground.

***NO** almacene en tierra desigual.*

HOISTING AND PLACEMENT OF TRUSS BUNDLES
RECOMENDACIONES PARA LEVANTAR PAQUETES DE TRUSSES

⊘ **DON'T** overload the crane.
***NO** sobrecargue la grúa.*

⊘ **NEVER** use banding to lift a bundle.
***NUNCA** use las ataduras para levantar un paquete.*

☑ A single lift point may be used for bundles of top chord pitch trusses up to 45′ (13.7 m) and parallel chord trusses up to 30′ (9.1 m). Use at least two lift points for bundles of top chord pitch trusses up to 60′ (18.3 m) and parallel chord trusses up to 45′ (13.7 m). Use at least three lift points for bundles of top chord pitch trusses >60′ (18.3m) and parallel chord trusses >45′ (13.7 m).

Puede usar un solo lugar de levantar para paquetes de trusses de la cuerda superior hasta 45′ y trusses de cuerdas paralelas de 30′ o menos. Use por lo menos dos puntos de levantar con grupos de trusses de cuerda superior inclinada hasta 60′ y trusses de cuerdas paralelas hasta 45′. Use por lo menos dos puntos de levantar con grupos de trusses de cuerda superior inclinada mas de 60′ y trusses de cuerdas paralelas mas de 45′.

WARNING Do not over load supporting structure with truss bundle.

¡ADVERTENCIA! No sobrecargue la estructura apoyada con el paquete de trusses.

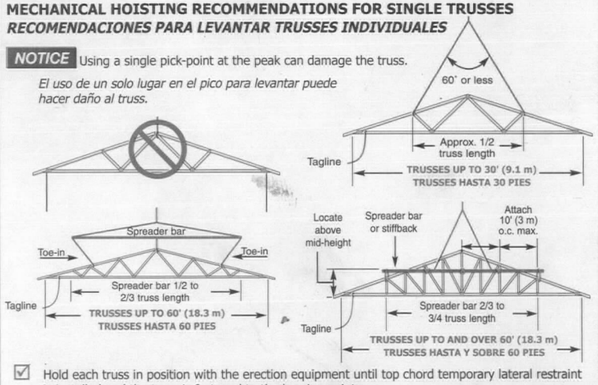
☑ Place truss bundles in stable position.

Puse paquetes de trusses en una posición estable.

MECHANICAL HOISTING RECOMMENDATIONS FOR SINGLE TRUSSES
RECOMENDACIONES PARA LEVANTAR TRUSSES INDIVIDUALES

NOTICE Using a single pick-point at the peak can damage the truss.

El uso de un solo lugar en el pico para levantar puede hacer daño al truss.



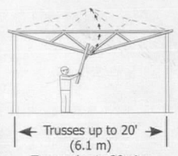
☑ Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points.

Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalado y el truss está asegurado en los soportes.

INSTALLATION OF SINGLE TRUSSES BY HAND
RECOMMENDACCIONES DE LEVANTAMIENTO DE TRUSSES INDIVIDUALES POR LA MANO

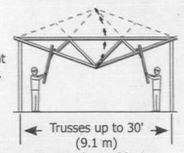
☑ Trusses 20′ (6.1 m) or less, support near peak.

Soporte cerca al pico los trusses de 20 pies o menos.



☑ Trusses 30′ (9.1 m) or less, support at quarter points.

Soporte de los cuartos de tramo los trusses de 30 pies o menos.



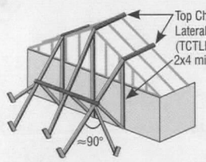
TEMPORARY RESTRAINT & BRACING
RESTRICCIÓN Y ARRIOSTRE TEMPORAL

NOTICE Refer to **BCSI-B2*** for more information.

*Vea el resumen **BCSI-B2**** para más información.*

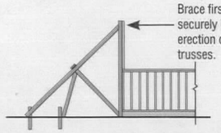
☑ Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).

Coloque los arriostres de tierra para el primer truss directamente en línea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la tabla en la próxima columna).



⊘ **DO NOT** walk on unbraced trusses.

***NO** camine en trusses sueltos.*



STEPS TO SETTING TRUSSES
LAS MEDIDAS DE LA INSTALACIÓN DE LOS TRUSSES

☑ **1)** Install ground bracing. **2)** Set first truss and attach securely to ground bracing. **3)** Set next 4 trusses with short member temporary lateral restraint (see below). **4)** Install top chord diagonal bracing (see below). **5)** Install web member plane diagonal bracing to stabilize the first five trusses (see below). **6)** Install bottom chord temporary lateral restraint and diagonal bracing (see below). **7)** Repeat process with groups of four trusses until all trusses are set.

***1)** Instale los arriostres de tierra. **2)** Instale el primero truss y ate seguramente al arriostre de tierra. **3)** Instale los próximos 4 trusses con restricción lateral temporal de miembro corto (vea abajo). **4)** Instale el arriostre diagonal de la cuerda superior (vea abajo). **5)** Instale arriostre diagonal para los planos de los miembros secundarios para estabilice los primeros cinco trusses (vea abajo). **6)** Instale la restricción lateral temporal y arriostre diagonal para la cuerda inferior (vea abajo). **7)** Repita éste procedimiento en grupos de cuatro trusses hasta que todos los trusses estén instalados.*

NOTICE Refer to **BCSI-B2*** for more information.
*Vea el resùmen **BCSI-B2**** para más información.*

RESTRAINT/BRACING FOR ALL PLANES OF TRUSSES
RESTRICCIÓN/ARRIOSTRE PARA TODOS PLANOS DE TRUSSES

☑ This restraint and bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses (PCTs). See top of next column for temporary restraint and bracing of PCTs.

Este método de restricción y arriostre es para todo trusses excepto trusses de cuerdas paralelas (PCTs) 3x2 y 4x2. Vea la parte superior de la columna para la restricción y arriostre temporal de PCTs.

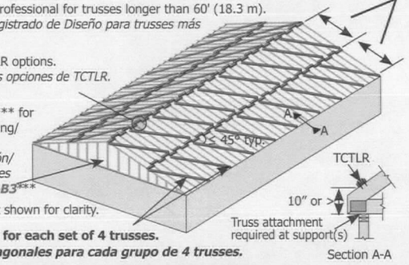
1) TOP CHORD — *CUERDA SUPERIOR*

Truss Span *Longitud de Tramo*	Top Chord Temporary Lateral Restraint (TCTLR) Spacing *Espaciamiento del Arriostre Temporal de la Cuerda Superior*
Up to 30′ (9.1 m)	10′ (3 m) o.c. max.
30′ (9.1 m) – 45′ (13.7 m)	8′ (2.4 m) o.c. max.
45′ (13.7 m) – 60′ (18.3 m)	6′ (1.8 m) o.c. max.
60′ (18.3 m) – 80′ (24.4 m)*	4′ (1.2 m) o.c. max.

*Consult a Registered Design Professional for trusses longer than 60′ (18.3 m).
**Consulte a un Professional Registrado de Diseño para trusses más de 60 pies.*

☑ See **BCSI-B2*** for TCTLR options.
*Vea el **BCSI-B2**** para las opciones de TCTLR.*

NOTICE Refer to **BCSI-B3*** for Gable End Frame restraint/bracing/reinforcement information.
*Para información sobre restricción/arriostre/refuerzo para Armazones Hastiales vea el resumen **BCSI-B3****



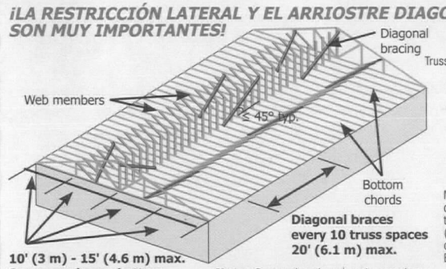
Note: Ground bracing not shown for clarity.

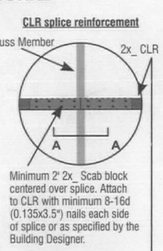
☑ **Repeat diagonal braces for each set of 4 trusses.**
Repita los arrisotres diagonales para cada grupo de 4 trusses.

2) WEB MEMBER PLANE — *PLANO DE LOS MIEMBROS SECUNDARIOS*

NOTICE **LATERAL RESTRAINT & DIAGONAL BRACING ARE VERY IMPORTANT**

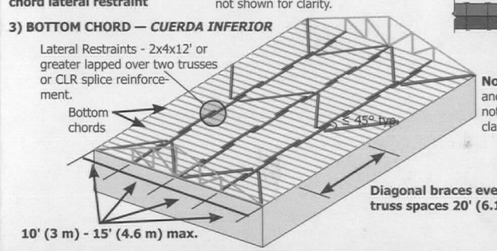
¡LA RESTRICCIÓN LATERAL Y EL ARRIOSTRE DIAGONAL SON MUY IMPORTANTES!





Note: Some chord and web members not shown for clarity.

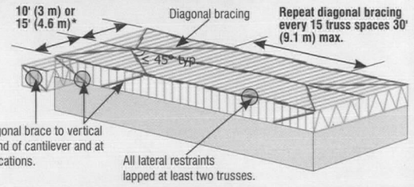
3) BOTTOM CHORD — *CUERDA INFERIOR*



Note: Some chord and web members not shown for clarity.

RESTRAINT & BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES
RESTRICCIÓN Y ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3X2 Y 4X2

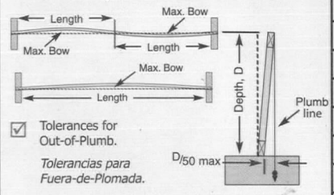
NOTICE Refer to **BCSI-B7*** for more information.
Vea el resumen **BCSI-B7*** para más información.



***Top chord temporary lateral restraint spacing shall be 10′ (3 m) o.c. max. for 3x2 chords and 15′ (4.6 m) o.c. for 4x2 chords.**

INSTALLING — *INSTALACIÓN*

☑ Tolerances for Out-of-Plane.
Tolerancias para Fuera-de-Plano.



☑ Tolerances for Out-of-Plumb.
Tolerancias para Fuera-de-Plomada.

Out-of-Plumb	
D/50	D (ft.)
1/4" (6 mm)	1′ (0.3 m)
1/2" (13 mm)	2′ (0.6 m)
3/4" (19 mm)	3′ (0.9 m)
1" (25 mm)	4′ (1.2 m)
1-1/4" (32 mm)	5′ (1.5 m)
1-1/2" (38 mm)	6′ (1.8 m)
1-3/4" (45 mm)	7′ (2.1 m)
2" (51 mm)	≥8′ (≥2.4 m)

Out-of-Plane	
Max. Bow	Truss Length
3/4" (19 mm)	12.5′ (3.8 m)
7/8" (22 mm)	14.6′ (4.5 m)
1" (25 mm)	16.7′ (5.1 m)
1-1/8" (29 mm)	18.8′ (5.7 m)
1-1/4" (32 mm)	20.8′ (6.3 m)
1-3/8" (35 mm)	22.9′ (7.0 m)
1-1/2" (38 mm)	25.0′ (7.6 m)
1-3/4" (45 mm)	29.2′ (8.9 m)
2" (51 mm)	≥33.3′ (10.1 m)

CONSTRUCTION LOADING
CARGA DE CONSTRUCCIÓN

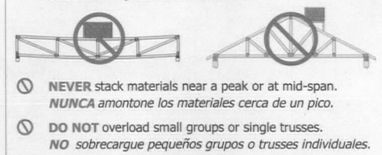
⊘ **DO NOT** proceed with construction until all lateral restraint and bracing is securely and properly in place.

***NO** proceda con la construcción hasta que todas las restricciones laterales y los arriostres estén colocados en forma apropiada y segura.*

⊘ **DO NOT** exceed maximum stack heights. Refer to **BCSI-B4***** for more information.

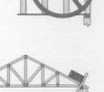
NO** exceda las alturas máximas de montón. Vea el resumen **BCSI-B4* para más información.*

Maximum Stack Height for Material on Trusses	
Material	Height
Gypsum Board	12" (305 mm)
Plywood or OSB	16" (406 mm)
Asphalt Shingles	2 bundles
Concrete Block	8" (203 mm)
Clay Tile	3-4 tiles high



⊘ **NEVER** stack materials near a peak or at mid-span.
***NUNCA** amontone los materiales cerca de un pico.*

⊘ **DO NOT** overload small groups or single trusses.
***NO** sobrecargue pequeños grupos o trusses individuales.*



☑ Place loads over as many trusses as possible.
Coloque las cargas sobre tantos trusses como sea posible.

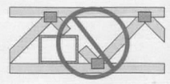
☑ Position loads over load bearing walls.
Coloque las cargas sobre las paredes soportantes.

ALTERATIONS — ALTERACIONES

NOTICE Refer to **BCSI-B5.***
*Vea el resumen **BCSI-B5.***

⊘ **DO NOT** cut, alter, or drill any structural member of a truss unless specifically permitted by the truss design drawing.

***NO** corte, altere o perfore ningún miembro estructural de un truss, a menos que esté específicamente permitido en el dibujo del diseño del truss.*



NOTICE Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.

Trusses que se han sobrecargado durante la construcción o han sido alterados sin la autorización previa del Fabricante de Trusses, pueden hacer nulo y sin efecto la garantía limitada del Fabricante de Trusses.

***Contact the Component Manufacturer for more information or consult a Registered Design Professional for assistance.**
To view a non-printing PDF of this document, visit sbcindustry.com/b1.

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WOOD TRUSS COUNCIL

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¡ADVERTENCIA! HOJA RESUMEN DE LA GUÍA DE BUENA PRÁCTICA PARA EL MANEJO, INSTALACIÓN, RESTRICCIÓN Y ARRIOSTRE DE LOS TRUSSES

Vanos más de 60 pies pueden requerir arriostre permanente complejo. Por favor, siempre consulte a un Profesional Registrado de Diseño.